

SUBAREA 29 SPECIFIC PLAN AMENDMENT HYDROLOGY AND HYDRAULICS REPORT

Ontario, California

May 11, 2022

Prepared for:

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JN 40.074.000

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Exhibits

- Exhibit A – Proposed Condition Hydrology Map
- Exhibit B – 2012 Ontario MP Delineation
- Exhibit C – Hydrology Map Delineation Comparison

Technical Appendix

- A. Hydrology
- B. Hydraulics
- C. Excerpt from 2012 City of Ontario Master Plan of Drainage with Subsequent Approved Revisions
- D. Excerpt from “Preliminary H&H for San Bernardino County Flood Control District County Line Channel” (Hunsaker & Associates, 2011)
- E. USDA NRCS NCSS Soil Report

1 INTRODUCTION

1.1 Project Overview

Lewis Management Corp. and Richland Communities has requested Q3 Consulting (Q3) to prepare a hydrology and hydraulics study of Planning Areas (PA) 32, 33, and 34 located in the City of Ontario, in San Bernardino County. PA 32, 33, and 34 are new planning areas that are proposed to be added to the Subarea 29 Specific Plan as part of the Subarea 29 Specific Plan Amendment. PA 30 and PA 31 are part of the Subarea 29 Specific Plan Amendment area, however, PAs 30 and 31 would adhere to the runoff yields identified in the City of Ontario 2012 Master Plan of Drainage with Subsequent Approved Revisions (2012 Ontario MPD), which anticipates development of the original Subarea 29 Specific Plan area (including PAs 30 and 31).

The “project site” for this study includes PAs 32, 33 and 34, which encompass approximately 113 acres, and the adjacent public roadway right-of-way (approximately 11.7 acres). The project site is bounded to the west by Haven Ave, to the south by Bellegrave Ave, to the east by Mill Creek, and to the north by Eucalyptus Ave. Proposed runoff from the project site is planned to drain to the existing storm drain underneath Haven Ave, the existing storm drain underneath Mill Creek, and to the existing County drainage facility located along Bellegrave Avenue (County Line Channel). The storm drain lines along Haven Ave and Mill Creek confluence with the County Line Channel. From the County Line Channel, runoff is ultimately discharged into Cucamonga Channel.

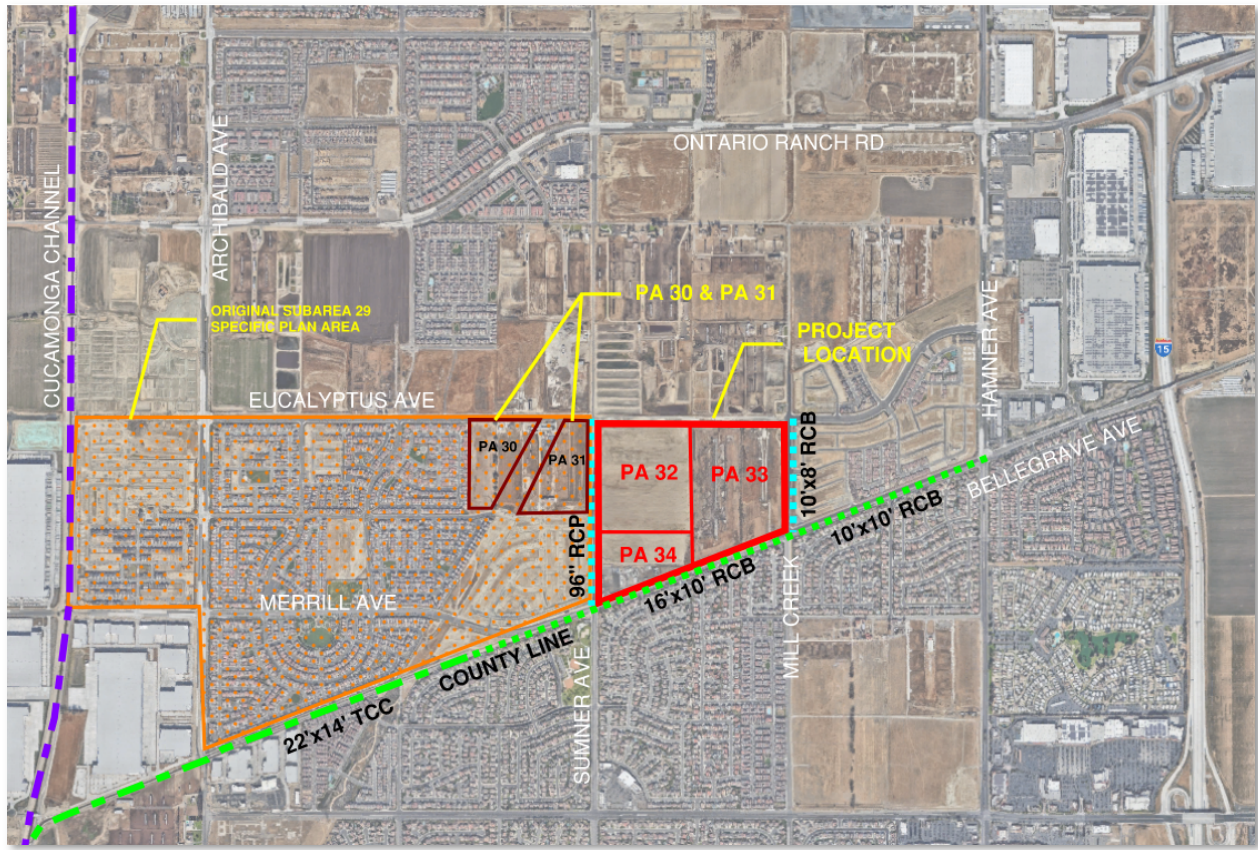
The land uses of PA 32, 33, 34 are proposed to be more impervious than what was proposed in the 2012 MPD. Specifically, the 2012 Ontario MPD assumed development of these PAs with low density residential area, school area, and lake area, and the Project includes the development of mixed residential and school uses. The following report provides an evaluation of the hydrologic and hydraulic impacts on the backbone storm drain lines along Mill Creek and Haven Ave, and the County Line Channel due to the amended land uses.

The location of the study area, project location, and the pertinent existing drainage facilities are shown in Figures 1-1 and 1-2.

Figure 1-1. Vicinity Map



Figure 1-2. Project Location Map



1.2 Goals and Objectives

The purpose of this report is to evaluate the hydrology and hydraulics of the revised land use plan and quantify any potential impacts and mitigation measures needed for the existing storm drain main lines and County Line Channel.

The primary objectives of this study include the following:

- Research, collect, and review previous studies completed in the watershed and along the project
- Establish the design criteria and requirements to be used for the hydrologic and hydraulic impact evaluation
- Calculate the 100-year peak flow per San Bernardino County Hydrology Manual methodology for the County Line Channel, the Haven Ave storm drain and the Mill Creek storm drain
- For each of the three drainage facilities, compare the difference between the peak flow rate from this analysis and the 2012 Ontario Master Plan of Drainage (MPD)
- Based on the hydrology results, develop Water Surface and Pressure Gradient (WSPG) models of the County Line Channel, the storm drain line under Mill Creek, and the storm drain line under Haven Avenue
- Provide recommendations for flood control mitigation measures if necessary

2 DESIGN CRITERIA

The hydrology and hydraulic criteria for the design of storm drain systems is based on the County of San Bernardino requirements. In general, the hydrology guidelines are stated in the San Bernardino County Hydrology Manual.

The 2012 Ontario MPD hydrology results were used as the basis for the evaluation of the County Line Channel and storm drain lines underneath Mill Creek and Haven Ave.

County Line Channel Point of Comparison (POC#1) for 100-Year Peak Flow:

For the County Line Channel analysis, the intersection at Bellegrave Ave and Haven Ave was selected as the point of comparison (POC) for peak flow. This POC is labeled as Node 4325 on the 2012 Ontario MP. At this location, the proposed development's impact on the 100-year peak flow in the CLC was compared to the peak flow previously calculated in the 2012 MP.

Haven Ave Storm Drain Point of Comparison (POC#2) for 100-Year Peak Flow:

For the hydrologic analysis of the storm drain along Haven Ave, just upstream of the confluence with the County Line Channel was selected as the point of comparison for peak flow.

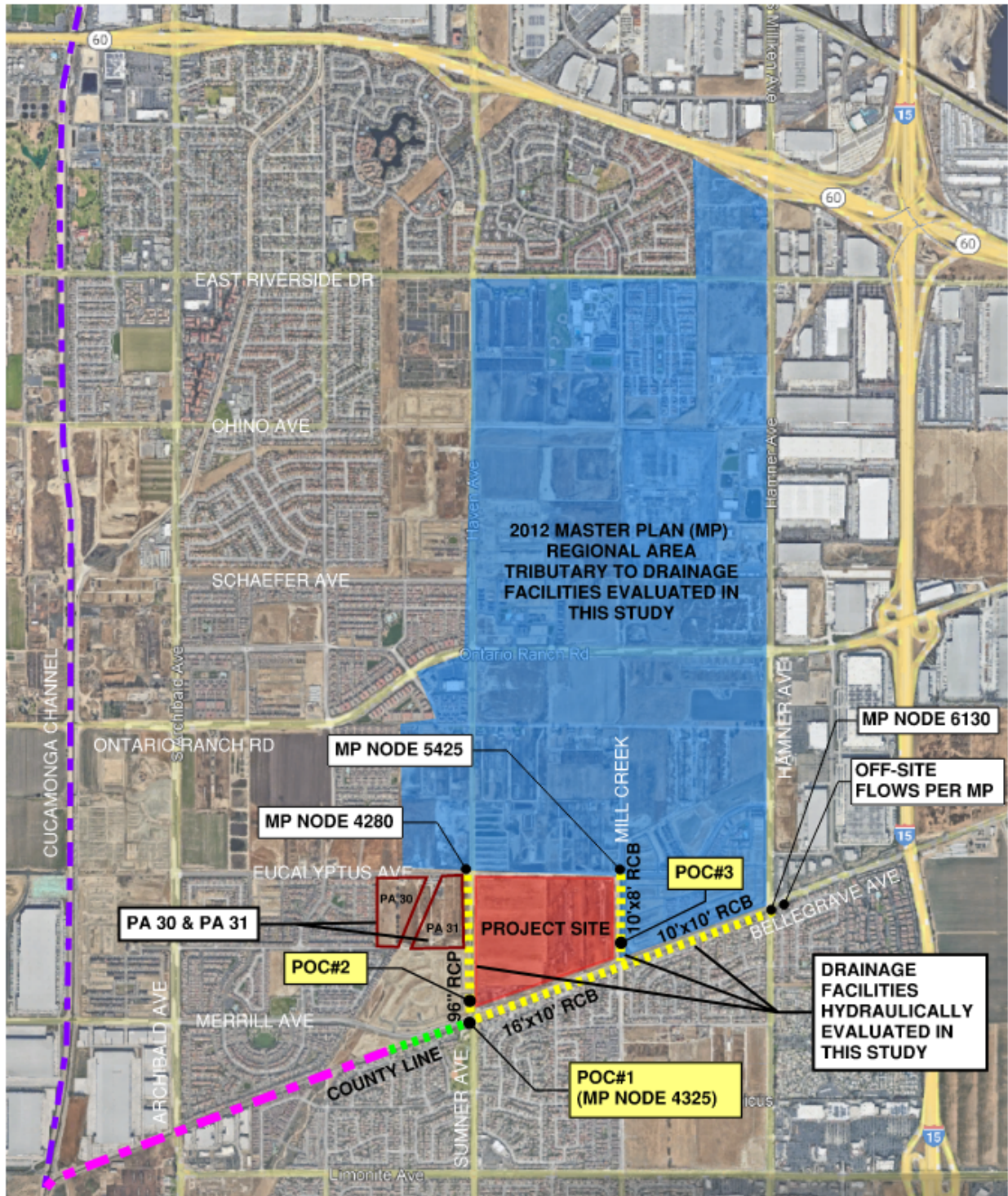
Mill Creek Storm Drain Point of Comparison (POC#3) for 100-Year Peak Flow:

For the hydrologic analysis of the storm drain along Mill Creek, just upstream of the confluence with the County Line Channel was selected as the point of comparison for peak flow.

County Line Channel Tailwater Elevation based on 2012 MP Hydrology:

A fixed tailwater elevation needed to be assumed for the hydraulic analysis of the County line. The tailwater elevation of 669.16 ft at Station 86+18.22 (approximately 500 feet downstream of the Haven Ave and Bellegrave Ave intersection) used for this analysis was taken from the Master Plan County line analysis in "Preliminary Hydrology and Hydraulic Study for the San Bernardino County Flood Control Districts County Line Channel" (2011, Hunsaker & Associates).

Figure 2-1. Regional Drainage Map



3 HYDROLOGY

In addition to the land use change, the proposed drainage pattern and delineation within PAs 32 to 34 differ from what was previously proposed in the 2012 Ontario MPD. Exhibit B and Exhibit C show the drainage delineation and patterns originally proposed in the 2012 Ontario MPD and provide a comparison to the current proposed design. New initial areas were delineated where there is a new inflow point along the County Line Channel that was not in the 2012 Ontario MPD.

3.1 Methodology

The hydrology was prepared in accordance with the San Bernardino County Hydrology Manual and used the same hydrologic parameters used in the 2012 Ontario MP. The 100-year peak flow rates were calculated using the Integrated Rational and Unit Hydrograph Method in Advanced Engineering Software (AES).

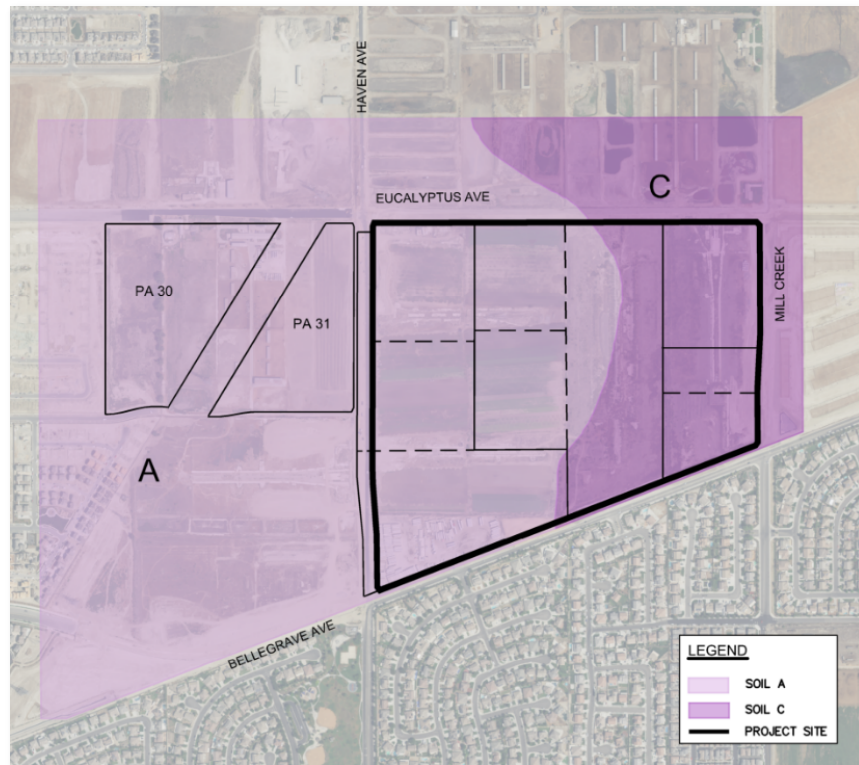
3.2 Rainfall Data

The same precipitation data used in the 2012 Ontario MP was used for this analysis. 100-year storm event, AMC 2, and 1-hour rainfall intensity of 1.2 in/hr was used.

3.3 Soil Type

The soil data from the United States Department of Agriculture Natural Resources Conservation Service National Cooperative Soil Survey (NCSS) was used for this analysis. PA 32 and PA 34 are primarily composed of soil group A and PA 33 is primarily composed of soil group C.

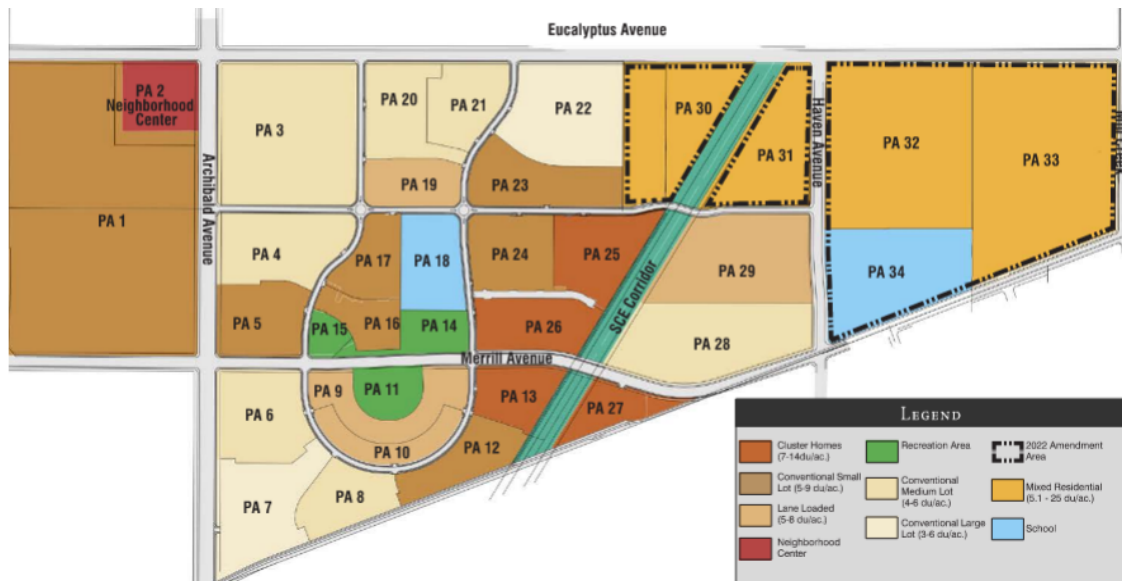
Figure 3-1. Soil Map



3.4 Land Use

The land uses of PA 32, 33, 34 are proposed to be more impervious than what was proposed in the 2012 Ontario MP. In the MP, PAs 32, 33, 34 were proposed to have low density residential area, school area, and lake area. Currently, PA 32 and PA 33 are proposed to be mixed residential and PA 34 is proposed to be for school use. For the hydrologic analysis, the land uses for PA 32 and PA 33 were selected from the standard land uses defined in San Bernardino County Hydrology Manual (SBCHM) that most resembled the PA’s imperviousness and intended function. PA 32 and PA 33 were assigned the SBCHM land use “Apartments” and PA 34 was assigned the SBCHM land use of “School.” The SBCHM “Apartment” land use is 80% impervious and the SBCHM “School” land use is 40% impervious. Figure 3.2 shows the proposed land uses for this study.

Figure 3-2. Proposed Land Use Plan



3.5 Regional Master Plan Flows

The tributary Master Plan hydrology for the following locations was included in this analysis:

- MP Node 4280 (intersection at Haven Ave and Eucalyptus Ave)
- MP Node 5425 (intersection at Mill Creek and Eucalyptus Ave)
- Tributary areas along Bellegrave Ave from Mill Creek to Hamner Ave
- MP Node 6130 (intersection at Hamner Ave and Bellegrave Ave)
- Master Plan off-site flows from Hamner Ave (formerly known as Milliken Ave)

3.6 Hydrology Results

The following table shows the peak 100-year flowrate at locations along both the City and County drainage systems. The node located furthest downstream in the analysis is along the County Line Channel at MP Node 4325/ POC #1.

Table 3-1. Hydrology Point of Comparison (POC) Results

POC	Drainage System	Total Tributary Area per 2012 MP (ac)	Proposed Total Tributary Area (ac)	Tc per 2012 MP (min)	Proposed Tc (min)	Q ₁₀₀ per 2012 MP (cfs)	Proposed Q ₁₀₀ (cfs)
1	County Line Channel	2,360	2,361.5	43.7	43.46	2,385.1	2,388.50
2	Haven SD	592.1	556.6	22.7	22.39	755	700.43
3	Mill Creek SD	886.1	864.9	39.4	39.34	1,153.5	1,129.93

At POC#1/MP Node 4325 along the County Line Channel, the 100-year peak flow rate is 2,388.5 cfs and the 2012 Ontario MPD peak flow rate was 2,385.1 cfs. There is an increase of 3.4 cfs to the County Line Channel due to the proposed design. Section 4 of this report demonstrates that the County Line Channel has the capacity to convey an additional 3.4 cfs and the increase in peak flow rate would not have an impact on the facility.

For Haven and Mill Creek storm drains (POC #2, POC #3), the peak flows are less than the 2012 Ontario MPD peak flows. This is primarily due to the reduction of proposed flows to discharge to the Haven Ave and Mill Creek storm drains compared to the previous 2012 Ontario MPD. In the current proposed design, areas that were once tributary to the Haven and Mill Creek storm drain lines (Areas N6, N7, P1, P3, P4 as shown in Exhibit A) in the MPD are now proposed to drain directly to the County Line Channel. Exhibit B and Exhibit C demonstrate the difference in tributary areas for the Haven and Mill Creek storm drains between the MPD and the currently proposed design.

4 HYDRAULIC ANALYSES

4.1 Methodology

The hydraulic analysis of the County Line Channel and the Mill Creek and Haven Ave storm drain lines was computed using the Water Surface and Pressure Gradient (WSPG) computer program.

The County line was hydraulically evaluated from Haven Ave to Mill Creek. The Haven storm drain was evaluated from Bellegrave Ave to Eucalyptus Ave. The Mill Creek storm drain was evaluated from Bellegrave Ave to Eucalyptus Ave.

County Line Channel Tailwater Elevation based on 2012 MP Hydrology:

A fixed tailwater elevation needed to be assumed for the hydraulic analysis of the County line. The tailwater elevation of 669.16 ft at Station 86+18.22 (approximately 500 feet downstream of the Haven Ave and Bellegrave Ave intersection) used for this analysis was taken from the Master Plan County Line analysis in “Preliminary Hydrology and Hydraulic Study for the San Bernardino County Flood Control Districts County Line Channel” (2011, Hunsaker & Associates).

Mainline Storm Drain Tailwater Elevation based on Updated County Line Hydrology:

A tailwater is the water surface elevation at the most downstream end of a drainage facility. The fixed tailwater elevations used for the Mill Creek storm drain and Haven Ave storm drain were taken from the updated hydraulic results of the County Line Channel.

4.2 Hydraulic Results

The updated hydraulic grade line (HGL) for the Haven Ave storm drain line is above the as-built HGL an approximate maximum of 0.6 feet from the County Line Channel confluence point to approximately 500 feet upstream the storm drain line. The increase in HGL is primarily due to the updated tailwater elevation from the County Line Channel, which is approximately 0.6 feet higher than the tailwater used on the as-built plans. For the remaining upstream portion of the Haven storm drain line, the updated HGL is below the as-built HGL. This is primarily due to the reduction in proposed flows to discharge to the Haven Ave storm drain compared to the previous 2012 Ontario MPD.

The updated hydrology (Q100 peak flowrate) for the Mill Creek storm drain is less than that identified in the 2012 Ontario MPD. Both this study and the 2012 Ontario MPD showed Q100 peak flowrates slightly higher than those identified in the as-built plans. Since the MPD is an approved report, the decrease in flowrate per this study is comparatively a benefit to the proposed condition flows identified in the MPD. The calculated HGL for the Mill Creek storm drain line varies between 0-1 feet above that identified in the as-built plans from approximately Sta 17+00 to Sta 24+79.67. However, within this reach there is a minimum of 3 feet of freeboard in the system. A low point exists in the proposed finished surface at approximately Sta 12+00. At this location the calculated HGL is similar to the as-built HGL and there is no calculated impact to the storm drain line.

The County Line Channel HGL based on the updated hydrology results is below the HGL that is shown in the as-builts. The updated HGL is above the MPD HGL from Hamner Avenue to approximately 1,400 feet downstream of Mill Creek (Node 8003 on Exhibit A) at a maximum of 0.2 feet, yet well below the finished grade. From MPD Node 8003 to Hamner Avenue the updated HGL is greater than the MPD HGL at a maximum of 0.1 feet. Based on the findings of this study the County Line Channel has the capacity to convey the updated 100-year peak flow.

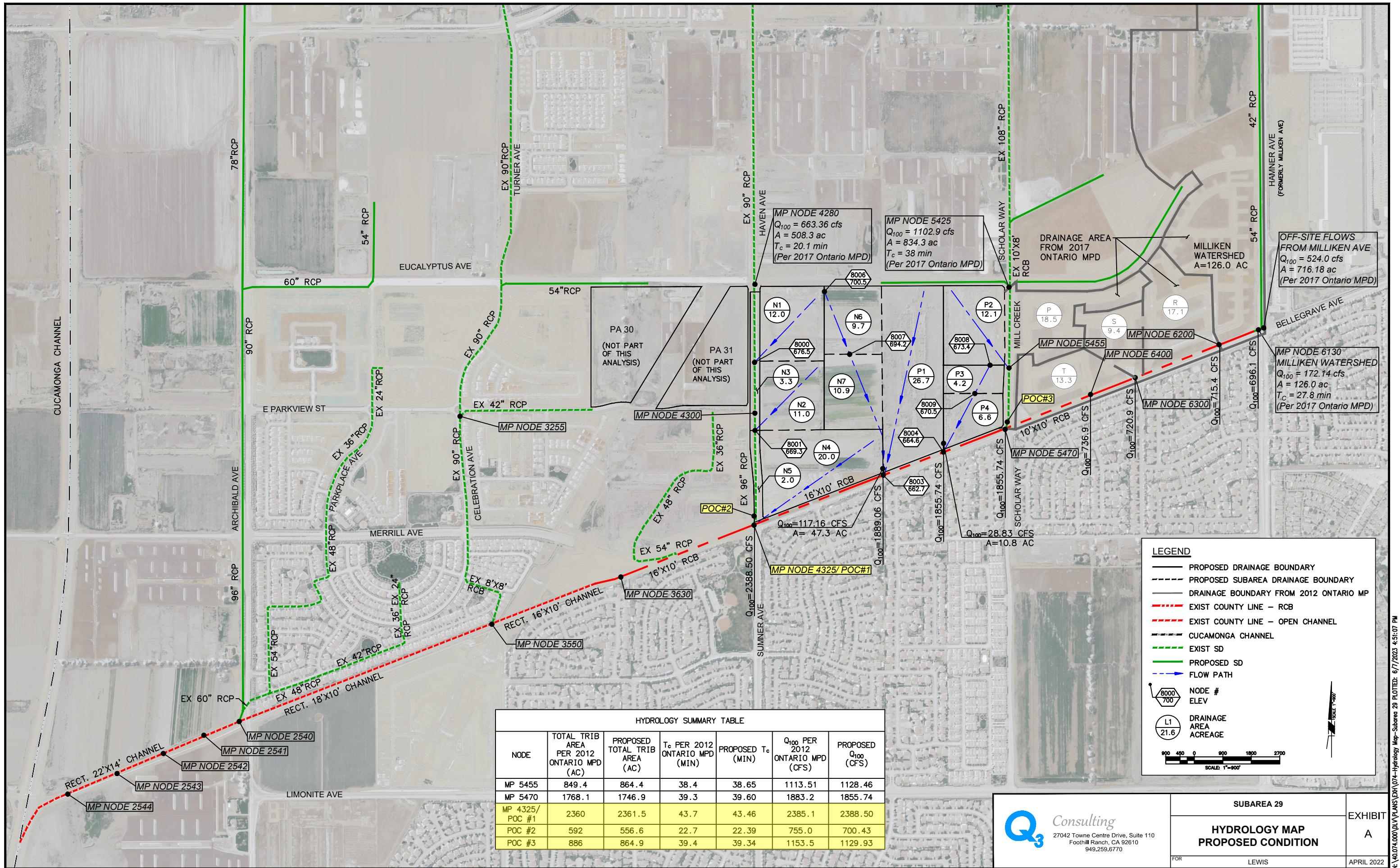
5 CONCLUSIONS

The updated HGL in Haven Avenue storm drain is above the as-built HGL by a maximum of 0.6 feet from the confluence point with the County Line Channel to approximately 500 feet upstream along the storm drain line. The remaining upstream portion of the Haven storm drain line is below the as-built HGL. The peak flows in the storm drain system are less than those identified in the approved MPD.

The calculated HGL for the Mill Creek storm drain line varies between 0-1 feet above that identified in the as-built plans from approximately Sta 17+00 to Sta 24+79.67. However, within this reach there is a minimum of 3 feet of freeboard in the system. A low point exists in the proposed finished surface at approximately Sta 12+00. At this location the calculated HGL is similar to the as-built HGL and there is no calculated impact to the storm drain line. The calculated peak flows in the storm drain system are less than those identified in the approved MPD.

The proposed peak 100-year flow calculated in this study for the County Line Channel is 3.4 cfs higher than the flows identified in the MPD at the Haven Avenue and Bellegrave Avenue intersection. The updated HGL in the County Line Channel is below the as-built HGL and slightly above the MPD HGL by a maximum of 0.2 feet. This slight increase in HGL still contains freeboard to the finished grade.

Based on this analysis, the County Channel Line and main storm drain lines servicing PAs 32, 33, and 34 maintain adequate freeboard capacity for the proposed change in land use designations. Therefore, development of PAs 32, 33 and 34 per the proposed Subarea 29 Specific Plan Amendment, would not require new or expanded storm drainage facilities beyond the existing storm drain system.



HYDROLOGY SUMMARY TABLE

NODE	TOTAL TRIB AREA PER 2012 ONTARIO MPD (AC)	PROPOSED TOTAL TRIB AREA (AC)	T _c PER 2012 ONTARIO MPD (MIN)	PROPOSED T _c (MIN)	Q ₁₀₀ PER 2012 ONTARIO MPD (CFS)	PROPOSED Q ₁₀₀ (CFS)
MP 5455	849.4	864.4	38.4	38.65	1113.51	1128.46
MP 5470	1768.1	1746.9	39.3	39.60	1883.2	1855.74
MP 4325/ POC #1	2360	2361.5	43.7	43.46	2385.1	2388.50
POC #2	592	556.6	22.7	22.39	755.0	700.43
POC #3	886	864.9	39.4	39.34	1153.5	1129.93

LEGEND

- PROPOSED DRAINAGE BOUNDARY
- - - PROPOSED SUBAREA DRAINAGE BOUNDARY
- DRAINAGE BOUNDARY FROM 2012 ONTARIO MPD
- EXIST COUNTY LINE - RCB
- EXIST COUNTY LINE - OPEN CHANNEL
- CUCAMONGA CHANNEL
- EXIST SD
- PROPOSED SD
- FLOW PATH
- 8000/700 NODE # ELEV
- L1/21.6 DRAINAGE AREA ACREAGE

900 450 0 900 1800 2700
SCALE: 1"=900'

Q3 Consulting
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Foothill Ranch, CA 92610
949.259.6770

SUBAREA 29

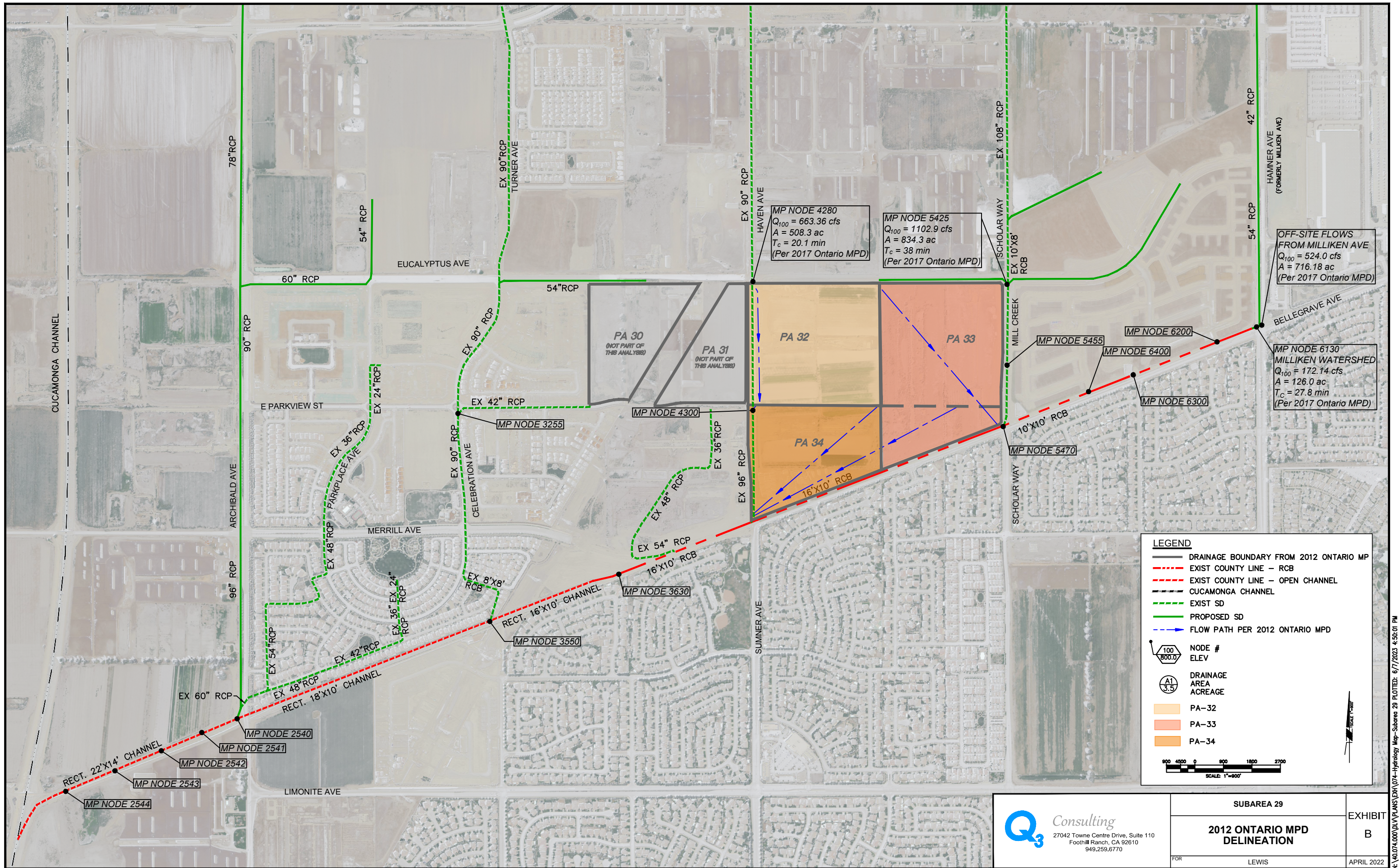
**HYDROLOGY MAP
PROPOSED CONDITION**

FOR LEWIS

EXHIBIT
A

APRIL 2022

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MP NODE 4280
 $Q_{100} = 663.36 \text{ cfs}$
 $A = 508.3 \text{ ac}$
 $T_c = 20.1 \text{ min}$
 (Per 2017 Ontario MPD)

MP NODE 5425
 $Q_{100} = 1102.9 \text{ cfs}$
 $A = 834.3 \text{ ac}$
 $T_c = 38 \text{ min}$
 (Per 2017 Ontario MPD)

OFF-SITE FLOWS FROM MILLIKEN AVE
 $Q_{100} = 524.0 \text{ cfs}$
 $A = 716.18 \text{ ac}$
 (Per 2017 Ontario MPD)

MP NODE 6130 MILLIKEN WATERSHED
 $Q_{100} = 172.14 \text{ cfs}$
 $A = 126.0 \text{ ac}$
 $T_c = 27.8 \text{ min}$
 (Per 2017 Ontario MPD)

LEGEND

- DRAINAGE BOUNDARY FROM 2012 ONTARIO MPD
- - - EXIST COUNTY LINE - RCB
- - - EXIST COUNTY LINE - OPEN CHANNEL
- - - CUCAMONGA CHANNEL
- - - EXIST SD
- PROPOSED SD
- FLOW PATH PER 2012 ONTARIO MPD

NODE #
 ELEV

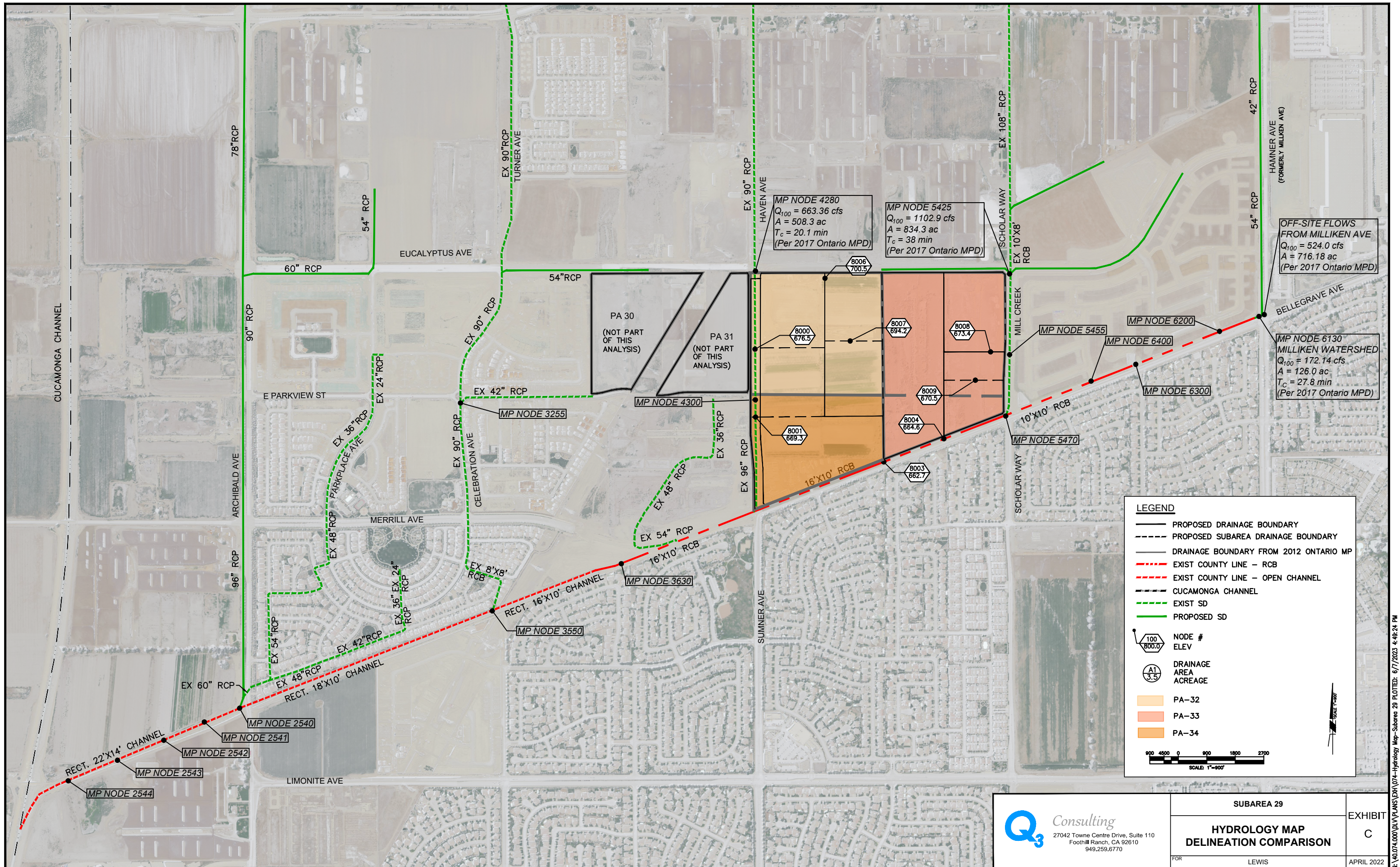
DRAINAGE AREA
 ACREAGE

PA-32
 PA-33
 PA-34

SCALE: 1"=900'

<p>Q3 Consulting 27042 Towne Centre Drive, Suite 110 Foothill Ranch, CA 92610 949.259.6770</p>	SUBAREA 29		EXHIBIT B
	2012 ONTARIO MPD DELINEATION		
FOR	LEWIS	APRIL 2022	

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MP NODE 4280
 $Q_{100} = 663.36$ cfs
 $A = 508.3$ ac
 $T_c = 20.1$ min
 (Per 2017 Ontario MPD)

MP NODE 5425
 $Q_{100} = 1102.9$ cfs
 $A = 834.3$ ac
 $T_c = 38$ min
 (Per 2017 Ontario MPD)

OFF-SITE FLOWS FROM MILLIKEN AVE
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 (Per 2017 Ontario MPD)

MP NODE 6130 MILLIKEN WATERSHED
 $Q_{100} = 172.14$ cfs
 $A = 126.0$ ac
 $T_c = 27.8$ min
 (Per 2017 Ontario MPD)

LEGEND

- PROPOSED DRAINAGE BOUNDARY
- - - PROPOSED SUBAREA DRAINAGE BOUNDARY
- DRAINAGE BOUNDARY FROM 2012 ONTARIO MP
- EXIST COUNTY LINE - RCB
- EXIST COUNTY LINE - OPEN CHANNEL
- CUCAMONGA CHANNEL
- EXIST SD
- PROPOSED SD

NODE #
 ELEV

DRAINAGE AREA
 ACREAGE

PA-32
 PA-33
 PA-34

SCALE: 1"=900'

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 27042 Towne Centre Drive, Suite 110
 Foothill Ranch, CA 92610
 949.259.6770

SUBAREA 29		EXHIBIT C
HYDROLOGY MAP DELINEATION COMPARISON		
FOR	LEWIS	APRIL 2022

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APPENDIX

Appendix A – Hydrology

- **County Line Channel 100-yr AES Results**
- **Haven Ave SD 100-yr AES Results**
- **Mill Creek SD 100-yr AES Results**
- **Milliken Watershed 100-yr AES Results**



FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
SIERRA MADRE DEPTH-AREA FACTORS USED.

AREA-AVERAGED	
DURATION	RAINFALL (INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 (Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
 (c) Copyright 1983-2015 Advanced Engineering Software (aes)
 Ver. 22.0 Release Date: 07/01/2015 License ID 1673

Analysis prepared by:

County Line Channel - Proposed Condition

***** DESCRIPTION OF STUDY *****
 * SUBAREA 29 *
 * COUNTY LINE CHANNEL *
 * PROPOSED CONDITION 100YR *

FILE NAME: COUNTY.DAT
 TIME/DATE OF STUDY: 12:39 04/22/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL
 HALF- CROWN TO STREET-CROSSFALL: CURB GUTTER-GEOMETRIES: MANNING
 WIDTH CROSSFALL IN- / OUT-/PARK- HEIGHT WIDTH LIP HIKE FACTOR
 NO. (FT) (FT) SIDE / SIDE/ WAY (FT) (FT) (FT) (FT) (n)
 === =====

1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150
---	------	------	-------------------	------	------	--------	-------	--------

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:

WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF
 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH

 FLOW PROCESS FROM NODE 6000.00 TO NODE 6130.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<< **Milliken Watershed per 2012 MP**

=====

PEAK FLOWRATE TABLE FILE NAME: MILLIKEN.DNA
 MEMORY BANK # 1 DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	172.14	27.82	0.65(0.20)	0.31	110.5	6080.00
2	171.76	29.40	0.65(0.20)	0.31	114.4	6110.00
3	161.71	36.89	0.66(0.19)	0.30	126.0	6000.00

TOTAL AREA(ACRES) = 126.0
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 14.0

>>>>MEMORY BANK # 1 COPIED ONTO MAIN-STREAM MEMORY<<<<<

=====

MAIN-STREAM MEMORY DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	172.14	27.82	0.65(0.20)	0.31	110.5	6080.00
2	171.76	29.40	0.65(0.20)	0.31	114.4	6110.00
3	161.71	36.89	0.66(0.19)	0.30	126.0	6000.00

TOTAL AREA(ACRES) = 126.0
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 16

>>>>USER SPECIFIED CONSTANT SOURCE FLOW AT NODE<<<<< **Offsite Flows per 2012 MP**

=====

USER-SPECIFIED CONSTANT SOURCE FLOW = 524.00(CFS)
 USER-SPECIFIED AREA ASSOCIATED TO SOURCE FLOW = 716.18(ACRES)
 * CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.18
 * SUMMED DATA: FLOW(CFS) = 696.14 TOTAL AREA(ACRES) = 842.18

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6200.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 673.60 DOWNSTREAM(FEET) = 672.60
 FLOW LENGTH(FEET) = 374.00 MANNING'S N = 0.015
 GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
 FLOWDEPTH IN BOX IS 6.73 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.34
 BOX-FLOW(CFS) = 696.14
 BOX-FLOW TRAVEL TIME(MIN.) = 0.60 Tc(MIN.) = 28.43
 * TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6200.00 = 13064.00 FEET.

 FLOW PROCESS FROM NODE 6200.00 TO NODE 6200.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 28.43 **Area R per 2012 MP**
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.879

SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "5-7 DWELLINGS/ACRE" C 17.10 0.57 0.500 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 17.10 SUBAREA RUNOFF(CFS) = 24.56
 EFFECTIVE AREA(ACRES) = 127.61 AREA-AVERAGED Fm(INCH/HR) = 0.21
 AREA-AVERAGED Fp(INCH/HR) = 0.63 AREA-AVERAGED Ap = 0.34
 TOTAL AREA(ACRES) = 143.1 PEAK FLOW RATE(CFS) = 191.30

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
 * SUMMED DATA: FLOW(CFS) = 715.30 TOTAL AREA(ACRES) = 859.3

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	SOURCE FLOW
1	191.41	28.40	1.880	0.63(0.21)	0.34	127.6	6080.00	524.0
2	190.26	29.95	1.821	0.63(0.21)	0.34	131.5	6110.00	0.0
3	178.71	37.43	1.593	0.64(0.21)	0.32	143.1	6000.00	0.0

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 191.41 Tc(MIN.) = 28.40
 AREA-AVERAGED Fm(INCH/HR) = 0.21 AREA-AVERAGED Fp(INCH/HR) = 0.63
 AREA-AVERAGED Ap = 0.34 EFFECTIVE AREA(ACRES) = 127.61

* CUMULATIVE SOURCE FLOW DATA:

FLOW(CFS) = 524.00 AREA(ACRES) = 716.2

* SUMMED DATA:

FLOW(CFS) = 715.41 TOTAL AREA(ACRES) = 843.8

 FLOW PROCESS FROM NODE 6200.00 TO NODE 6300.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 672.60 DOWNSTREAM(FEET) = 669.40
 FLOW LENGTH(FEET) = 1140.00 MANNING'S N = 0.015
 GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
 FLOWDEPTH IN BOX IS 6.75 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.60
 BOX-FLOW(CFS) = 715.41
 BOX-FLOW TRAVEL TIME(MIN.) = 1.79 Tc(MIN.) = 30.19

* TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6300.00 = 14204.00 FEET.

 FLOW PROCESS FROM NODE 6300.00 TO NODE 6300.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 30.19 **Area S per 2012 MP**
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.812

SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "5-7 DWELLINGS/ACRE" C 9.40 0.57 0.500 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 9.40 SUBAREA RUNOFF(CFS) = 12.93
 EFFECTIVE AREA(ACRES) = 137.01 AREA-AVERAGED Fm(INCH/HR) = 0.22
 AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 152.5 PEAK FLOW RATE(CFS) = 196.56

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
 * SUMMED DATA: FLOW(CFS) = 720.56 TOTAL AREA(ACRES) = 868.7

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	SOURCE FLOW
1	196.89	30.12	1.814	0.62(0.22)	0.35	137.0	6080.00	524.0
2	195.96	31.60	1.763	0.63(0.22)	0.35	140.9	6110.00	0.0
3	184.39	39.02	1.553	0.63(0.21)	0.33	152.5	6000.00	0.0

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 196.89 Tc(MIN.) = 30.12
 AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.62
 AREA-AVERAGED Ap = 0.35 EFFECTIVE AREA(ACRES) = 137.01

* CUMULATIVE SOURCE FLOW DATA:

FLOW(CFS) = 524.00 AREA(ACRES) = 716.2

* SUMMED DATA:

FLOW(CFS) = 720.89 TOTAL AREA(ACRES) = 853.2

 FLOW PROCESS FROM NODE 6300.00 TO NODE 6400.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 669.40 DOWNSTREAM(FEET) = 668.60
 FLOW LENGTH(FEET) = 293.00 MANNING'S N = 0.015
 GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
 FLOWDEPTH IN BOX IS 6.86 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.51
 BOX-FLOW(CFS) = 720.89
 BOX-FLOW TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 30.59
 * TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6400.00 = 14497.00 FEET.

 FLOW PROCESS FROM NODE 6400.00 TO NODE 6400.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 30.59
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.798
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE" C 12.30 0.57 0.500 69
PUBLIC PARK C 1.00 0.57 0.850 69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.526
SUBAREA AREA(ACRES) = 13.30 SUBAREA RUNOFF(CFS) = 17.95
EFFECTIVE AREA(ACRES) = 150.31 AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.36
TOTAL AREA(ACRES) = 165.8 PEAK FLOW RATE(CFS) = 212.80
* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
* SUMMED DATA: FLOW(CFS) = 736.80 TOTAL AREA(ACRES) = 882.0

Area T per
2012 MP

** PEAK FLOW RATE TABLE **

Table with 9 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE, SOURCE FLOW. Contains 3 rows of data.

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 212.88 Tc(MIN.) = 30.57
AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.62
AREA-AVERAGED Ap = 0.36 EFFECTIVE AREA(ACRES) = 150.31

* CUMULATIVE SOURCE FLOW DATA:

FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
* SUMMED DATA:
FLOW(CFS) = 736.88 TOTAL AREA(ACRES) = 866.5

FLOW PROCESS FROM NODE 6400.00 TO NODE 5470.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 668.60 DOWNSTREAM(FEET) = 666.10
FLOW LENGTH(FEET) = 990.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
FLOWDEPTH IN BOX IS 7.20 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.24
BOX-FLOW(CFS) = 736.88
BOX-FLOW TRAVEL TIME(MIN.) = 1.61 Tc(MIN.) = 32.18
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 5470.00 = 15487.00 FEET.

FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<

PEAK FLOWRATE TABLE FILE NAME: PRMCRK.DNA
MEMORY BANK # 1 DEFINED AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1129.93 Tc(MIN.) = 39.34
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 864.9
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.

FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

Mill Creek SD

** MAIN STREAM CONFLUENCE DATA **

Table with 9 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE, SOURCE FLOW. Contains 3 rows of data.

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18

LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 5470.00 = 15487.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

PEAK FLOW RATE(CFS) = 1129.93 Tc(MIN.) = 39.34
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 864.9
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:
RAINFALL (INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.66; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;
3HR = 0.99; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1030.7
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0298; Lca/L=0.4,n=.0267; Lca/L=0.5,n=.0246;Lca/L=0.6,n=.0229
TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 395.87
PEAK FLOW RATE(CFS) = 1331.74
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 1855.74 TOTAL AREA(ACRES) = 1746.9

FLOW PROCESS FROM NODE 5470.00 TO NODE 8004.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 666.50 DOWNSTREAM(FEET) = 664.60
FLOW LENGTH(FEET) = 689.20 MANNING'S N = 0.014
GIVEN BOX BASEWIDTH(FEET) = 16.00 GIVEN BOX HEIGHT(FEET) = 10.00

*GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 16.47
 ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 11.27
 BOX-FLOW(CFS) = 1855.74
 BOX-FLOW TRAVEL TIME(MIN.) = 1.02 Tc(MIN.) = 40.36
 * TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8004.00 = 17327.20 FEET.

 FLOW PROCESS FROM NODE 8004.00 TO NODE 8004.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 8004.00 TO NODE 8004.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<< **Flows from P3 & P4**

PEAK FLOWRATE TABLE FILE NAME: p3.DNA
 MEMORY BANK # 1 DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap (INCH/HR)	Ae (ACRES)	HEADWATER NODE
1	28.83	12.47	0.57(0.11)	0.20	10.8	8008.00

 TOTAL AREA(ACRES) = 10.8
 LONGEST FLOWPATH FROM NODE 8008.00 TO NODE 8004.00 = 1026.80 FEET.

 FLOW PROCESS FROM NODE 8004.00 TO NODE 8004.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **
 PEAK FLOW RATE(CFS) = 1331.74 Tc(MIN.) = 40.36
 AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
 TOTAL AREA(ACRES) = 1030.7

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8004.00 = 17327.20 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap (INCH/HR)	Ae (ACRES)	HEADWATER NODE
1	28.83	12.47	3.079	0.57(0.11)	0.20	10.8	8008.00

 LONGEST FLOWPATH FROM NODE 8008.00 TO NODE 8004.00 = 1026.80 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.67; LAG(HR) = 0.54; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;
 3HR = 0.99; 6HR = 1.00; 24HR = 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1041.5
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8004.00 = 17327.20 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0295; Lca/L=0.4,n=.0265; Lca/L=0.5,n=.0243;Lca/L=0.6,n=.0227

TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 400.54
 PEAK FLOW RATE(CFS) = 1328.80
 (UPSTREAM NODE PEAK FLOW RATE(CFS) = 1331.74)
 PEAK FLOW RATE(CFS) USED = 1331.74
 * CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2
 * SUMMED DATA: FLOW(CFS) = 1855.74 TOTAL AREA(ACRES) = 1757.7

 FLOW PROCESS FROM NODE 8004.00 TO NODE 8003.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 664.60 DOWNSTREAM(FEET) = 662.70
 FLOW LENGTH(FEET) = 670.20 MANNING'S N = 0.014
 GIVEN BOX BASEWIDTH(FEET) = 16.00 GIVEN BOX HEIGHT(FEET) = 10.00
 *GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 16.29
 ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 11.39
 BOX-FLOW(CFS) = 1855.74
 BOX-FLOW TRAVEL TIME(MIN.) = 0.98 Tc(MIN.) = 41.34
 * TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8003.00 = 17997.40 FEET.

 FLOW PROCESS FROM NODE 8003.00 TO NODE 8003.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 8003.00 TO NODE 8003.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<

PEAK FLOWRATE TABLE FILE NAME: np.DNA **Flows from N6, N7 & P1**
 MEMORY BANK # 1 DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap (INCH/HR)	Ae (ACRES)	HEADWATER NODE
1	117.16	13.65	0.82(0.16)	0.20	47.3	8006.00

 TOTAL AREA(ACRES) = 47.3
 LONGEST FLOWPATH FROM NODE 8006.00 TO NODE 8003.00 = 2011.96 FEET.

 FLOW PROCESS FROM NODE 8003.00 TO NODE 8003.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **
 PEAK FLOW RATE(CFS) = 1331.74 Tc(MIN.) = 41.34
 AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
 TOTAL AREA(ACRES) = 1041.5

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8003.00 = 17997.40 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap (INCH/HR)	Ae (ACRES)	HEADWATER NODE
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1 117.16 13.65 2.917 0.82(0.16) 0.20 47.3 8006.00
 LONGEST FLOWPATH FROM NODE 8006.00 TO NODE 8003.00 = 2011.96 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00

S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.69; LAG(HR) = 0.55; Fm(INCH/HR) = 0.22; Ybar = 0.24

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;

3HR = 0.99; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1088.8

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 8003.00 = 17997.40 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0292; Lca/L=0.4,n=.0262; Lca/L=0.5,n=.0241;Lca/L=0.6,n=.0225

TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 419.79

PEAK FLOW RATE(CFS) = 1365.06

* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2

* SUMMED DATA: FLOW(CFS) = 1889.06 TOTAL AREA(ACRES) = 1805.0

FLOW PROCESS FROM NODE 8003.00 TO NODE 4325.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 662.70 DOWNSTREAM(FEET) = 658.70

FLOW LENGTH(FEET) = 1442.10 MANNING'S N = 0.014

GIVEN BOX BASEWIDTH(FEET) = 16.00 GIVEN BOX HEIGHT(FEET) = 10.00

*GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 16.67

ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 11.34

BOX-FLOW(CFS) = 1889.06

BOX-FLOW TRAVEL TIME(MIN.) = 2.12 Tc(MIN.) = 43.46

* TOTAL SOURCE FLOW(CFS) = 524.00

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19439.50 FEET.

FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<

Haven SD

PEAK FLOWRATE TABLE FILE NAME: PRHAVEN.DNA

MEMORY BANK # 1 DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	683.12	18.66	0.96(0.35)	0.36	366.6	4175.00
2	700.43	22.39	0.96(0.35)	0.36	427.8	4210.00
3	690.56	31.86	0.96(0.36)	0.37	549.1	4020.00
4	684.33	32.70	0.96(0.36)	0.37	555.4	1.00
5	684.05	32.72	0.96(0.36)	0.37	555.5	4000.00
6	676.74	33.26	0.96(0.36)	0.37	556.6	4045.00
TOTAL AREA(ACRES) =						556.6

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4325.00 = 15347.70 FEET.

FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

PEAK FLOW RATE(CFS) = 1365.06 Tc(MIN.) = 43.46

AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.24

TOTAL AREA(ACRES) = 1088.8

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19439.50 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	683.12	18.66	2.418	0.96(0.35)	0.36	366.6	4175.00
2	700.43	22.39	2.168	0.96(0.35)	0.36	427.8	4210.00
3	690.56	31.86	1.754	0.96(0.36)	0.37	549.1	4020.00
4	684.33	32.70	1.727	0.96(0.36)	0.37	555.4	1.00
5	684.05	32.72	1.727	0.96(0.36)	0.37	555.5	4000.00
6	676.74	33.26	1.710	0.96(0.36)	0.37	556.6	4045.00
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4325.00 = 15347.70 FEET.							

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00

S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.72; LAG(HR) = 0.58; Fm(INCH/HR) = 0.26; Ybar = 0.29

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.93; 30M = 0.93; 1HR = 0.93;

3HR = 0.99; 6HR = 0.99; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1645.3

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19439.50 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0287; Lca/L=0.4,n=.0257; Lca/L=0.5,n=.0236;Lca/L=0.6,n=.0221

TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 596.73

PEAK FLOW RATE(CFS) = 1864.50

* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2

* SUMMED DATA: FLOW(CFS) = 2388.50 TOTAL AREA(ACRES) = 2361.5

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 1645.3 TC(MIN.) = 43.46

AREA-AVERAGED Fm(INCH/HR)= 0.26 Ybar = 0.29

PEAK FLOW RATE(CFS) = 1864.50

* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2

* SUMMED DATA: FLOW(CFS) = 2388.50 TOTAL AREA(ACRES) = 2361.5

END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS



RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 (Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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Analysis prepared by:

Haven Ave - Recreated 2012 MP Hydrology Tributary to MP Node 4280

***** DESCRIPTION OF STUDY *****

- * SUBAREA 29 *
- * HAVEN AVE SD *
- * 2017 ONTARIO MP - TRIB TO MP NODE 4280 *

FILE NAME: MPHAVEN.DAT
 TIME/DATE OF STUDY: 11:27 04/22/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL IN- / OUT- / PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GEOMETRIES LIP (FT)	MANNING HIKE (FT)	FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
 *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:

WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
 SIERRA MADRE DEPTH-AREA FACTORS USED.

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

FLOW PROCESS FROM NODE 4000.00 TO NODE 4005.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 469.00
 ELEVATION DATA: UPSTREAM(FEET) = 790.50 DOWNSTREAM(FEET) = 787.60

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.594
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.062
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	2.44	0.98	0.500	32	12.59

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA RUNOFF(CFS) = 5.65
 TOTAL AREA(ACRES) = 2.44 PEAK FLOW RATE(CFS) = 5.65

FLOW PROCESS FROM NODE 4005.00 TO NODE 4010.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 787.60 DOWNSTREAM ELEVATION(FEET) = 776.50
 STREET LENGTH(FEET) = 585.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.018
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 15.37
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.42
HALFSTREET FLOOD WIDTH(FEET) = 14.57
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.68
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.55
STREET FLOW TRAVEL TIME(MIN.) = 2.65 Tc(MIN.) = 15.25
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.730
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	A	9.60	0.98	0.500	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500					
SUBAREA AREA(ACRES) = 9.60 SUBAREA RUNOFF(CFS) = 19.38					
EFFECTIVE AREA(ACRES) = 12.04 AREA-AVERAGED Fm(INCH/HR) = 0.49					
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.50					
TOTAL AREA(ACRES) = 12.0 PEAK FLOW RATE(CFS) = 24.30					

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.48 HALFSTREET FLOOD WIDTH(FEET) = 17.62
FLOW VELOCITY(FEET/SEC.) = 4.10 DEPTH*VELOCITY(FT*FT/SEC.) = 1.96
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4010.00 = 1054.00 FEET.

FLOW PROCESS FROM NODE 4010.00 TO NODE 4015.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 770.50	DOWNSTREAM(FEET) = 766.00
FLOW LENGTH(FEET) = 685.00	MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 19.8 INCHES	
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.09	
ESTIMATED PIPE DIAMETER(INCH) = 30.00	NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 24.30	
PIPE TRAVEL TIME(MIN.) = 1.61	Tc(MIN.) = 16.86
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4015.00 = 1739.00 FEET.	

FLOW PROCESS FROM NODE 4015.00 TO NODE 4015.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====

MAINLINE Tc(MIN.) = 16.86					
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.570					
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	A	9.60	0.98	0.500	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500					
SUBAREA AREA(ACRES) = 9.60 SUBAREA RUNOFF(CFS) = 19.38					
EFFECTIVE AREA(ACRES) = 12.04 AREA-AVERAGED Fm(INCH/HR) = 0.49					
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.50					
TOTAL AREA(ACRES) = 12.0 PEAK FLOW RATE(CFS) = 24.30					

"5-7 DWELLINGS/ACRE"	A	11.46	0.98	0.500	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500					
SUBAREA AREA(ACRES) = 11.46 SUBAREA RUNOFF(CFS) = 21.48					
EFFECTIVE AREA(ACRES) = 23.50 AREA-AVERAGED Fm(INCH/HR) = 0.49					
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50					
TOTAL AREA(ACRES) = 23.5 PEAK FLOW RATE(CFS) = 44.05					

FLOW PROCESS FROM NODE 4015.00 TO NODE 4015.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====

MAINLINE Tc(MIN.) = 16.86					
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.570					
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	2.05	0.98	0.100	32
PUBLIC PARK	A	1.09	0.98	0.850	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.360					
SUBAREA AREA(ACRES) = 3.14 SUBAREA RUNOFF(CFS) = 6.27					
EFFECTIVE AREA(ACRES) = 26.64 AREA-AVERAGED Fm(INCH/HR) = 0.47					
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.48					
TOTAL AREA(ACRES) = 26.6 PEAK FLOW RATE(CFS) = 50.33					

FLOW PROCESS FROM NODE 4015.00 TO NODE 4016.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

ELEVATION DATA: UPSTREAM(FEET) = 766.00	DOWNSTREAM(FEET) = 761.50
FLOW LENGTH(FEET) = 344.00	MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.0 INCHES	
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.89	
ESTIMATED PIPE DIAMETER(INCH) = 33.00	NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 50.33	
PIPE TRAVEL TIME(MIN.) = 0.53	Tc(MIN.) = 17.38
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4016.00 = 2083.00 FEET.	

FLOW PROCESS FROM NODE 4016.00 TO NODE 4016.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====

MAINLINE Tc(MIN.) = 17.38					
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.523					
SUBAREA LOSS RATE DATA(AMC II):					
DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.89	0.98	0.100	32
PUBLIC PARK	A	1.26	0.98	0.850	32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98					

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.540
 SUBAREA AREA(ACRES) = 2.15 SUBAREA RUNOFF(CFS) = 3.87
 EFFECTIVE AREA(ACRES) = 28.79 AREA-AVERAGED Fm(INCH/HR) = 0.48
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.49
 TOTAL AREA(ACRES) = 28.8 PEAK FLOW RATE(CFS) = 53.06

 FLOW PROCESS FROM NODE 4016.00 TO NODE 4025.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	761.50	DOWNSTREAM(FEET) =	761.30
FLOW LENGTH(FEET) =	166.00	MANNING'S N =	0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 39.9 INCHES			
PIPE-FLOW VELOCITY(FEET/SEC.) =	4.46		
ESTIMATED PIPE DIAMETER(INCH) =	51.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	53.06		
PIPE TRAVEL TIME(MIN.) =	0.62	Tc(MIN.) =	18.00
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4025.00 = 2249.00 FEET.			

 FLOW PROCESS FROM NODE 4025.00 TO NODE 4025.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 18.00
 RAINFALL INTENSITY(INCH/HR) = 2.47
 AREA-AVERAGED Fm(INCH/HR) = 0.48
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.49
 EFFECTIVE STREAM AREA(ACRES) = 28.79
 TOTAL STREAM AREA(ACRES) = 28.79
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 53.06

 FLOW PROCESS FROM NODE 4020.00 TO NODE 4022.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 336.00
 ELEVATION DATA: UPSTREAM(FEET) = 776.00 DOWNSTREAM(FEET) = 774.30

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.473
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.238
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	1.23	0.98	0.500	32	11.47

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA RUNOFF(CFS) = 3.04
 TOTAL AREA(ACRES) = 1.23 PEAK FLOW RATE(CFS) = 3.04

 FLOW PROCESS FROM NODE 4022.00 TO NODE 4025.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) =	768.32	DOWNSTREAM(FEET) =	761.30
FLOW LENGTH(FEET) =	1317.00	MANNING'S N =	0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 24.000			
DEPTH OF FLOW IN 24.0 INCH PIPE IS 7.2 INCHES			
PIPE-FLOW VELOCITY(FEET/SEC.) =	3.86		
ESTIMATED PIPE DIAMETER(INCH) =	24.00	NUMBER OF PIPES =	1
PIPE-FLOW(CFS) =	3.04		
PIPE TRAVEL TIME(MIN.) =	5.68	Tc(MIN.) =	17.15
LONGEST FLOWPATH FROM NODE 4020.00 TO NODE 4025.00 = 1653.00 FEET.			

 FLOW PROCESS FROM NODE 4025.00 TO NODE 4025.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 17.15
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.544
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	12.42	0.98	0.500	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 12.42 SUBAREA RUNOFF(CFS) = 22.98
 EFFECTIVE AREA(ACRES) = 13.65 AREA-AVERAGED Fm(INCH/HR) = 0.49
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50
 TOTAL AREA(ACRES) = 13.6 PEAK FLOW RATE(CFS) = 25.26

 FLOW PROCESS FROM NODE 4025.00 TO NODE 4025.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 17.15
 RAINFALL INTENSITY(INCH/HR) = 2.54
 AREA-AVERAGED Fm(INCH/HR) = 0.49
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.50
 EFFECTIVE STREAM AREA(ACRES) = 13.65
 TOTAL STREAM AREA(ACRES) = 13.65
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 25.26

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	53.06	18.00	2.471	0.98(0.48)	0.49	28.8	4000.00
2	25.26	17.15	2.544	0.98(0.49)	0.50	13.6	4020.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	77.66	17.15	2.544	0.98(0.48)	0.49	41.1	4020.00
2	77.43	18.00	2.471	0.97(0.48)	0.49	42.4	4000.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 77.66 Tc(MIN.) = 17.15
EFFECTIVE AREA(ACRES) = 41.08 AREA-AVERAGED Fm(INCH/HR) = 0.48
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.49
TOTAL AREA(ACRES) = 42.4
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4025.00 = 2249.00 FEET.

FLOW PROCESS FROM NODE 4025.00 TO NODE 4026.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 761.30 DOWNSTREAM(FEET) = 745.80
FLOW LENGTH(FEET) = 1141.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 27.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.36
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 77.66
PIPE TRAVEL TIME(MIN.) = 1.54 Tc(MIN.) = 18.69
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4026.00 = 3390.00 FEET.

FLOW PROCESS FROM NODE 4026.00 TO NODE 4026.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 18.69
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.416
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	5.91	0.98	0.100	32
PUBLIC PARK	A	2.01	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.290
SUBAREA AREA(ACRES) = 7.92 SUBAREA RUNOFF(CFS) = 15.20
EFFECTIVE AREA(ACRES) = 49.00 AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.46

TOTAL AREA(ACRES) = 50.4 PEAK FLOW RATE(CFS) = 86.80

FLOW PROCESS FROM NODE 4026.00 TO NODE 4070.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 745.80 DOWNSTREAM(FEET) = 744.50
FLOW LENGTH(FEET) = 130.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 31.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.24
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 86.80
PIPE TRAVEL TIME(MIN.) = 0.19 Tc(MIN.) = 18.89
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4070.00 = 3520.00 FEET.

FLOW PROCESS FROM NODE 4070.00 TO NODE 4070.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 18.89
RAINFALL INTENSITY(INCH/HR) = 2.40
AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.98
AREA-AVERAGED Ap = 0.46
EFFECTIVE STREAM AREA(ACRES) = 49.00
TOTAL STREAM AREA(ACRES) = 50.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 86.80

FLOW PROCESS FROM NODE 4045.00 TO NODE 4050.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 444.00
ELEVATION DATA: UPSTREAM(FEET) = 772.80 DOWNSTREAM(FEET) = 770.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.273
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.110
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	4.70	0.98	0.500	32	12.27

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA RUNOFF(CFS) = 11.09
TOTAL AREA(ACRES) = 4.70 PEAK FLOW RATE(CFS) = 11.09

FLOW PROCESS FROM NODE 4050.00 TO NODE 4055.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 770.00 DOWNSTREAM ELEVATION(FEET) = 767.40
STREET LENGTH(FEET) = 517.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.018
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 20.51
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.55
HALFSTREET FLOOD WIDTH(FEET) = 21.52
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.37
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.30
STREET FLOW TRAVEL TIME(MIN.) = 3.64 Tc(MIN.) = 15.91
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.661

SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE" A 9.60 0.98 0.500 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 9.60 SUBAREA RUNOFF(CFS) = 18.78
EFFECTIVE AREA(ACRES) = 14.30 AREA-AVERAGED Fm(INCH/HR) = 0.49
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50
TOTAL AREA(ACRES) = 14.3 PEAK FLOW RATE(CFS) = 27.97

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.60 HALFSTREET FLOOD WIDTH(FEET) = 24.34
FLOW VELOCITY(FEET/SEC.) = 2.55 DEPTH*VELOCITY(FT*FT/SEC.) = 1.52
LONGEST FLOWPATH FROM NODE 4045.00 TO NODE 4055.00 = 961.00 FEET.

FLOW PROCESS FROM NODE 4055.00 TO NODE 4060.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 750.70 DOWNSTREAM(FEET) = 749.60
FLOW LENGTH(FEET) = 374.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.41
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 27.97
PIPE TRAVEL TIME(MIN.) = 1.15 Tc(MIN.) = 17.06
LONGEST FLOWPATH FROM NODE 4045.00 TO NODE 4060.00 = 1335.00 FEET.

FLOW PROCESS FROM NODE 4060.00 TO NODE 4060.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 17.06
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.552
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK B 0.51 0.75 0.850 56
RESIDENTIAL
"5-7 DWELLINGS/ACRE" B 4.06 0.75 0.500 56
PUBLIC PARK A 2.28 0.98 0.850 32
RESIDENTIAL
"5-7 DWELLINGS/ACRE" A 4.93 0.98 0.500 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.89
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.583
SUBAREA AREA(ACRES) = 11.78 SUBAREA RUNOFF(CFS) = 21.53
EFFECTIVE AREA(ACRES) = 26.08 AREA-AVERAGED Fm(INCH/HR) = 0.50
AREA-AVERAGED Fp(INCH/HR) = 0.94 AREA-AVERAGED Ap = 0.54
TOTAL AREA(ACRES) = 26.1 PEAK FLOW RATE(CFS) = 48.10

FLOW PROCESS FROM NODE 4060.00 TO NODE 4065.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 749.60 DOWNSTREAM(FEET) = 748.60
FLOW LENGTH(FEET) = 540.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 45.0 INCH PIPE IS 35.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.09
ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 48.10
PIPE TRAVEL TIME(MIN.) = 1.77 Tc(MIN.) = 18.83
LONGEST FLOWPATH FROM NODE 4045.00 TO NODE 4065.00 = 1875.00 FEET.

FLOW PROCESS FROM NODE 4065.00 TO NODE 4065.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 18.83
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.405
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK B 0.58 0.75 0.850 56
RESIDENTIAL
"5-7 DWELLINGS/ACRE" B 5.10 0.75 0.500 56

PUBLIC PARK A 0.74 0.98 0.850 32
 RESIDENTIAL
 "5-7 DWELLINGS/ACRE" A 4.07 0.98 0.500 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.85
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.544
 SUBAREA AREA(ACRES) = 10.49 SUBAREA RUNOFF(CFS) = 18.32
 EFFECTIVE AREA(ACRES) = 36.57 AREA-AVERAGED Fm(INCH/HR) = 0.49
 AREA-AVERAGED Fp(INCH/HR) = 0.91 AREA-AVERAGED Ap = 0.54
 TOTAL AREA(ACRES) = 36.6 PEAK FLOW RATE(CFS) = 62.98

 FLOW PROCESS FROM NODE 4065.00 TO NODE 4066.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 748.60 DOWNSTREAM(FEET) = 745.00
 FLOW LENGTH(FEET) = 585.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 8.72
 ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 62.98
 PIPE TRAVEL TIME(MIN.) = 1.12 Tc(MIN.) = 19.95
 LONGEST FLOWPATH FROM NODE 4045.00 TO NODE 4066.00 = 2460.00 FEET.

 FLOW PROCESS FROM NODE 4066.00 TO NODE 4066.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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MAINLINE Tc(MIN.) = 19.95
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.323
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	B	2.13	0.75	0.100	56
COMMERCIAL	A	3.00	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.88
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 5.13 SUBAREA RUNOFF(CFS) = 10.32
 EFFECTIVE AREA(ACRES) = 41.70 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.91 AREA-AVERAGED Ap = 0.49
 TOTAL AREA(ACRES) = 41.7 PEAK FLOW RATE(CFS) = 70.61

 FLOW PROCESS FROM NODE 4066.00 TO NODE 4070.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 745.00 DOWNSTREAM(FEET) = 744.50
 FLOW LENGTH(FEET) = 130.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 45.0 INCH PIPE IS 36.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.34
 ESTIMATED PIPE DIAMETER(INCH) = 45.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 70.61
 PIPE TRAVEL TIME(MIN.) = 0.30 Tc(MIN.) = 20.24
 LONGEST FLOWPATH FROM NODE 4045.00 TO NODE 4070.00 = 2590.00 FEET.

 FLOW PROCESS FROM NODE 4070.00 TO NODE 4070.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 20.24
 RAINFALL INTENSITY(INCH/HR) = 2.30
 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.91
 AREA-AVERAGED Ap = 0.49
 EFFECTIVE STREAM AREA(ACRES) = 41.70
 TOTAL STREAM AREA(ACRES) = 41.70
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 70.61

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	86.80	18.89	2.401	0.98(0.45)	0.46	49.0	4020.00
1	86.28	19.74	2.338	0.97(0.45)	0.46	50.4	4000.00
2	70.61	20.24	2.303	0.91(0.44)	0.49	41.7	4045.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	156.14	18.89	2.401	0.95(0.45)	0.47	87.9	4020.00
2	156.43	19.74	2.338	0.95(0.45)	0.47	91.0	4000.00
3	155.28	20.24	2.303	0.95(0.45)	0.47	92.1	4045.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 156.43 Tc(MIN.) = 19.74
 EFFECTIVE AREA(ACRES) = 91.02 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.95 AREA-AVERAGED Ap = 0.47
 TOTAL AREA(ACRES) = 92.1
 LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4070.00 = 3520.00 FEET.

 FLOW PROCESS FROM NODE 4070.00 TO NODE 4071.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 744.50 DOWNSTREAM(FEET) = 740.50
 FLOW LENGTH(FEET) = 566.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 54.0 INCH PIPE IS 44.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.24

ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 156.43
 PIPE TRAVEL TIME(MIN.) = 0.84 Tc(MIN.) = 20.57
 LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4071.00 = 4086.00 FEET.

 FLOW PROCESS FROM NODE 4071.00 TO NODE 4071.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 20.57
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.281
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.85	0.98	0.100	32
PUBLIC PARK	A	1.45	0.98	0.850	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.573
 SUBAREA AREA(ACRES) = 2.30 SUBAREA RUNOFF(CFS) = 3.56
 EFFECTIVE AREA(ACRES) = 93.32 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.95 AREA-AVERAGED Ap = 0.47
 TOTAL AREA(ACRES) = 94.4 PEAK FLOW RATE(CFS) = 156.43
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 4071.00 TO NODE 4075.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 740.50 DOWNSTREAM(FEET) = 740.00
 FLOW LENGTH(FEET) = 125.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 63.0 INCH PIPE IS 45.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.27
 ESTIMATED PIPE DIAMETER(INCH) = 63.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 156.43
 PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 20.80
 LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4075.00 = 4211.00 FEET.

 FLOW PROCESS FROM NODE 4075.00 TO NODE 4075.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 20.80
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.266
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	B	2.41	0.75	0.850	56
PUBLIC PARK	A	12.00	0.98	0.850	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.94
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
 SUBAREA AREA(ACRES) = 14.41 SUBAREA RUNOFF(CFS) = 19.06

EFFECTIVE AREA(ACRES) = 107.73 AREA-AVERAGED Fm(INCH/HR) = 0.49
 AREA-AVERAGED Fp(INCH/HR) = 0.94 AREA-AVERAGED Ap = 0.52
 TOTAL AREA(ACRES) = 108.8 PEAK FLOW RATE(CFS) = 171.70

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	172.01	19.95	2.323	0.94(0.50)	0.53	104.6	4020.00
2	171.70	20.80	2.266	0.94(0.49)	0.52	107.7	4000.00
3	170.20	21.31	2.233	0.94(0.49)	0.52	108.8	4045.00

NEW PEAK FLOW DATA ARE:
 PEAK FLOW RATE(CFS) = 172.01 Tc(MIN.) = 19.95
 AREA-AVERAGED Fm(INCH/HR) = 0.50 AREA-AVERAGED Fp(INCH/HR) = 0.94
 AREA-AVERAGED Ap = 0.53 EFFECTIVE AREA(ACRES) = 104.62

 FLOW PROCESS FROM NODE 4075.00 TO NODE 4076.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 740.00 DOWNSTREAM(FEET) = 729.80
 FLOW LENGTH(FEET) = 855.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 51.0 INCH PIPE IS 41.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 14.06
 ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 172.01
 PIPE TRAVEL TIME(MIN.) = 1.01 Tc(MIN.) = 20.97
 LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4076.00 = 5066.00 FEET.

 FLOW PROCESS FROM NODE 4076.00 TO NODE 4076.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 20.97
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.255
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	1.75	0.98	0.100	32
PUBLIC PARK	A	2.82	0.98	0.850	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.563
 SUBAREA AREA(ACRES) = 4.57 SUBAREA RUNOFF(CFS) = 7.02
 EFFECTIVE AREA(ACRES) = 109.19 AREA-AVERAGED Fm(INCH/HR) = 0.50
 AREA-AVERAGED Fp(INCH/HR) = 0.95 AREA-AVERAGED Ap = 0.53
 TOTAL AREA(ACRES) = 113.3 PEAK FLOW RATE(CFS) = 172.62

 FLOW PROCESS FROM NODE 4076.00 TO NODE 4120.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 729.80 DOWNSTREAM(FEET) = 729.30
 FLOW LENGTH(FEET) = 176.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 69.0 INCH PIPE IS 51.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 8.33
 ESTIMATED PIPE DIAMETER(INCH) = 69.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 172.62
 PIPE TRAVEL TIME(MIN.) = 0.35 Tc(MIN.) = 21.32
 LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4120.00 = 5242.00 FEET.

 FLOW PROCESS FROM NODE 4120.00 TO NODE 4120.00 IS CODE = 10

 >>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
 =====

 FLOW PROCESS FROM NODE 1.00 TO NODE 2.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 425.00
 ELEVATION DATA: UPSTREAM(FEET) = 795.00 DOWNSTREAM(FEET) = 790.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.276
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.272
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
SCHOOL	A	4.60	0.98	0.600	32	11.28

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600
 SUBAREA RUNOFF(CFS) = 11.12
 TOTAL AREA(ACRES) = 4.60 PEAK FLOW RATE(CFS) = 11.12

 FLOW PROCESS FROM NODE 2.00 TO NODE 3.00 IS CODE = 51

 >>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 790.00 DOWNSTREAM(FEET) = 785.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 400.00 CHANNEL SLOPE = 0.0125
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 5.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.069
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
SCHOOL	A	4.50	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 16.15
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.25

AVERAGE FLOW DEPTH(FEET) = 0.43 TRAVEL TIME(MIN.) = 1.27
 Tc(MIN.) = 12.54
 SUBAREA AREA(ACRES) = 4.50 SUBAREA RUNOFF(CFS) = 10.06
 EFFECTIVE AREA(ACRES) = 9.10 AREA-AVERAGED Fm(INCH/HR) = 0.58
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60
 TOTAL AREA(ACRES) = 9.1 PEAK FLOW RATE(CFS) = 20.34

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.49 FLOW VELOCITY(FEET/SEC.) = 5.64
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 3.00 = 825.00 FEET.

 FLOW PROCESS FROM NODE 3.00 TO NODE 4039.00 IS CODE = 51

 >>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 785.00 DOWNSTREAM(FEET) = 762.36
 CHANNEL LENGTH THRU SUBAREA(FEET) = 900.00 CHANNEL SLOPE = 0.0252
 CHANNEL BASE(FEET) = 5.00 "Z" FACTOR = 5.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.853
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
SCHOOL	A	26.30	0.98	0.600	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.600
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 47.21
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 9.23
 AVERAGE FLOW DEPTH(FEET) = 0.63 TRAVEL TIME(MIN.) = 1.63
 Tc(MIN.) = 14.17
 SUBAREA AREA(ACRES) = 26.30 SUBAREA RUNOFF(CFS) = 53.67
 EFFECTIVE AREA(ACRES) = 35.40 AREA-AVERAGED Fm(INCH/HR) = 0.59
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.60
 TOTAL AREA(ACRES) = 35.4 PEAK FLOW RATE(CFS) = 72.25

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.78 FLOW VELOCITY(FEET/SEC.) = 10.39
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4039.00 = 1725.00 FEET.

 FLOW PROCESS FROM NODE 4039.00 TO NODE 4039.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 14.17
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.853
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
MOBILE HOME PARK	A	6.30	0.98	0.250	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.250
 SUBAREA AREA(ACRES) = 6.30 SUBAREA RUNOFF(CFS) = 14.79

EFFECTIVE AREA(ACRES) = 41.70 AREA-AVERAGED Fm(INCH/HR) = 0.53
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.55
 TOTAL AREA(ACRES) = 41.7 PEAK FLOW RATE(CFS) = 87.04

 FLOW PROCESS FROM NODE 4039.00 TO NODE 4040.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 762.36 DOWNSTREAM(FEET) = 760.10
 FLOW LENGTH(FEET) = 987.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 54.0 INCH PIPE IS 43.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.40
 ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 87.04
 PIPE TRAVEL TIME(MIN.) = 2.57 Tc(MIN.) = 16.74
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4040.00 = 2712.00 FEET.

 FLOW PROCESS FROM NODE 4040.00 TO NODE 4040.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 16.74
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.581
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	12.63	0.98	0.500	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 12.63 SUBAREA RUNOFF(CFS) = 23.80
 EFFECTIVE AREA(ACRES) = 54.33 AREA-AVERAGED Fm(INCH/HR) = 0.52
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.54
 TOTAL AREA(ACRES) = 54.3 PEAK FLOW RATE(CFS) = 100.64

 FLOW PROCESS FROM NODE 4040.00 TO NODE 4080.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 760.10 DOWNSTREAM(FEET) = 749.00
 FLOW LENGTH(FEET) = 407.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 28.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 16.84
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 100.64
 PIPE TRAVEL TIME(MIN.) = 0.40 Tc(MIN.) = 17.14
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4080.00 = 3119.00 FEET.

 FLOW PROCESS FROM NODE 4080.00 TO NODE 4080.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 17.14
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.544
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	10.57	0.98	0.500	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 10.57 SUBAREA RUNOFF(CFS) = 19.57
 EFFECTIVE AREA(ACRES) = 64.90 AREA-AVERAGED Fm(INCH/HR) = 0.52
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.53
 TOTAL AREA(ACRES) = 64.9 PEAK FLOW RATE(CFS) = 118.42

 FLOW PROCESS FROM NODE 4080.00 TO NODE 4082.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 749.00 DOWNSTREAM(FEET) = 748.50
 FLOW LENGTH(FEET) = 345.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 66.0 INCH PIPE IS 52.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 5.82
 ESTIMATED PIPE DIAMETER(INCH) = 66.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 118.42
 PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 18.13
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4082.00 = 3464.00 FEET.

 FLOW PROCESS FROM NODE 4082.00 TO NODE 4082.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 18.13
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.460
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	1.87	0.98	0.100	32
PUBLIC PARK	A	1.27	0.98	0.850	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.403
 SUBAREA AREA(ACRES) = 3.14 SUBAREA RUNOFF(CFS) = 5.84
 EFFECTIVE AREA(ACRES) = 68.04 AREA-AVERAGED Fm(INCH/HR) = 0.51
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.52
 TOTAL AREA(ACRES) = 68.0 PEAK FLOW RATE(CFS) = 119.35

 FLOW PROCESS FROM NODE 4082.00 TO NODE 4120.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

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>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 748.50 DOWNSTREAM(FEET) = 729.30
FLOW LENGTH(FEET) = 2619.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 36.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.89
ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 119.35
PIPE TRAVEL TIME(MIN.) = 4.01 Tc(MIN.) = 22.14
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4120.00 = 6083.00 FEET.

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*****
FLOW PROCESS FROM NODE 4120.00 TO NODE 4120.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 22.14
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.182
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE"  A      11.55   0.98   0.200  32
PUBLIC PARK           A      10.51   0.98   0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.510
SUBAREA AREA(ACRES) = 22.06 SUBAREA RUNOFF(CFS) = 33.47
EFFECTIVE AREA(ACRES) = 90.10 AREA-AVERAGED Fm(INCH/HR) = 0.51
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.52
TOTAL AREA(ACRES) = 90.1 PEAK FLOW RATE(CFS) = 135.80

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*****
FLOW PROCESS FROM NODE 4120.00 TO NODE 4120.00 IS CODE = 11
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>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
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** MAIN STREAM CONFLUENCE DATA **
STREAM   Q      Tc  Intensity  Fp(Fm)   Ap   Ae  HEADWATER
NUMBER  (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES)  NODE
1      135.80 22.14 2.182 0.98(0.51) 0.52 90.1 1.00
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4120.00 = 6083.00 FEET.

```

```

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM   Q      Tc  Intensity  Fp(Fm)   Ap   Ae  HEADWATER
NUMBER  (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES)  NODE
1      172.62 21.32 2.233 0.95(0.50) 0.53 109.2 4020.00
2      172.32 22.17 2.181 0.95(0.50) 0.53 112.3 4000.00
3      170.86 22.68 2.151 0.95(0.50) 0.53 113.3 4045.00
LONGEST FLOWPATH FROM NODE 4000.00 TO NODE 4120.00 = 5242.00 FEET.

```

```

** PEAK FLOW RATE TABLE **
STREAM   Q      Tc  Intensity  Fp(Fm)   Ap   Ae  HEADWATER
NUMBER  (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES)  NODE
1      307.30 21.32 2.233 0.96(0.50) 0.52 195.9 4020.00

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2      308.13 22.14 2.182 0.96(0.50) 0.52 202.3 1.00
3      308.00 22.17 2.181 0.96(0.50) 0.52 202.4 4000.00
4      304.14 22.68 2.151 0.96(0.50) 0.52 203.4 4045.00
TOTAL AREA(ACRES) = 203.4

```

```

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 308.13 Tc(MIN.) = 22.141
EFFECTIVE AREA(ACRES) = 202.31 AREA-AVERAGED Fm(INCH/HR) = 0.50
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.52
TOTAL AREA(ACRES) = 203.4
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4120.00 = 6083.00 FEET.

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*****
FLOW PROCESS FROM NODE 4120.00 TO NODE 4120.00 IS CODE = 12
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>>>>CLEAR MEMORY BANK # 1 <<<<<
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*****
FLOW PROCESS FROM NODE 4120.00 TO NODE 4125.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====

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ELEVATION DATA: UPSTREAM(FEET) = 729.30 DOWNSTREAM(FEET) = 720.30
FLOW LENGTH(FEET) = 790.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 66.0 INCH PIPE IS 49.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.21
ESTIMATED PIPE DIAMETER(INCH) = 66.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 308.13
PIPE TRAVEL TIME(MIN.) = 0.81 Tc(MIN.) = 22.95
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4125.00 = 6873.00 FEET.

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*****
FLOW PROCESS FROM NODE 4125.00 TO NODE 4125.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
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MAINLINE Tc(MIN.) = 22.95
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.136
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE"  A      16.30   0.98   0.200  32
PUBLIC PARK           A      3.36   0.98   0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.311
SUBAREA AREA(ACRES) = 19.66 SUBAREA RUNOFF(CFS) = 32.42
EFFECTIVE AREA(ACRES) = 221.97 AREA-AVERAGED Fm(INCH/HR) = 0.48
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.50
TOTAL AREA(ACRES) = 223.1 PEAK FLOW RATE(CFS) = 329.92

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*****
FLOW PROCESS FROM NODE 4125.00 TO NODE 4126.00 IS CODE = 31

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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 720.30 DOWNSTREAM(FEET) = 714.40
FLOW LENGTH(FEET) = 683.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 69.0 INCH PIPE IS 55.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.63
ESTIMATED PIPE DIAMETER(INCH) = 69.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 329.92
PIPE TRAVEL TIME(MIN.) = 0.78 Tc(MIN.) = 23.73
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4126.00 = 7556.00 FEET.

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*****
FLOW PROCESS FROM NODE 4126.00 TO NODE 4126.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 23.73
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.094
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       2.69   0.98  0.100  32
PUBLIC PARK         A       4.25   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.559
SUBAREA AREA(ACRES) = 6.94 SUBAREA RUNOFF(CFS) = 9.67
EFFECTIVE AREA(ACRES) = 228.91 AREA-AVERAGED Fm(INCH/HR) = 0.49
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.51
TOTAL AREA(ACRES) = 230.0 PEAK FLOW RATE(CFS) = 331.15

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*****
FLOW PROCESS FROM NODE 4126.00 TO NODE 4145.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 714.40 DOWNSTREAM(FEET) = 713.90
FLOW LENGTH(FEET) = 120.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 81.0 INCH PIPE IS 62.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.27
ESTIMATED PIPE DIAMETER(INCH) = 81.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 331.15
PIPE TRAVEL TIME(MIN.) = 0.18 Tc(MIN.) = 23.91
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4145.00 = 7676.00 FEET.

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*****
FLOW PROCESS FROM NODE 4145.00 TO NODE 4145.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 23.91
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.084
SUBAREA LOSS RATE DATA(AMC II):

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DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       26.70   0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 26.70 SUBAREA RUNOFF(CFS) = 47.74
EFFECTIVE AREA(ACRES) = 255.61 AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 256.7 PEAK FLOW RATE(CFS) = 376.96

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** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	377.70	23.09	2.128	0.96(0.44)	0.46	249.2	4020.00
2	376.96	23.91	2.084	0.96(0.45)	0.46	255.6	1.00
3	376.80	23.93	2.083	0.96(0.45)	0.46	255.7	4000.00
4	372.25	24.45	2.057	0.96(0.45)	0.46	256.7	4045.00

```

NEW PEAK FLOW DATA ARE:
PEAK FLOW RATE(CFS) = 377.70 Tc(MIN.) = 23.09
AREA-AVERAGED Fm(INCH/HR) = 0.44 AREA-AVERAGED Fp(INCH/HR) = 0.96
AREA-AVERAGED Ap = 0.46 EFFECTIVE AREA(ACRES) = 249.24

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*****
FLOW PROCESS FROM NODE 4145.00 TO NODE 4145.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 23.09
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.128
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE" A       2.09   0.98  0.200  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA(ACRES) = 2.09 SUBAREA RUNOFF(CFS) = 3.64
EFFECTIVE AREA(ACRES) = 251.33 AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 258.8 PEAK FLOW RATE(CFS) = 381.34

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*****
FLOW PROCESS FROM NODE 4145.00 TO NODE 4146.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====

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ELEVATION DATA: UPSTREAM(FEET) = 713.90 DOWNSTREAM(FEET) = 712.50
FLOW LENGTH(FEET) = 551.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 93.0 INCH PIPE IS 72.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.67
ESTIMATED PIPE DIAMETER(INCH) = 93.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 381.34
PIPE TRAVEL TIME(MIN.) = 0.95 Tc(MIN.) = 24.04
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4146.00 = 8227.00 FEET.

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*****
FLOW PROCESS FROM NODE 4146.00 TO NODE 4146.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 24.04
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.077
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       1.06   0.98  0.100  32
PUBLIC PARK         A       1.62   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.553
SUBAREA AREA(ACRES) = 2.68   SUBAREA RUNOFF(CFS) = 3.71
EFFECTIVE AREA(ACRES) = 254.01   AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.96   AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 261.5   PEAK FLOW RATE(CFS) = 381.34
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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FLOW PROCESS FROM NODE 4146.00 TO NODE 4170.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 712.50   DOWNSTREAM(FEET) = 711.41
FLOW LENGTH(FEET) = 128.00   MANNING'S N = 0.013
DEPTH OF FLOW IN 75.0 INCH PIPE IS 56.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.29
ESTIMATED PIPE DIAMETER(INCH) = 75.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 381.34
PIPE TRAVEL TIME(MIN.) = 0.14   Tc(MIN.) = 24.18
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4170.00 = 8355.00 FEET.

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*****
FLOW PROCESS FROM NODE 4170.00 TO NODE 4170.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 24.18
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.070
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       6.26   0.98  0.100  32
PUBLIC PARK         A       4.13   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.398
SUBAREA AREA(ACRES) = 10.39   SUBAREA RUNOFF(CFS) = 15.73
EFFECTIVE AREA(ACRES) = 264.40   AREA-AVERAGED Fm(INCH/HR) = 0.44
AREA-AVERAGED Fp(INCH/HR) = 0.96   AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 271.9   PEAK FLOW RATE(CFS) = 387.63

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*****
FLOW PROCESS FROM NODE 4170.00 TO NODE 4170.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 24.18
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.070
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       19.56   0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 19.56   SUBAREA RUNOFF(CFS) = 34.73
EFFECTIVE AREA(ACRES) = 283.96   AREA-AVERAGED Fm(INCH/HR) = 0.42
AREA-AVERAGED Fp(INCH/HR) = 0.96   AREA-AVERAGED Ap = 0.43
TOTAL AREA(ACRES) = 291.5   PEAK FLOW RATE(CFS) = 422.35

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*****
FLOW PROCESS FROM NODE 4170.00 TO NODE 4171.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 711.41   DOWNSTREAM(FEET) = 700.00
FLOW LENGTH(FEET) = 1251.00   MANNING'S N = 0.013
DEPTH OF FLOW IN 75.0 INCH PIPE IS 60.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.90
ESTIMATED PIPE DIAMETER(INCH) = 75.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 422.35
PIPE TRAVEL TIME(MIN.) = 1.31   Tc(MIN.) = 25.49
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4171.00 = 9606.00 FEET.

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*****
FLOW PROCESS FROM NODE 4171.00 TO NODE 4171.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 25.49
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.006
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       3.47   0.98  0.100  32
PUBLIC PARK         A       3.36   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.469
SUBAREA AREA(ACRES) = 6.83   SUBAREA RUNOFF(CFS) = 9.52
EFFECTIVE AREA(ACRES) = 290.79   AREA-AVERAGED Fm(INCH/HR) = 0.42
AREA-AVERAGED Fp(INCH/HR) = 0.96   AREA-AVERAGED Ap = 0.44
TOTAL AREA(ACRES) = 298.3   PEAK FLOW RATE(CFS) = 422.35
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 4171.00 TO NODE 4171.00 IS CODE = 81

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-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 25.49
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.006
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A      35.70  0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 35.70 SUBAREA RUNOFF(CFS) = 61.31
EFFECTIVE AREA(ACRES) = 326.49 AREA-AVERAGED Fm(INCH/HR) = 0.38
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.40
TOTAL AREA(ACRES) = 334.0 PEAK FLOW RATE(CFS) = 476.68

*****
FLOW PROCESS FROM NODE 4171.00 TO NODE 4200.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 700.00 DOWNSTREAM(FEET) = 699.50
FLOW LENGTH(FEET) = 148.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 96.0 INCH PIPE IS 74.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.38
ESTIMATED PIPE DIAMETER(INCH) = 96.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 476.68
PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 25.71
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4200.00 = 9754.00 FEET.

*****
FLOW PROCESS FROM NODE 4200.00 TO NODE 4200.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 25.71
RAINFALL INTENSITY(INCH/HR) = 2.00
AREA-AVERAGED Fm(INCH/HR) = 0.38
AREA-AVERAGED Fp(INCH/HR) = 0.96
AREA-AVERAGED Ap = 0.40
EFFECTIVE STREAM AREA(ACRES) = 326.49
TOTAL STREAM AREA(ACRES) = 333.99
PEAK FLOW RATE(CFS) AT CONFLUENCE = 476.68

*****
FLOW PROCESS FROM NODE 4175.00 TO NODE 4180.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 561.00
ELEVATION DATA: UPSTREAM(FEET) = 723.50 DOWNSTREAM(FEET) = 720.00

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.249
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.276
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS   Tc
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"11+ DWELLINGS/ACRE"   A      6.59  0.98  0.200  32  11.25
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA RUNOFF(CFS) = 18.28
TOTAL AREA(ACRES) = 6.59 PEAK FLOW RATE(CFS) = 18.28

*****
FLOW PROCESS FROM NODE 4180.00 TO NODE 4200.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 720.00 DOWNSTREAM(FEET) = 699.00
FLOW LENGTH(FEET) = 770.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 24.000
DEPTH OF FLOW IN 24.0 INCH PIPE IS 12.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.37
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 18.28
PIPE TRAVEL TIME(MIN.) = 1.13 Tc(MIN.) = 12.38
LONGEST FLOWPATH FROM NODE 4175.00 TO NODE 4200.00 = 1331.00 FEET.

*****
FLOW PROCESS FROM NODE 4200.00 TO NODE 4200.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 12.38
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.094
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK          A      1.10  0.98  0.850  32
RESIDENTIAL
"5-7 DWELLINGS/ACRE"   A      5.15  0.98  0.500  32
RESIDENTIAL
"11+ DWELLINGS/ACRE"   A      5.38  0.98  0.200  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.394
SUBAREA AREA(ACRES) = 11.63 SUBAREA RUNOFF(CFS) = 28.36
EFFECTIVE AREA(ACRES) = 18.22 AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.32
TOTAL AREA(ACRES) = 18.2 PEAK FLOW RATE(CFS) = 45.55

*****
FLOW PROCESS FROM NODE 4200.00 TO NODE 4200.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4205.00 = 10253.00 FEET.

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 12.38
 RAINFALL INTENSITY(INCH/HR) = 3.09
 AREA-AVERAGED Fm(INCH/HR) = 0.32
 AREA-AVERAGED Fp(INCH/HR) = 0.97
 AREA-AVERAGED Ap = 0.32
 EFFECTIVE STREAM AREA(ACRES) = 18.22
 TOTAL STREAM AREA(ACRES) = 18.22
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 45.55

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	476.68	25.71	1.996	0.96(0.38)	0.40	326.5	4020.00
1	474.07	26.53	1.958	0.96(0.39)	0.40	332.9	1.00
1	473.86	26.55	1.957	0.96(0.39)	0.40	332.9	4000.00
1	468.46	27.07	1.935	0.96(0.39)	0.40	334.0	4045.00
2	45.55	12.38	3.094	0.97(0.32)	0.32	18.2	4175.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	431.45	12.38	3.094	0.96(0.38)	0.39	175.4	4175.00
2	504.22	25.71	1.996	0.96(0.38)	0.39	344.7	4020.00
3	501.00	26.53	1.958	0.96(0.38)	0.40	351.1	1.00
4	500.78	26.55	1.957	0.96(0.38)	0.40	351.2	4000.00
5	495.00	27.07	1.935	0.96(0.38)	0.40	352.2	4045.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 504.22 Tc(MIN.) = 25.71
 EFFECTIVE AREA(ACRES) = 344.71 AREA-AVERAGED Fm(INCH/HR) = 0.38
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.39
 TOTAL AREA(ACRES) = 352.2
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4200.00 = 9754.00 FEET.

=====

FLOW PROCESS FROM NODE 4200.00 TO NODE 4205.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 699.00 DOWNSTREAM(FEET) = 697.00
 FLOW LENGTH(FEET) = 499.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 96.0 INCH PIPE IS 72.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 12.37
 ESTIMATED PIPE DIAMETER(INCH) = 96.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 504.22
 PIPE TRAVEL TIME(MIN.) = 0.67 Tc(MIN.) = 26.38

FLOW PROCESS FROM NODE 4205.00 TO NODE 4205.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 26.38
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.965
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" A 7.04 0.98 0.200 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 7.04 SUBAREA RUNOFF(CFS) = 11.21
 EFFECTIVE AREA(ACRES) = 351.75 AREA-AVERAGED Fm(INCH/HR) = 0.38
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.39
 TOTAL AREA(ACRES) = 359.3 PEAK FLOW RATE(CFS) = 504.22
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 4205.00 TO NODE 4205.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 26.38
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.965
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 0.80 0.98 0.100 32
 PUBLIC PARK A 1.36 0.98 0.850 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.572
 SUBAREA AREA(ACRES) = 2.16 SUBAREA RUNOFF(CFS) = 2.74
 EFFECTIVE AREA(ACRES) = 353.91 AREA-AVERAGED Fm(INCH/HR) = 0.38
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.39
 TOTAL AREA(ACRES) = 361.4 PEAK FLOW RATE(CFS) = 505.66

FLOW PROCESS FROM NODE 4205.00 TO NODE 4206.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 697.00 DOWNSTREAM(FEET) = 692.50
 FLOW LENGTH(FEET) = 1132.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 96.0 INCH PIPE IS 73.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 12.32
 ESTIMATED PIPE DIAMETER(INCH) = 96.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 505.66
 PIPE TRAVEL TIME(MIN.) = 1.53 Tc(MIN.) = 27.91
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4206.00 = 11385.00 FEET.

```

*****
FLOW PROCESS FROM NODE 4206.00 TO NODE 4206.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 27.91
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.899
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp      Ap      SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A        1.30    0.98    0.100   32
PUBLIC PARK         A        2.16    0.98    0.850   32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.568
SUBAREA AREA(ACRES) = 3.46 SUBAREA RUNOFF(CFS) = 4.19
EFFECTIVE AREA(ACRES) = 357.37 AREA-AVERAGED Fm(INCH/HR) = 0.38
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.39
TOTAL AREA(ACRES) = 364.9 PEAK FLOW RATE(CFS) = 505.66
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 4206.00 TO NODE 4235.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 692.50 DOWNSTREAM(FEET) = 692.00
FLOW LENGTH(FEET) = 200.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 102.0 INCH PIPE IS 83.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.22
ESTIMATED PIPE DIAMETER(INCH) = 102.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 505.66
PIPE TRAVEL TIME(MIN.) = 0.33 Tc(MIN.) = 28.23
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4235.00 = 11585.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 4235.00 TO NODE 4235.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 28.23
RAINFALL INTENSITY(INCH/HR) = 1.89
AREA-AVERAGED Fm(INCH/HR) = 0.38
AREA-AVERAGED Fp(INCH/HR) = 0.96
AREA-AVERAGED Ap = 0.39
EFFECTIVE STREAM AREA(ACRES) = 357.37
TOTAL STREAM AREA(ACRES) = 364.87
PEAK FLOW RATE(CFS) AT CONFLUENCE = 505.66

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*****
FLOW PROCESS FROM NODE 4210.00 TO NODE 4215.00 IS CODE = 21
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 945.00
ELEVATION DATA: UPSTREAM(FEET) = 721.00 DOWNSTREAM(FEET) = 711.10

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 15.887
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.664
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp      Ap      SCS   Tc
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
SCHOOL              A        9.77    0.98    0.600   32  15.89
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600
SUBAREA RUNOFF(CFS) = 18.28
TOTAL AREA(ACRES) = 9.77 PEAK FLOW RATE(CFS) = 18.28

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*****
FLOW PROCESS FROM NODE 4215.00 TO NODE 4235.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 702.50 DOWNSTREAM(FEET) = 692.00
FLOW LENGTH(FEET) = 1233.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.15
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 18.28
PIPE TRAVEL TIME(MIN.) = 2.87 Tc(MIN.) = 18.76
LONGEST FLOWPATH FROM NODE 4210.00 TO NODE 4235.00 = 2178.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 4235.00 TO NODE 4235.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 18.76
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.411
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp      Ap      SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
SCHOOL              A        1.82    0.98    0.600   32
RESIDENTIAL
"11+ DWELLINGS/ACRE" A        19.00    0.98    0.200   32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.235
SUBAREA AREA(ACRES) = 20.82 SUBAREA RUNOFF(CFS) = 40.88
EFFECTIVE AREA(ACRES) = 30.59 AREA-AVERAGED Fm(INCH/HR) = 0.34
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.35
TOTAL AREA(ACRES) = 30.6 PEAK FLOW RATE(CFS) = 56.93

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*****
FLOW PROCESS FROM NODE 4235.00 TO NODE 4235.00 IS CODE = 81

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 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 18.76
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.411
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" A 35.18 0.98 0.200 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 35.18 SUBAREA RUNOFF(CFS) = 70.15
 EFFECTIVE AREA(ACRES) = 65.77 AREA-AVERAGED Fm(INCH/HR) = 0.26
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.27
 TOTAL AREA(ACRES) = 65.8 PEAK FLOW RATE(CFS) = 127.09

 FLOW PROCESS FROM NODE 4235.00 TO NODE 4235.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 18.76
 RAINFALL INTENSITY(INCH/HR) = 2.41
 AREA-AVERAGED Fm(INCH/HR) = 0.26
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.27
 EFFECTIVE STREAM AREA(ACRES) = 65.77
 TOTAL STREAM AREA(ACRES) = 65.77
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 127.09

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	435.62	15.00	2.756	0.96(0.38)	0.39	188.1	4175.00
1	505.66	28.23	1.886	0.96(0.38)	0.39	357.4	4020.00
1	502.38	29.07	1.854	0.96(0.38)	0.40	363.7	1.00
1	502.16	29.09	1.853	0.96(0.38)	0.40	363.8	4000.00
1	496.53	29.63	1.832	0.96(0.38)	0.40	364.9	4045.00
2	127.09	18.76	2.411	0.98(0.26)	0.27	65.8	4210.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	553.63	15.00	2.756	0.97(0.35)	0.36	240.7	4175.00
2	582.58	18.76	2.411	0.97(0.35)	0.36	301.9	4210.00
3	601.70	28.23	1.886	0.96(0.36)	0.37	423.1	4020.00
4	596.49	29.07	1.854	0.96(0.36)	0.38	429.5	1.00
5	596.22	29.09	1.853	0.96(0.36)	0.38	429.6	4000.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 601.70 Tc(MIN.) = 28.23
 EFFECTIVE AREA(ACRES) = 423.14 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.37
 TOTAL AREA(ACRES) = 430.6
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4235.00 = 11585.00 FEET.

 FLOW PROCESS FROM NODE 4235.00 TO NODE 4236.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 692.00 DOWNSTREAM(FEET) = 684.00
 FLOW LENGTH(FEET) = 1134.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 90.0 INCH PIPE IS 72.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 15.79
 ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 601.70
 PIPE TRAVEL TIME(MIN.) = 1.20 Tc(MIN.) = 29.43
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4236.00 = 12719.00 FEET.

 FLOW PROCESS FROM NODE 4236.00 TO NODE 4236.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 29.43
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.840
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 6.25 0.98 0.100 32
 PUBLIC PARK A 0.85 0.98 0.850 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.190
 SUBAREA AREA(ACRES) = 7.10 SUBAREA RUNOFF(CFS) = 10.57
 EFFECTIVE AREA(ACRES) = 430.24 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.37
 TOTAL AREA(ACRES) = 437.7 PEAK FLOW RATE(CFS) = 601.70
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 4236.00 TO NODE 4236.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 29.43
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.840
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL

"11+ DWELLINGS/ACRE" A 19.00 0.98 0.200 32
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" C 16.49 0.57 0.200 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.78
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 35.49 SUBAREA RUNOFF(CFS) = 53.75
 EFFECTIVE AREA(ACRES) = 465.73 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.36
 TOTAL AREA(ACRES) = 473.2 PEAK FLOW RATE(CFS) = 627.47

FLOW PROCESS FROM NODE 4236.00 TO NODE 4280.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 684.00 DOWNSTREAM(FEET) = 683.30
 FLOW LENGTH(FEET) = 130.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 96.0 INCH PIPE IS 77.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 14.40
 ESTIMATED PIPE DIAMETER(INCH) = 96.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 627.47
 PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 29.58
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4280.00 = 12849.00 FEET.

FLOW PROCESS FROM NODE 4280.00 TO NODE 4280.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 29.58
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.834
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" A 2.50 0.98 0.200 32
 PUBLIC PARK A 32.54 0.98 0.850 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.804
 SUBAREA AREA(ACRES) = 35.04 SUBAREA RUNOFF(CFS) = 33.13
 EFFECTIVE AREA(ACRES) = 500.77 AREA-AVERAGED Fm(INCH/HR) = 0.37
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.39
 TOTAL AREA(ACRES) = 508.3 PEAK FLOW RATE(CFS) = 658.24

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	642.33	16.38	2.615	0.96(0.37)	0.39	318.3	4175.00
2	663.37	20.11	2.312	0.96(0.37)	0.39	379.5	4210.00
3	658.24	29.58	1.834	0.96(0.37)	0.39	500.8	4020.00
4	652.13	30.42	1.804	0.96(0.38)	0.39	507.1	1.00
5	651.85	30.44	1.803	0.96(0.38)	0.39	507.2	4000.00
6	644.43	30.98	1.784	0.96(0.38)	0.39	508.3	4045.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 663.37 Tc(MIN.) = 20.11
 AREA-AVERAGED Fm(INCH/HR) = 0.37 AREA-AVERAGED Fp(INCH/HR) = 0.96
 AREA-AVERAGED Ap = 0.39 EFFECTIVE AREA(ACRES) = 379.54

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 508.3 TC(MIN.) = 20.11
 EFFECTIVE AREA(ACRES) = 379.54 AREA-AVERAGED Fm(INCH/HR) = 0.37
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.386
 PEAK FLOW RATE(CFS) = 663.37

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	642.33	16.38	2.615	0.96(0.37)	0.39	318.3	4175.00
2	663.37	20.11	2.312	0.96(0.37)	0.39	379.5	4210.00
3	658.24	29.58	1.834	0.96(0.37)	0.39	500.8	4020.00
4	652.13	30.42	1.804	0.96(0.38)	0.39	507.1	1.00
5	651.85	30.44	1.803	0.96(0.38)	0.39	507.2	4000.00
6	644.43	30.98	1.784	0.96(0.38)	0.39	508.3	4045.00

END OF RATIONAL METHOD ANALYSIS



 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
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Analysis prepared by:

Haven Ave - Proposed Condition

***** DESCRIPTION OF STUDY *****
 * SUBAREA 29 *
 * HAVEN AVE SD *
 * PROPOSED CONDITION 100YR *

FILE NAME: PRHAVEN.DAT
 TIME/DATE OF STUDY: 12:38 04/22/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
 =====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL IN- / OUT- / SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GEOMETRIES LIP (FT)	MANNING HIKE (FT)	FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
 1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.
 *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:
 WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
 SIERRA MADRE DEPTH-AREA FACTORS USED.

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

 FLOW PROCESS FROM NODE 4280.00 TO NODE 4280.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<

=====

PEAK FLOWRATE TABLE FILE NAME: MPHAVEN.DNA **MP Node 4280 per 2012 MP**
 MEMORY BANK # 1 DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	642.33	16.38	0.96(0.37)	0.39	318.3	4175.00
2	663.37	20.11	0.96(0.37)	0.39	379.5	4210.00
3	658.24	29.58	0.96(0.37)	0.39	500.8	4020.00
4	652.13	30.42	0.96(0.38)	0.39	507.1	1.00
5	651.85	30.44	0.96(0.38)	0.39	507.2	4000.00
6	644.43	30.98	0.96(0.38)	0.39	508.3	4045.00
TOTAL AREA(ACRES) =		508.3				

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4280.00 = 12849.00 FEET.

 FLOW PROCESS FROM NODE 4280.00 TO NODE 4280.00 IS CODE = 14.0

>>>>MEMORY BANK # 1 COPIED ONTO MAIN-STREAM MEMORY<<<<<

=====

MAIN-STREAM MEMORY DEFINED AS FOLLOWS:

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	642.33	16.38	0.96(0.37)	0.39	318.3	4175.00
2	663.37	20.11	0.96(0.37)	0.39	379.5	4210.00
3	658.24	29.58	0.96(0.37)	0.39	500.8	4020.00
4	652.13	30.42	0.96(0.38)	0.39	507.1	1.00
5	651.85	30.44	0.96(0.38)	0.39	507.2	4000.00
6	644.43	30.98	0.96(0.38)	0.39	508.3	4045.00
TOTAL AREA(ACRES) =		508.3				

LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4280.00 = 12849.00 FEET.

FLOW PROCESS FROM NODE 4280.00 TO NODE 8000.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 683.30 DOWNSTREAM(FEET) = 676.50
 FLOW LENGTH(FEET) = 812.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 90.0 INCH PIPE IS 73.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 17.20
 ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 663.37
 PIPE TRAVEL TIME(MIN.) = 0.79 Tc(MIN.) = 20.90
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8000.00 = 13661.00 FEET.

 FLOW PROCESS FROM NODE 8000.00 TO NODE 8000.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<
 =====
 MAINLINE Tc(MIN.) = 20.90
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.260
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 APARTMENTS A 12.00 0.98 0.200 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 12.00 SUBAREA RUNOFF(CFS) = 22.30
 EFFECTIVE AREA(ACRES) = 391.54 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.38
 TOTAL AREA(ACRES) = 520.3 PEAK FLOW RATE(CFS) = 667.69

 FLOW PROCESS FROM NODE 8000.00 TO NODE 4300.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 676.50 DOWNSTREAM(FEET) = 672.00
 FLOW LENGTH(FEET) = 527.40 MANNING'S N = 0.013
 DEPTH OF FLOW IN 90.0 INCH PIPE IS 73.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 17.36
 ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 667.69
 PIPE TRAVEL TIME(MIN.) = 0.51 Tc(MIN.) = 21.40
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4300.00 = 14188.40 FEET.

 FLOW PROCESS FROM NODE 4300.00 TO NODE 8001.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 672.00 DOWNSTREAM(FEET) = 669.30
 FLOW LENGTH(FEET) = 253.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 87.0 INCH PIPE IS 69.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.98
 ESTIMATED PIPE DIAMETER(INCH) = 87.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 667.69
 PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 21.62
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 8001.00 = 14441.40 FEET.

 FLOW PROCESS FROM NODE 8001.00 TO NODE 8001.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<
 =====

MAINLINE Tc(MIN.) = 21.62
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.214
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 APARTMENTS A 11.00 0.98 0.200 32
 COMMERCIAL A 3.30 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.177
 SUBAREA AREA(ACRES) = 14.30 SUBAREA RUNOFF(CFS) = 26.27
 EFFECTIVE AREA(ACRES) = 405.84 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.37
 TOTAL AREA(ACRES) = 534.6 PEAK FLOW RATE(CFS) = 677.76

N2, N3

 FLOW PROCESS FROM NODE 8001.00 TO NODE 4325.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 669.30 DOWNSTREAM(FEET) = 658.70
 FLOW LENGTH(FEET) = 906.30 MANNING'S N = 0.013
 DEPTH OF FLOW IN 87.0 INCH PIPE IS 67.1 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 19.83
 ESTIMATED PIPE DIAMETER(INCH) = 87.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 677.76
 PIPE TRAVEL TIME(MIN.) = 0.76 Tc(MIN.) = 22.39
 LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4325.00 = 15347.70 FEET.

 FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<
 =====

MAINLINE Tc(MIN.) = 22.39
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.168
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 APARTMENTS A 19.30 0.98 0.200 32
 APARTMENTS C 0.70 0.57 0.200 69
 COMMERCIAL A 2.00 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.96

N4, N5

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.191
 SUBAREA AREA(ACRES) = 22.00 SUBAREA RUNOFF(CFS) = 39.30
 EFFECTIVE AREA(ACRES) = 427.84 AREA-AVERAGED Fm(INCH/HR) = 0.35
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.36
 TOTAL AREA(ACRES) = 556.6 PEAK FLOW RATE(CFS) = 700.43

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 556.6 TC(MIN.) = 22.39
 EFFECTIVE AREA(ACRES) = 427.84 AREA-AVERAGED Fm(INCH/HR) = 0.35
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.364
 PEAK FLOW RATE(CFS) = 700.43

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	683.12	18.66	2.418	0.96(0.35)	0.36	366.6	4175.00
2	700.43	22.39	2.168	0.96(0.35)	0.36	427.8	4210.00
3	690.56	31.86	1.754	0.96(0.36)	0.37	549.1	4020.00
4	684.33	32.70	1.727	0.96(0.36)	0.37	555.4	1.00
5	684.05	32.72	1.727	0.96(0.36)	0.37	555.5	4000.00
6	676.74	33.26	1.710	0.96(0.36)	0.37	556.6	4045.00

=====

END OF RATIONAL METHOD ANALYSIS



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Analysis prepared by:

Mill Creek - Recreated 2012 MP Hydrology Tributary to MP Node 5425

***** DESCRIPTION OF STUDY *****
 * SUBAREA 29 *
 * MILL CREEK SD *
 * 2017 ONTARIO MP - TRIB TO MP NODE 5425 *

FILE NAME: MPMCRK.DAT
 TIME/DATE OF STUDY: 10:52 04/22/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	(FT)	(FT)	SIDE / SIDE/ WAY	(FT)	(FT)	(FT)	(FT)	(n)
1	18.0	12.0	0.020/0.020/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
 - (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)
- *SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
 *USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:

WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
 SIERRA MADRE DEPTH-AREA FACTORS USED.

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

 FLOW PROCESS FROM NODE 5000.00 TO NODE 5002.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 900.00
 ELEVATION DATA: UPSTREAM(FEET) = 810.00 DOWNSTREAM(FEET) = 806.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.646
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.918
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	3.75	0.98	0.100	32	13.65

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF(CFS) = 9.52
 TOTAL AREA(ACRES) = 3.75 PEAK FLOW RATE(CFS) = 9.52

 FLOW PROCESS FROM NODE 5002.00 TO NODE 5004.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 806.00 DOWNSTREAM ELEVATION(FEET) = 804.00
 STREET LENGTH(FEET) = 700.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

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**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      21.85
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.60
HALFSTREET FLOOD WIDTH(FEET) = 18.00
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.19
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.33
STREET FLOW TRAVEL TIME(MIN.) = 5.32  Tc(MIN.) = 18.96
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.395
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL         A      11.83  0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 11.83  SUBAREA RUNOFF(CFS) = 24.46
EFFECTIVE AREA(ACRES) = 15.58  AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98  AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 15.6  PEAK FLOW RATE(CFS) = 32.22

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.68  HALFSTREET FLOOD WIDTH(FEET) = 18.54
FLOW VELOCITY(FEET/SEC.) = 2.56  DEPTH*VELOCITY(FT*FT/SEC.) = 1.73
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5004.00 = 1600.00 FEET.

*****
FLOW PROCESS FROM NODE 5004.00 TO NODE 5006.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 804.00  DOWNSTREAM ELEVATION(FEET) = 800.00
STREET LENGTH(FEET) = 530.00  CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) =      43.96
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.64
HALFSTREET FLOOD WIDTH(FEET) = 18.00
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.88
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 2.49
STREET FLOW TRAVEL TIME(MIN.) = 2.27  Tc(MIN.) = 21.24
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.238
SUBAREA LOSS RATE DATA(AMC II):

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DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL         A      12.19  0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 12.19  SUBAREA RUNOFF(CFS) = 23.48
EFFECTIVE AREA(ACRES) = 27.77  AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98  AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 27.8  PEAK FLOW RATE(CFS) = 53.49

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.68  HALFSTREET FLOOD WIDTH(FEET) = 18.73
FLOW VELOCITY(FEET/SEC.) = 4.20  DEPTH*VELOCITY(FT*FT/SEC.) = 2.86
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5006.00 = 2130.00 FEET.

*****
FLOW PROCESS FROM NODE 5006.00 TO NODE 5010.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 793.00  DOWNSTREAM(FEET) = 784.00
FLOW LENGTH(FEET) = 670.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 11.11
ESTIMATED PIPE DIAMETER(INCH) = 33.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 53.49
PIPE TRAVEL TIME(MIN.) = 1.01  Tc(MIN.) = 22.24
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5010.00 = 2800.00 FEET.

*****
FLOW PROCESS FROM NODE 5010.00 TO NODE 5010.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 22.24
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.177
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE           GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK         A      1.27  0.98  0.850  32
COMMERCIAL         A      12.40  0.98  0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.170
SUBAREA AREA(ACRES) = 13.67  SUBAREA RUNOFF(CFS) = 24.74
EFFECTIVE AREA(ACRES) = 41.44  AREA-AVERAGED Fm(INCH/HR) = 0.12
AREA-AVERAGED Fp(INCH/HR) = 0.98  AREA-AVERAGED Ap = 0.12
TOTAL AREA(ACRES) = 41.4  PEAK FLOW RATE(CFS) = 76.70

*****
FLOW PROCESS FROM NODE 5010.00 TO NODE 5010.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 22.24

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* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.177
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       0.61   0.98  0.100  32
PUBLIC PARK         A       0.16   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.256
SUBAREA AREA(ACRES) = 0.77   SUBAREA RUNOFF(CFS) = 1.34
EFFECTIVE AREA(ACRES) = 42.21  AREA-AVERAGED Fm(INCH/HR) = 0.12
AREA-AVERAGED Fp(INCH/HR) = 0.97  AREA-AVERAGED Ap = 0.13
TOTAL AREA(ACRES) = 42.2   PEAK FLOW RATE(CFS) = 78.04

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*****
FLOW PROCESS FROM NODE 5010.00 TO NODE 5010.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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=====
MAINLINE Tc(MIN.) = 22.24
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.177
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       0.61   0.98  0.100  32
PUBLIC PARK         A       0.51   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.442
SUBAREA AREA(ACRES) = 1.12   SUBAREA RUNOFF(CFS) = 1.76
EFFECTIVE AREA(ACRES) = 43.33  AREA-AVERAGED Fm(INCH/HR) = 0.13
AREA-AVERAGED Fp(INCH/HR) = 0.97  AREA-AVERAGED Ap = 0.13
TOTAL AREA(ACRES) = 43.3   PEAK FLOW RATE(CFS) = 79.80

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*****
FLOW PROCESS FROM NODE 5010.00 TO NODE 5011.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 784.00  DOWNSTREAM(FEET) = 782.48
FLOW LENGTH(FEET) = 479.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.18
ESTIMATED PIPE DIAMETER(INCH) = 51.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 79.80
PIPE TRAVEL TIME(MIN.) = 1.11  Tc(MIN.) = 23.35
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5011.00 = 3279.00 FEET.

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*****
FLOW PROCESS FROM NODE 5011.00 TO NODE 5011.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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=====
MAINLINE Tc(MIN.) = 23.35
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.114
SUBAREA LOSS RATE DATA(AMC II):

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DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       0.63   0.98  0.100  32
PUBLIC PARK         A       0.52   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.439
SUBAREA AREA(ACRES) = 1.15   SUBAREA RUNOFF(CFS) = 1.74
EFFECTIVE AREA(ACRES) = 44.48  AREA-AVERAGED Fm(INCH/HR) = 0.14
AREA-AVERAGED Fp(INCH/HR) = 0.97  AREA-AVERAGED Ap = 0.14
TOTAL AREA(ACRES) = 44.5   PEAK FLOW RATE(CFS) = 79.80
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 5011.00 TO NODE 5012.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 782.48  DOWNSTREAM(FEET) = 780.95
FLOW LENGTH(FEET) = 482.00  MANNING'S N = 0.013
DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.18
ESTIMATED PIPE DIAMETER(INCH) = 51.00  NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 79.80
PIPE TRAVEL TIME(MIN.) = 1.12  Tc(MIN.) = 24.47
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5012.00 = 3761.00 FEET.

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*****
FLOW PROCESS FROM NODE 5012.00 TO NODE 5012.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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=====
MAINLINE Tc(MIN.) = 24.47
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.055
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP   (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A       0.53   0.98  0.100  32
PUBLIC PARK         A       0.36   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.403
SUBAREA AREA(ACRES) = 0.89   SUBAREA RUNOFF(CFS) = 1.33
EFFECTIVE AREA(ACRES) = 45.37  AREA-AVERAGED Fm(INCH/HR) = 0.14
AREA-AVERAGED Fp(INCH/HR) = 0.97  AREA-AVERAGED Ap = 0.15
TOTAL AREA(ACRES) = 45.4   PEAK FLOW RATE(CFS) = 79.80
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 5012.00 TO NODE 5015.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 780.95  DOWNSTREAM(FEET) = 779.48
FLOW LENGTH(FEET) = 460.00  MANNING'S N = 0.013

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DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.20
 ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 79.80
 PIPE TRAVEL TIME(MIN.) = 1.06 Tc(MIN.) = 25.54
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5015.00 = 4221.00 FEET.

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.460
 SUBAREA AREA(ACRES) = 2.31 SUBAREA RUNOFF(CFS) = 2.97
 EFFECTIVE AREA(ACRES) = 48.52 AREA-AVERAGED Fm(INCH/HR) = 0.16
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.17
 TOTAL AREA(ACRES) = 48.5 PEAK FLOW RATE(CFS) = 79.80
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5015.00 TO NODE 5015.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 25.54
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.003
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.46	0.98	0.100	32
PUBLIC PARK	A	0.38	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.439
 SUBAREA AREA(ACRES) = 0.84 SUBAREA RUNOFF(CFS) = 1.19
 EFFECTIVE AREA(ACRES) = 46.21 AREA-AVERAGED Fm(INCH/HR) = 0.15
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.15
 TOTAL AREA(ACRES) = 46.2 PEAK FLOW RATE(CFS) = 79.80
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5015.00 TO NODE 5020.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 779.48 DOWNSTREAM(FEET) = 760.54
 FLOW LENGTH(FEET) = 1937.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.00
 ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 79.80
 PIPE TRAVEL TIME(MIN.) = 2.93 Tc(MIN.) = 28.47
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5020.00 = 6158.00 FEET.

FLOW PROCESS FROM NODE 5020.00 TO NODE 5020.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 28.47
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.877
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	1.20	0.98	0.100	32
PUBLIC PARK	A	1.11	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

FLOW PROCESS FROM NODE 5020.00 TO NODE 5020.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 28.47
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.877
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
SCHOOL	A	4.59	0.98	0.600	32
COMMERCIAL	A	2.10	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.443
 SUBAREA AREA(ACRES) = 6.69 SUBAREA RUNOFF(CFS) = 8.70
 EFFECTIVE AREA(ACRES) = 55.21 AREA-AVERAGED Fm(INCH/HR) = 0.20
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 55.2 PEAK FLOW RATE(CFS) = 83.57

FLOW PROCESS FROM NODE 5020.00 TO NODE 5114.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 760.54 DOWNSTREAM(FEET) = 748.44
 FLOW LENGTH(FEET) = 1019.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 39.0 INCH PIPE IS 31.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.73
 ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 83.57
 PIPE TRAVEL TIME(MIN.) = 1.45 Tc(MIN.) = 29.92
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5114.00 = 7177.00 FEET.

FLOW PROCESS FROM NODE 5114.00 TO NODE 5114.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 29.92
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.822
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.68	0.98	0.100	32
PUBLIC PARK	A	0.64	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.464

SUBAREA AREA(ACRES) = 1.32 SUBAREA RUNOFF(CFS) = 1.63
 EFFECTIVE AREA(ACRES) = 56.53 AREA-AVERAGED Fm(INCH/HR) = 0.20
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.21
 TOTAL AREA(ACRES) = 56.5 PEAK FLOW RATE(CFS) = 83.57
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5114.00 TO NODE 5114.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 29.92
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.822
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.75	0.98	0.100	32
PUBLIC PARK	A	0.57	0.98	0.850	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.424
 SUBAREA AREA(ACRES) = 1.32 SUBAREA RUNOFF(CFS) = 1.67
 EFFECTIVE AREA(ACRES) = 57.85 AREA-AVERAGED Fm(INCH/HR) = 0.21
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.21
 TOTAL AREA(ACRES) = 57.9 PEAK FLOW RATE(CFS) = 84.13

 FLOW PROCESS FROM NODE 5114.00 TO NODE 5114.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 29.92
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.822
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.83	0.98	0.100	32
PUBLIC PARK	A	0.66	0.98	0.850	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.432
 SUBAREA AREA(ACRES) = 1.49 SUBAREA RUNOFF(CFS) = 1.88
 EFFECTIVE AREA(ACRES) = 59.34 AREA-AVERAGED Fm(INCH/HR) = 0.21
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.22
 TOTAL AREA(ACRES) = 59.3 PEAK FLOW RATE(CFS) = 86.01

 FLOW PROCESS FROM NODE 5114.00 TO NODE 5155.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====

ELEVATION DATA: UPSTREAM(FEET) = 748.44 DOWNSTREAM(FEET) = 747.58
 FLOW LENGTH(FEET) = 47.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 39.0 INCH PIPE IS 26.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 14.23
 ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1

PIPE-FLOW(CFS) = 86.01
 PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 29.97
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5155.00 = 7224.00 FEET.

 FLOW PROCESS FROM NODE 5115.00 TO NODE 5115.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 5043.00 TO NODE 5044.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 410.00
 ELEVATION DATA: UPSTREAM(FEET) = 773.59 DOWNSTREAM(FEET) = 768.25

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.036
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.009
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	0.32	0.98	0.100	32	8.04
PUBLIC PARK	A	0.30	0.98	0.850	32	12.77

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.463
 SUBAREA RUNOFF(CFS) = 1.99
 TOTAL AREA(ACRES) = 0.62 PEAK FLOW RATE(CFS) = 1.99

 FLOW PROCESS FROM NODE 5044.00 TO NODE 5044.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 8.04
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.009
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.32	0.98	0.100	32
PUBLIC PARK	A	0.30	0.98	0.850	32

SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.463
 SUBAREA AREA(ACRES) = 0.62 SUBAREA RUNOFF(CFS) = 1.99
 EFFECTIVE AREA(ACRES) = 1.24 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.46
 TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.97

 FLOW PROCESS FROM NODE 5044.00 TO NODE 5045.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

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>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 757.77 DOWNSTREAM(FEET) = 753.86
FLOW LENGTH(FEET) = 400.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
DEPTH OF FLOW IN 18.0 INCH PIPE IS 8.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.28
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 3.97
PIPE TRAVEL TIME(MIN.) = 1.26 Tc(MIN.) = 9.30
LONGEST FLOWPATH FROM NODE 5043.00 TO NODE 5045.00 = 810.00 FEET.

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FLOW PROCESS FROM NODE 5045.00 TO NODE 5045.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
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MAINLINE Tc(MIN.) = 9.30
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.673
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL  AREA      Fp      Ap      SCS
LAND USE            GROUP  (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A      0.29    0.98    0.100  32
PUBLIC PARK         A      0.27    0.98    0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.462
SUBAREA AREA(ACRES) = 0.56 SUBAREA RUNOFF(CFS) = 1.62
EFFECTIVE AREA(ACRES) = 1.80 AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 1.8 PEAK FLOW RATE(CFS) = 5.22

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*****
FLOW PROCESS FROM NODE 5045.00 TO NODE 5045.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
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MAINLINE Tc(MIN.) = 9.30
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.673
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL  AREA      Fp      Ap      SCS
LAND USE            GROUP  (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A      0.29    0.98    0.100  32
PUBLIC PARK         A      0.27    0.98    0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.462
SUBAREA AREA(ACRES) = 0.56 SUBAREA RUNOFF(CFS) = 1.62
EFFECTIVE AREA(ACRES) = 2.36 AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.46
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 6.84

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*****
FLOW PROCESS FROM NODE 5045.00 TO NODE 5045.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
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TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 9.30
RAINFALL INTENSITY(INCH/HR) = 3.67
AREA-AVERAGED Fm(INCH/HR) = 0.45
AREA-AVERAGED Fp(INCH/HR) = 0.98
AREA-AVERAGED Ap = 0.46
EFFECTIVE STREAM AREA(ACRES) = 2.36
TOTAL STREAM AREA(ACRES) = 2.36
PEAK FLOW RATE(CFS) AT CONFLUENCE = 6.84

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*****
FLOW PROCESS FROM NODE 5025.00 TO NODE 5030.00 IS CODE = 21
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

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INITIAL SUBAREA FLOW-LENGTH(FEET) = 720.00
ELEVATION DATA: UPSTREAM(FEET) = 793.00 DOWNSTREAM(FEET) = 785.00

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Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.391
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.436
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL  AREA      Fp      Ap      SCS  Tc
LAND USE            GROUP  (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL          A      5.96    0.98    0.100  32  10.39
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 17.91
TOTAL AREA(ACRES) = 5.96 PEAK FLOW RATE(CFS) = 17.91

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FLOW PROCESS FROM NODE 5030.00 TO NODE 5035.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<
=====

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ELEVATION DATA: UPSTREAM(FEET) = 785.00 DOWNSTREAM(FEET) = 784.30
CHANNEL LENGTH THRU SUBAREA(FEET) = 340.00 CHANNEL SLOPE = 0.0021
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.941
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL  AREA      Fp      Ap      SCS
LAND USE            GROUP  (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A      8.48    0.98    0.100  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 28.78
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.84
AVERAGE FLOW DEPTH(FEET) = 0.39 TRAVEL TIME(MIN.) = 3.08
Tc(MIN.) = 13.47
SUBAREA AREA(ACRES) = 8.48 SUBAREA RUNOFF(CFS) = 21.70
EFFECTIVE AREA(ACRES) = 14.44 AREA-AVERAGED Fm(INCH/HR) = 0.10

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AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 14.4 PEAK FLOW RATE(CFS) = 36.95

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.45 FLOW VELOCITY(FEET/SEC.) = 1.96
 LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5035.00 = 1060.00 FEET.

FLOW PROCESS FROM NODE 5035.00 TO NODE 5040.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 774.30 DOWNSTREAM(FEET) = 770.10
 FLOW LENGTH(FEET) = 860.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 25.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.00
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 36.95
 PIPE TRAVEL TIME(MIN.) = 2.05 Tc(MIN.) = 15.52
 LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5040.00 = 1920.00 FEET.

FLOW PROCESS FROM NODE 5040.00 TO NODE 5040.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<<<

MAINLINE Tc(MIN.) = 15.52
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.701
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 19.84 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 19.84 SUBAREA RUNOFF(CFS) = 46.50
 EFFECTIVE AREA(ACRES) = 34.28 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 34.3 PEAK FLOW RATE(CFS) = 80.34

FLOW PROCESS FROM NODE 5040.00 TO NODE 5045.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 770.10 DOWNSTREAM(FEET) = 753.86
 FLOW LENGTH(FEET) = 1010.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 39.0 INCH PIPE IS 26.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 13.33
 ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 80.34
 PIPE TRAVEL TIME(MIN.) = 1.26 Tc(MIN.) = 16.78
 LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5045.00 = 2930.00 FEET.

FLOW PROCESS FROM NODE 5045.00 TO NODE 5045.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<<<

MAINLINE Tc(MIN.) = 16.78
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.578
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 23.34 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 23.34 SUBAREA RUNOFF(CFS) = 52.10
 EFFECTIVE AREA(ACRES) = 57.62 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 57.6 PEAK FLOW RATE(CFS) = 128.61

FLOW PROCESS FROM NODE 5045.00 TO NODE 5045.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 16.78
 RAINFALL INTENSITY(INCH/HR) = 2.58
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.97
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 57.62
 TOTAL STREAM AREA(ACRES) = 57.62
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 128.61

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	6.84	9.30	3.673	0.98(0.45)	0.46	2.4	5043.00
2	128.61	16.78	2.578	0.97(0.10)	0.10	57.6	5025.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	109.60	9.30	3.673	0.97(0.12)	0.12	34.3	5043.00
2	133.13	16.78	2.578	0.97(0.11)	0.11	60.0	5025.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 133.13 Tc(MIN.) = 16.78
 EFFECTIVE AREA(ACRES) = 59.98 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.11
 TOTAL AREA(ACRES) = 60.0


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*****
FLOW PROCESS FROM NODE 5110.00 TO NODE 5110.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 18.79
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.408
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A         0.25   0.98  0.100  32
PUBLIC PARK         A         0.24   0.98  0.850  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.467
SUBAREA AREA(ACRES) = 0.49   SUBAREA RUNOFF(CFS) = 0.86
EFFECTIVE AREA(ACRES) = 76.36   AREA-AVERAGED Fm(INCH/HR) = 0.25
AREA-AVERAGED Fp(INCH/HR) = 0.97   AREA-AVERAGED Ap = 0.26
TOTAL AREA(ACRES) = 76.4   PEAK FLOW RATE(CFS) = 152.70
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 5110.00 TO NODE 5110.00 IS CODE = 10
-----
>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<
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*****
FLOW PROCESS FROM NODE 5050.00 TO NODE 5055.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 520.00
ELEVATION DATA: UPSTREAM(FEET) = 797.72   DOWNSTREAM(FEET) = 783.05

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.689
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.584
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS   Tc
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"5-7 DWELLINGS/ACRE"   A         6.57   0.98  0.500  32   9.69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA RUNOFF(CFS) = 18.31
TOTAL AREA(ACRES) = 6.57   PEAK FLOW RATE(CFS) = 18.31

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*****
FLOW PROCESS FROM NODE 5055.00 TO NODE 5060.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 773.05   DOWNSTREAM(FEET) = 764.33

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```

FLOW LENGTH(FEET) = 740.00   MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.21
ESTIMATED PIPE DIAMETER(INCH) = 24.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 18.31
PIPE TRAVEL TIME(MIN.) = 1.50   Tc(MIN.) = 11.19
LONGEST FLOWPATH FROM NODE 5050.00 TO NODE 5060.00 = 1260.00 FEET.

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*****
FLOW PROCESS FROM NODE 5060.00 TO NODE 5060.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 11.19
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.287
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE"   A         9.33   0.98  0.500  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 9.33   SUBAREA RUNOFF(CFS) = 23.50
EFFECTIVE AREA(ACRES) = 15.90   AREA-AVERAGED Fm(INCH/HR) = 0.49
AREA-AVERAGED Fp(INCH/HR) = 0.98   AREA-AVERAGED Ap = 0.50
TOTAL AREA(ACRES) = 15.9   PEAK FLOW RATE(CFS) = 40.06

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*****
FLOW PROCESS FROM NODE 5060.00 TO NODE 5065.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 764.33   DOWNSTREAM(FEET) = 761.54
FLOW LENGTH(FEET) = 525.00   MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 25.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.35
ESTIMATED PIPE DIAMETER(INCH) = 36.00   NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 40.06
PIPE TRAVEL TIME(MIN.) = 1.19   Tc(MIN.) = 12.38
LONGEST FLOWPATH FROM NODE 5050.00 TO NODE 5065.00 = 1785.00 FEET.

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*****
FLOW PROCESS FROM NODE 5065.00 TO NODE 5065.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 12.38
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.093
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA   Fp   Ap   SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE"   A         5.49   0.98  0.200  32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

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SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 5.49 SUBAREA RUNOFF(CFS) = 14.32
 EFFECTIVE AREA(ACRES) = 21.39 AREA-AVERAGED Fm(INCH/HR) = 0.41
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.42
 TOTAL AREA(ACRES) = 21.4 PEAK FLOW RATE(CFS) = 51.61

 FLOW PROCESS FROM NODE 5065.00 TO NODE 5100.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 761.54 DOWNSTREAM(FEET) = 759.53
 FLOW LENGTH(FEET) = 165.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 25.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.59
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 51.61
 PIPE TRAVEL TIME(MIN.) = 0.26 Tc(MIN.) = 12.64
 LONGEST FLOWPATH FROM NODE 5050.00 TO NODE 5100.00 = 1950.00 FEET.

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 12.64
 RAINFALL INTENSITY(INCH/HR) = 3.05
 AREA-AVERAGED Fm(INCH/HR) = 0.41
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.42
 EFFECTIVE STREAM AREA(ACRES) = 21.39
 TOTAL STREAM AREA(ACRES) = 21.39
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 51.61

 FLOW PROCESS FROM NODE 5070.00 TO NODE 5075.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 730.00
 ELEVATION DATA: UPSTREAM(FEET) = 797.68 DOWNSTREAM(FEET) = 779.53

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.381
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.254

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						
"5-7 DWELLINGS/ACRE"	A	1.43	0.98	0.500	32	11.38

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA RUNOFF(CFS) = 3.56
 TOTAL AREA(ACRES) = 1.43 PEAK FLOW RATE(CFS) = 3.56

 FLOW PROCESS FROM NODE 5075.00 TO NODE 5100.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 779.53 DOWNSTREAM ELEVATION(FEET) = 769.53
 STREET LENGTH(FEET) = 1040.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.97

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.37
 HALFSTREET FLOOD WIDTH(FEET) = 10.45
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.33
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.86
 STREET FLOW TRAVEL TIME(MIN.) = 7.44 Tc(MIN.) = 18.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.406

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	A	1.00	0.98	0.500	32
RESIDENTIAL					
"11+ DWELLINGS/ACRE"	A	1.54	0.98	0.200	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.318
 SUBAREA AREA(ACRES) = 2.54 SUBAREA RUNOFF(CFS) = 4.79
 EFFECTIVE AREA(ACRES) = 3.97 AREA-AVERAGED Fm(INCH/HR) = 0.37
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.38
 TOTAL AREA(ACRES) = 4.0 PEAK FLOW RATE(CFS) = 7.26

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.39 HALFSTREET FLOOD WIDTH(FEET) = 11.39
 FLOW VELOCITY(FEET/SEC.) = 2.44 DEPTH*VELOCITY(FT*FT/SEC.) = 0.94
 LONGEST FLOWPATH FROM NODE 5070.00 TO NODE 5100.00 = 1770.00 FEET.

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 18.82
 RAINFALL INTENSITY(INCH/HR) = 2.41
 AREA-AVERAGED Fm(INCH/HR) = 0.37
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.38
 EFFECTIVE STREAM AREA(ACRES) = 3.97
 TOTAL STREAM AREA(ACRES) = 3.97
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.26

FLOW PROCESS FROM NODE 5080.00 TO NODE 5085.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 520.00
 ELEVATION DATA: UPSTREAM(FEET) = 797.72 DOWNSTREAM(FEET) = 783.05

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.689
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.584
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	6.58	0.98	0.500	32	9.69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA RUNOFF(CFS) = 18.33
 TOTAL AREA(ACRES) = 6.58 PEAK FLOW RATE(CFS) = 18.33

FLOW PROCESS FROM NODE 5085.00 TO NODE 5090.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 773.05 DOWNSTREAM(FEET) = 767.13
 FLOW LENGTH(FEET) = 410.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 8.91
 ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 18.33
 PIPE TRAVEL TIME(MIN.) = 0.77 Tc(MIN.) = 10.46
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5090.00 = 930.00 FEET.

FLOW PROCESS FROM NODE 5090.00 TO NODE 5090.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 10.46
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.423

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	6.91	0.98	0.500	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 6.91 SUBAREA RUNOFF(CFS) = 18.26
 EFFECTIVE AREA(ACRES) = 13.49 AREA-AVERAGED Fm(INCH/HR) = 0.49
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.50
 TOTAL AREA(ACRES) = 13.5 PEAK FLOW RATE(CFS) = 35.65

FLOW PROCESS FROM NODE 5090.00 TO NODE 5095.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 767.13 DOWNSTREAM(FEET) = 763.05
 FLOW LENGTH(FEET) = 355.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 21.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.55
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 35.65
 PIPE TRAVEL TIME(MIN.) = 0.62 Tc(MIN.) = 11.08
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5095.00 = 1285.00 FEET.

FLOW PROCESS FROM NODE 5095.00 TO NODE 5095.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 11.08
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.307
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "11+ DWELLINGS/ACRE"	A	3.86	0.98	0.200	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 3.86 SUBAREA RUNOFF(CFS) = 10.81
 EFFECTIVE AREA(ACRES) = 17.35 AREA-AVERAGED Fm(INCH/HR) = 0.42
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.43
 TOTAL AREA(ACRES) = 17.4 PEAK FLOW RATE(CFS) = 45.05

FLOW PROCESS FROM NODE 5095.00 TO NODE 5100.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 763.05 DOWNSTREAM(FEET) = 759.53
 FLOW LENGTH(FEET) = 686.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 29.3 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 7.31
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 45.05
 PIPE TRAVEL TIME(MIN.) = 1.56 Tc(MIN.) = 12.64
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5100.00 = 1971.00 FEET.

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5100.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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MAINLINE Tc(MIN.) = 12.64
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.055
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "11+ DWELLINGS/ACRE"	A	6.90	0.98	0.200	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 6.90 SUBAREA RUNOFF(CFS) = 17.76
 EFFECTIVE AREA(ACRES) = 24.25 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.37
 TOTAL AREA(ACRES) = 24.2 PEAK FLOW RATE(CFS) = 58.87

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5100.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

=====

TOTAL NUMBER OF STREAMS = 3
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 3 ARE:
 TIME OF CONCENTRATION(MIN.) = 12.64
 RAINFALL INTENSITY(INCH/HR) = 3.06
 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.37
 EFFECTIVE STREAM AREA(ACRES) = 24.25
 TOTAL STREAM AREA(ACRES) = 24.25
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 58.87

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	51.61	12.64	3.055	0.98(0.41)	0.42	21.4	5050.00
2	7.26	18.82	2.406	0.98(0.37)	0.38	4.0	5070.00
3	58.87	12.64	3.055	0.98(0.36)	0.37	24.2	5080.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 3 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
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1	116.91	12.64	3.055	0.98(0.38)	0.39	48.3	5080.00
2	116.91	12.64	3.055	0.97(0.38)	0.39	48.3	5050.00
3	90.89	18.82	2.406	0.98(0.38)	0.39	49.6	5070.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 116.91 Tc(MIN.) = 12.64
 EFFECTIVE AREA(ACRES) = 48.30 AREA-AVERAGED Fm(INCH/HR) = 0.38
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.39
 TOTAL AREA(ACRES) = 49.6
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5100.00 = 1971.00 FEET.

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5105.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 759.53 DOWNSTREAM(FEET) = 757.79
 FLOW LENGTH(FEET) = 250.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 51.0 INCH PIPE IS 37.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.62
 ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 116.91
 PIPE TRAVEL TIME(MIN.) = 0.39 Tc(MIN.) = 13.03
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5105.00 = 2221.00 FEET.

 FLOW PROCESS FROM NODE 5105.00 TO NODE 5105.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 13.03
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.000
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "11+ DWELLINGS/ACRE"	A	7.08	0.98	0.200	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 7.08 SUBAREA RUNOFF(CFS) = 17.87
 EFFECTIVE AREA(ACRES) = 55.38 AREA-AVERAGED Fm(INCH/HR) = 0.36
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.37
 TOTAL AREA(ACRES) = 56.7 PEAK FLOW RATE(CFS) = 131.63

 FLOW PROCESS FROM NODE 5105.00 TO NODE 5110.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 757.79 DOWNSTREAM(FEET) = 754.21
 FLOW LENGTH(FEET) = 520.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 51.0 INCH PIPE IS 41.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.68

ESTIMATED PIPE DIAMETER(INCH) = 51.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 131.63
 PIPE TRAVEL TIME(MIN.) = 0.81 Tc(MIN.) = 13.84
 LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5110.00 = 2741.00 FEET.

 FLOW PROCESS FROM NODE 5110.00 TO NODE 5110.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 13.84
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.893
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "11+ DWELLINGS/ACRE"	A	14.26	0.98	0.200	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 14.26 SUBAREA RUNOFF(CFS) = 34.62
 EFFECTIVE AREA(ACRES) = 69.64 AREA-AVERAGED Fm(INCH/HR) = 0.33
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.33
 TOTAL AREA(ACRES) = 71.0 PEAK FLOW RATE(CFS) = 160.93

 FLOW PROCESS FROM NODE 5100.00 TO NODE 5100.00 IS CODE = 11

 >>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

=====

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	160.93	13.84	2.893	0.98(0.33)	0.33	69.6	5080.00
2	160.93	13.84	2.893	0.98(0.33)	0.33	69.6	5050.00
3	126.89	20.09	2.313	0.98(0.33)	0.33	71.0	5070.00

LONGEST FLOWPATH FROM NODE 5080.00 TO NODE 5100.00 = 2741.00 FEET.

** MEMORY BANK # 2 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	140.63	11.38	3.254	0.97(0.33)	0.34	50.7	5043.00
2	152.70	18.79	2.408	0.97(0.25)	0.26	76.4	5025.00

LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5100.00 = 3981.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	291.52	11.38	3.254	0.98(0.33)	0.34	107.9	5043.00
2	305.58	13.84	2.893	0.98(0.32)	0.32	128.9	5080.00
3	305.57	13.84	2.893	0.97(0.32)	0.32	128.9	5050.00
4	286.68	18.79	2.408	0.98(0.29)	0.29	147.0	5025.00
5	272.88	20.09	2.313	0.98(0.29)	0.29	147.3	5070.00

TOTAL AREA(ACRES) = 147.3

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 305.58 Tc(MIN.) = 13.843
 EFFECTIVE AREA(ACRES) = 128.85 AREA-AVERAGED Fm(INCH/HR) = 0.32
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.34
 TOTAL AREA(ACRES) = 147.3
 LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5100.00 = 3981.00 FEET.

 FLOW PROCESS FROM NODE 5110.00 TO NODE 5111.00 IS CODE = 12

 >>>>CLEAR MEMORY BANK # 2 <<<<<

=====

 FLOW PROCESS FROM NODE 5110.00 TO NODE 5111.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 749.99 DOWNSTREAM(FEET) = 749.73
 FLOW LENGTH(FEET) = 86.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 84.0 INCH PIPE IS 63.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.82
 ESTIMATED PIPE DIAMETER(INCH) = 84.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 305.58
 PIPE TRAVEL TIME(MIN.) = 0.15 Tc(MIN.) = 13.99
 LONGEST FLOWPATH FROM NODE 5025.00 TO NODE 5111.00 = 4067.00 FEET.

 FLOW PROCESS FROM NODE 5111.00 TO NODE 5111.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 13.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.875
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.29	0.98	0.100	32
PUBLIC PARK	A	0.21	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.415
 SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 1.11
 EFFECTIVE AREA(ACRES) = 129.35 AREA-AVERAGED Fm(INCH/HR) = 0.32
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.32
 TOTAL AREA(ACRES) = 147.8 PEAK FLOW RATE(CFS) = 305.58
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5111.00 TO NODE 5111.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 13.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.875


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>>>>CLEAR MEMORY BANK # 1 <<<<<
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*****
FLOW PROCESS FROM NODE 5115.00 TO NODE 5116.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 747.58 DOWNSTREAM(FEET) = 746.68
FLOW LENGTH(FEET) = 301.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 90.0 INCH PIPE IS 69.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.25
ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 373.59
PIPE TRAVEL TIME(MIN.) = 0.49 Tc(MIN.) = 15.42
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5116.00 = 7525.00 FEET.

*****
FLOW PROCESS FROM NODE 5116.00 TO NODE 5116.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 15.42
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.711
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK A 7.72 0.98 0.850 32
RESIDENTIAL
"11+ DWELLINGS/ACRE" A 1.94 0.98 0.200 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.719
SUBAREA AREA(ACRES) = 9.66 SUBAREA RUNOFF(CFS) = 17.47
EFFECTIVE AREA(ACRES) = 170.07 AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.33
TOTAL AREA(ACRES) = 218.3 PEAK FLOW RATE(CFS) = 373.59
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE 5116.00 TO NODE 5117.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 746.68 DOWNSTREAM(FEET) = 744.57
FLOW LENGTH(FEET) = 389.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 81.0 INCH PIPE IS 61.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.85
ESTIMATED PIPE DIAMETER(INCH) = 81.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 373.59
PIPE TRAVEL TIME(MIN.) = 0.50 Tc(MIN.) = 15.93
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5117.00 = 7914.00 FEET.

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*****
FLOW PROCESS FROM NODE 5117.00 TO NODE 5117.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 15.93
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.659
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 0.51 0.98 0.100 32
PUBLIC PARK A 0.48 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.464
SUBAREA AREA(ACRES) = 0.99 SUBAREA RUNOFF(CFS) = 1.97
EFFECTIVE AREA(ACRES) = 171.06 AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.33
TOTAL AREA(ACRES) = 219.3 PEAK FLOW RATE(CFS) = 373.59
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

*****
FLOW PROCESS FROM NODE 5117.00 TO NODE 5141.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 744.57 DOWNSTREAM(FEET) = 734.24
FLOW LENGTH(FEET) = 1156.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 72.0 INCH PIPE IS 58.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 15.31
ESTIMATED PIPE DIAMETER(INCH) = 72.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 373.59
PIPE TRAVEL TIME(MIN.) = 1.26 Tc(MIN.) = 17.19
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5141.00 = 9070.00 FEET.

*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5141.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 17.19
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.541
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 0.85 0.98 0.100 32
PUBLIC PARK A 0.79 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.461
SUBAREA AREA(ACRES) = 1.64 SUBAREA RUNOFF(CFS) = 3.09
EFFECTIVE AREA(ACRES) = 172.70 AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.33
TOTAL AREA(ACRES) = 220.9 PEAK FLOW RATE(CFS) = 373.59
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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```
*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5141.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 17.19
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.541
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp        Ap    SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL          A         1.65     0.98     0.100   32
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 1.65   SUBAREA RUNOFF(CFS) = 3.63
EFFECTIVE AREA(ACRES) = 174.35   AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.97   AREA-AVERAGED Ap = 0.33
TOTAL AREA(ACRES) = 222.6   PEAK FLOW RATE(CFS) = 373.59
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE
```

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*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5141.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 17.19
RAINFALL INTENSITY(INCH/HR) = 2.54
AREA-AVERAGED Fm(INCH/HR) = 0.32
AREA-AVERAGED Fp(INCH/HR) = 0.97
AREA-AVERAGED Ap = 0.33
EFFECTIVE STREAM AREA(ACRES) = 174.35
TOTAL STREAM AREA(ACRES) = 222.58
PEAK FLOW RATE(CFS) AT CONFLUENCE = 373.59
```

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*****
FLOW PROCESS FROM NODE 5142.00 TO NODE 5143.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 800.00
ELEVATION DATA: UPSTREAM(FEET) = 762.00   DOWNSTREAM(FEET) = 753.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.739
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.194
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp        Ap    SCS   Tc
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"11+ DWELLINGS/ACRE" A         5.30     0.98     0.200   32   11.74
PUBLIC PARK          A         0.84     0.98     0.850   32   17.50
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.289
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SUBAREA RUNOFF(CFS) = 16.09
TOTAL AREA(ACRES) = 6.14   PEAK FLOW RATE(CFS) = 16.09
-----
*****
FLOW PROCESS FROM NODE 5143.00 TO NODE 5144.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 753.80   DOWNSTREAM ELEVATION(FEET) = 741.50
STREET LENGTH(FEET) = 1190.00   CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200
```

```
**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 43.90
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.61
HALFSTREET FLOOD WIDTH(FEET) = 18.00
AVERAGE FLOW VELOCITY(FEET/SEC.) = 4.26
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 2.62
STREET FLOW TRAVEL TIME(MIN.) = 4.66   Tc(MIN.) = 16.40
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.614
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/   SCS SOIL   AREA      Fp        Ap    SCS
LAND USE            GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK          A         1.00     0.98     0.850   32
RESIDENTIAL
"11+ DWELLINGS/ACRE" A         24.59    0.98     0.200   32
SUBAREA AVERAGE PVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PVIOUS AREA FRACTION, Ap = 0.225
SUBAREA AREA(ACRES) = 25.59   SUBAREA RUNOFF(CFS) = 55.13
EFFECTIVE AREA(ACRES) = 31.73   AREA-AVERAGED Fm(INCH/HR) = 0.23
AREA-AVERAGED Fp(INCH/HR) = 0.98   AREA-AVERAGED Ap = 0.24
TOTAL AREA(ACRES) = 31.7   PEAK FLOW RATE(CFS) = 68.02

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.70   HALFSTREET FLOOD WIDTH(FEET) = 19.64
FLOW VELOCITY(FEET/SEC.) = 5.06   DEPTH*VELOCITY(FT*FT/SEC.) = 3.54
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 1190.0 FT WITH ELEVATION-DROP = 12.3 FT, IS 61.9 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 5144.00
LONGEST FLOWPATH FROM NODE 5142.00 TO NODE 5144.00 = 1990.00 FEET.
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*****
FLOW PROCESS FROM NODE 5144.00 TO NODE 5141.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 741.50 DOWNSTREAM(FEET) = 735.66
FLOW LENGTH(FEET) = 776.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.61
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 68.02
PIPE TRAVEL TIME(MIN.) = 1.35 Tc(MIN.) = 17.74
LONGEST FLOWPATH FROM NODE 5142.00 TO NODE 5141.00 = 2766.00 FEET.

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*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5141.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 17.74
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.493
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE" A 17.41 0.98 0.200 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA(ACRES) = 17.41 SUBAREA RUNOFF(CFS) = 36.00
EFFECTIVE AREA(ACRES) = 49.14 AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.22
TOTAL AREA(ACRES) = 49.1 PEAK FLOW RATE(CFS) = 100.57

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*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5141.00 IS CODE = 1
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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 17.74
RAINFALL INTENSITY(INCH/HR) = 2.49
AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.98
AREA-AVERAGED Ap = 0.22
EFFECTIVE STREAM AREA(ACRES) = 49.14
TOTAL STREAM AREA(ACRES) = 49.14
PEAK FLOW RATE(CFS) AT CONFLUENCE = 100.57

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** CONFLUENCE DATA **

```

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	355.39	14.77	2.782	0.97(0.33)	0.34	148.6	5043.00
1	373.59	17.19	2.541	0.98(0.32)	0.33	174.3	5080.00
1	373.59	17.19	2.541	0.97(0.32)	0.33	174.4	5050.00
1	361.79	22.18	2.180	0.97(0.29)	0.30	202.4	5025.00

1	349.62	23.50	2.106	0.98(0.29)	0.30	205.2	5070.00
1	300.24	32.36	1.738	0.98(0.29)	0.29	222.6	5000.00
2	100.57	17.74	2.493	0.98(0.22)	0.22	49.1	5142.00

```

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

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** PEAK FLOW RATE TABLE **

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STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	449.78	14.77	2.782	0.97(0.31)	0.32	189.5	5043.00
2	473.06	17.19	2.541	0.98(0.30)	0.31	221.9	5080.00
3	473.06	17.19	2.541	0.97(0.30)	0.31	222.0	5050.00
4	472.84	17.74	2.493	0.97(0.30)	0.30	226.6	5142.00
5	448.54	22.18	2.180	0.97(0.28)	0.29	251.5	5025.00
6	433.09	23.50	2.106	0.98(0.28)	0.29	254.4	5070.00
7	367.43	32.36	1.738	0.98(0.27)	0.28	271.7	5000.00

```

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
PEAK FLOW RATE(CFS) = 473.06 Tc(MIN.) = 17.19
EFFECTIVE AREA(ACRES) = 221.96 AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.31
TOTAL AREA(ACRES) = 271.7
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5141.00 = 9070.00 FEET.

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*****
FLOW PROCESS FROM NODE 5141.00 TO NODE 5215.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 734.24 DOWNSTREAM(FEET) = 720.42
FLOW LENGTH(FEET) = 900.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 72.0 INCH PIPE IS 56.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.04
ESTIMATED PIPE DIAMETER(INCH) = 72.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 473.06
PIPE TRAVEL TIME(MIN.) = 0.75 Tc(MIN.) = 17.94
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5215.00 = 9970.00 FEET.

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*****
FLOW PROCESS FROM NODE 5215.00 TO NODE 5215.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 17.94
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.477
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 0.66 0.98 0.100 32
PUBLIC PARK A 0.63 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.466
SUBAREA AREA(ACRES) = 1.29 SUBAREA RUNOFF(CFS) = 2.35

```

EFFECTIVE AREA(ACRES) = 223.25 AREA-AVERAGED Fm(INCH/HR) = 0.30
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.31
 TOTAL AREA(ACRES) = 273.0 PEAK FLOW RATE(CFS) = 473.06
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5215.00 TO NODE 5215.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 17.94
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.477
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.80	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 0.80 SUBAREA RUNOFF(CFS) = 1.71
 EFFECTIVE AREA(ACRES) = 224.05 AREA-AVERAGED Fm(INCH/HR) = 0.30
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.31
 TOTAL AREA(ACRES) = 273.8 PEAK FLOW RATE(CFS) = 473.06
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5215.00 TO NODE 5215.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

 FLOW PROCESS FROM NODE 5120.00 TO NODE 5125.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 563.00
 ELEVATION DATA: UPSTREAM(FEET) = 768.00 DOWNSTREAM(FEET) = 766.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 12.530
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.071
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	4.78	0.98	0.100	32	12.53

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF(CFS) = 12.79
 TOTAL AREA(ACRES) = 4.78 PEAK FLOW RATE(CFS) = 12.79

 FLOW PROCESS FROM NODE 5125.00 TO NODE 5130.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 766.50 DOWNSTREAM(FEET) = 763.50
 CHANNEL LENGTH THRU SUBAREA(FEET) = 819.00 CHANNEL SLOPE = 0.0037
 CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.379

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	7.05	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 20.07
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.06
 AVERAGE FLOW DEPTH(FEET) = 0.29 TRAVEL TIME(MIN.) = 6.64
 Tc(MIN.) = 19.17
 SUBAREA AREA(ACRES) = 7.05 SUBAREA RUNOFF(CFS) = 14.48
 EFFECTIVE AREA(ACRES) = 11.83 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 11.8 PEAK FLOW RATE(CFS) = 24.30

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.32 FLOW VELOCITY(FEET/SEC.) = 2.15
 LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5130.00 = 1382.00 FEET.

 FLOW PROCESS FROM NODE 5130.00 TO NODE 5135.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 763.50 DOWNSTREAM(FEET) = 759.00
 CHANNEL LENGTH THRU SUBAREA(FEET) = 481.00 CHANNEL SLOPE = 0.0094
 CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.221

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	14.16	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 37.83
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.44
 AVERAGE FLOW DEPTH(FEET) = 0.31 TRAVEL TIME(MIN.) = 2.33
 Tc(MIN.) = 21.50
 SUBAREA AREA(ACRES) = 14.16 SUBAREA RUNOFF(CFS) = 27.06
 EFFECTIVE AREA(ACRES) = 25.99 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 26.0 PEAK FLOW RATE(CFS) = 49.67

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.35 FLOW VELOCITY(FEET/SEC.) = 3.72
 LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5135.00 = 1863.00 FEET.

FLOW PROCESS FROM NODE 5135.00 TO NODE 5136.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 759.00 DOWNSTREAM(FEET) = 755.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 387.00 CHANNEL SLOPE = 0.0090
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.124
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 11.50 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 60.16
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.88
AVERAGE FLOW DEPTH(FEET) = 0.39 TRAVEL TIME(MIN.) = 1.66
Tc(MIN.) = 23.17
SUBAREA AREA(ACRES) = 11.50 SUBAREA RUNOFF(CFS) = 20.97
EFFECTIVE AREA(ACRES) = 37.49 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 37.5 PEAK FLOW RATE(CFS) = 68.37

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.42 FLOW VELOCITY(FEET/SEC.) = 4.02
LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5136.00 = 2250.00 FEET.

FLOW PROCESS FROM NODE 5136.00 TO NODE 5155.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 755.50 DOWNSTREAM(FEET) = 751.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 438.00 CHANNEL SLOPE = 0.0103
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.037
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 12.89 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 79.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.34
AVERAGE FLOW DEPTH(FEET) = 0.44 TRAVEL TIME(MIN.) = 1.68
Tc(MIN.) = 24.85
SUBAREA AREA(ACRES) = 12.89 SUBAREA RUNOFF(CFS) = 22.49
EFFECTIVE AREA(ACRES) = 50.38 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10

TOTAL AREA(ACRES) = 50.4 PEAK FLOW RATE(CFS) = 87.92

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.45 FLOW VELOCITY(FEET/SEC.) = 4.52
LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5155.00 = 2688.00 FEET.

FLOW PROCESS FROM NODE 5155.00 TO NODE 5160.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 751.00 DOWNSTREAM(FEET) = 747.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 415.00 CHANNEL SLOPE = 0.0096
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.965
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 12.97 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 98.82
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.52
AVERAGE FLOW DEPTH(FEET) = 0.49 TRAVEL TIME(MIN.) = 1.53
Tc(MIN.) = 26.38
SUBAREA AREA(ACRES) = 12.97 SUBAREA RUNOFF(CFS) = 21.80
EFFECTIVE AREA(ACRES) = 63.35 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 63.3 PEAK FLOW RATE(CFS) = 106.46

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.51 FLOW VELOCITY(FEET/SEC.) = 4.61
LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5160.00 = 3103.00 FEET.

FLOW PROCESS FROM NODE 5160.00 TO NODE 5210.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 747.00 DOWNSTREAM(FEET) = 738.30
CHANNEL LENGTH THRU SUBAREA(FEET) = 503.00 CHANNEL SLOPE = 0.0173
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.904
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 16.23 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 119.65
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.89

AVERAGE FLOW DEPTH(FEET) = 0.47 TRAVEL TIME(MIN.) = 1.42
 Tc(MIN.) = 27.81
 SUBAREA AREA(ACRES) = 16.23 SUBAREA RUNOFF(CFS) = 26.38
 EFFECTIVE AREA(ACRES) = 79.58 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 79.6 PEAK FLOW RATE(CFS) = 129.36

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.49 FLOW VELOCITY(FEET/SEC.) = 6.02
 LONGEST FLOWPATH FROM NODE 5120.00 TO NODE 5210.00 = 3606.00 FEET.

FLOW PROCESS FROM NODE 5210.00 TO NODE 5210.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 27.81
 RAINFALL INTENSITY(INCH/HR) = 1.90
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 79.58
 TOTAL STREAM AREA(ACRES) = 79.58
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 129.36

FLOW PROCESS FROM NODE 5170.00 TO NODE 5175.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 524.00
 ELEVATION DATA: UPSTREAM(FEET) = 771.10 DOWNSTREAM(FEET) = 766.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.396
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.650
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	4.16	0.98	0.100	32	9.40

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF(CFS) = 13.30
 TOTAL AREA(ACRES) = 4.16 PEAK FLOW RATE(CFS) = 13.30

FLOW PROCESS FROM NODE 5175.00 TO NODE 5180.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 766.00 DOWNSTREAM(FEET) = 764.00

CHANNEL LENGTH THRU SUBAREA(FEET) = 917.00 CHANNEL SLOPE = 0.0022
 CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.451

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	7.07	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 20.87
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.73
 AVERAGE FLOW DEPTH(FEET) = 0.33 TRAVEL TIME(MIN.) = 8.86
 Tc(MIN.) = 18.25

SUBAREA AREA(ACRES) = 7.07 SUBAREA RUNOFF(CFS) = 14.97
 EFFECTIVE AREA(ACRES) = 11.23 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 11.2 PEAK FLOW RATE(CFS) = 23.78

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.35 FLOW VELOCITY(FEET/SEC.) = 1.78
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5180.00 = 1441.00 FEET.

FLOW PROCESS FROM NODE 5180.00 TO NODE 5185.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
 >>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 764.00 DOWNSTREAM(FEET) = 758.50
 CHANNEL LENGTH THRU SUBAREA(FEET) = 477.00 CHANNEL SLOPE = 0.0115
 CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
 MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.290

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	13.46	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 37.08
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.66
 AVERAGE FLOW DEPTH(FEET) = 0.29 TRAVEL TIME(MIN.) = 2.17
 Tc(MIN.) = 20.43
 SUBAREA AREA(ACRES) = 13.46 SUBAREA RUNOFF(CFS) = 26.57
 EFFECTIVE AREA(ACRES) = 24.69 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 24.7 PEAK FLOW RATE(CFS) = 48.73

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.33 FLOW VELOCITY(FEET/SEC.) = 4.00
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5185.00 = 1918.00 FEET.

FLOW PROCESS FROM NODE 5185.00 TO NODE 5190.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 758.50 DOWNSTREAM(FEET) = 755.50
CHANNEL LENGTH THRU SUBAREA(FEET) = 394.00 CHANNEL SLOPE = 0.0076
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.176
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 11.16 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 59.17
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.60
AVERAGE FLOW DEPTH(FEET) = 0.41 TRAVEL TIME(MIN.) = 1.82
Tc(MIN.) = 22.25
SUBAREA AREA(ACRES) = 11.16 SUBAREA RUNOFF(CFS) = 20.88
EFFECTIVE AREA(ACRES) = 35.85 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 35.8 PEAK FLOW RATE(CFS) = 67.06

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.43 FLOW VELOCITY(FEET/SEC.) = 3.73
LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5190.00 = 2312.00 FEET.

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FLOW PROCESS FROM NODE 5190.00 TO NODE 5195.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 755.50 DOWNSTREAM(FEET) = 751.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 430.00 CHANNEL SLOPE = 0.0105
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.085
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 12.16 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 77.94
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.37
AVERAGE FLOW DEPTH(FEET) = 0.43 TRAVEL TIME(MIN.) = 1.64
Tc(MIN.) = 23.89
SUBAREA AREA(ACRES) = 12.16 SUBAREA RUNOFF(CFS) = 21.75
EFFECTIVE AREA(ACRES) = 48.01 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 48.0 PEAK FLOW RATE(CFS) = 85.88

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

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DEPTH(FEET) = 0.45 FLOW VELOCITY(FEET/SEC.) = 4.44
LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5195.00 = 2742.00 FEET.

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FLOW PROCESS FROM NODE 5195.00 TO NODE 5200.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 751.00 DOWNSTREAM(FEET) = 747.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 420.00 CHANNEL SLOPE = 0.0095
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.007
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 11.98 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 96.17
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 4.48
AVERAGE FLOW DEPTH(FEET) = 0.49 TRAVEL TIME(MIN.) = 1.56
Tc(MIN.) = 25.46
SUBAREA AREA(ACRES) = 11.98 SUBAREA RUNOFF(CFS) = 20.59
EFFECTIVE AREA(ACRES) = 59.99 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 60.0 PEAK FLOW RATE(CFS) = 103.10

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.50 FLOW VELOCITY(FEET/SEC.) = 4.57
LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5200.00 = 3162.00 FEET.

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FLOW PROCESS FROM NODE 5200.00 TO NODE 5205.00 IS CODE = 51
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 747.00 DOWNSTREAM(FEET) = 744.10
CHANNEL LENGTH THRU SUBAREA(FEET) = 583.00 CHANNEL SLOPE = 0.0050
CHANNEL BASE(FEET) = 20.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 2.00
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.892
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 14.13 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 114.51
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.69
AVERAGE FLOW DEPTH(FEET) = 0.61 TRAVEL TIME(MIN.) = 2.64
Tc(MIN.) = 28.09
SUBAREA AREA(ACRES) = 14.13 SUBAREA RUNOFF(CFS) = 22.82

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EFFECTIVE AREA(ACRES) = 74.12 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 74.1 PEAK FLOW RATE(CFS) = 119.70

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
 DEPTH(FEET) = 0.63 FLOW VELOCITY(FEET/SEC.) = 3.70
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5205.00 = 3745.00 FEET.

 FLOW PROCESS FROM NODE 5205.00 TO NODE 5210.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 734.60 DOWNSTREAM(FEET) = 728.80
 FLOW LENGTH(FEET) = 1150.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 54.0 INCH PIPE IS 40.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.43
 ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 119.70
 PIPE TRAVEL TIME(MIN.) = 2.03 Tc(MIN.) = 30.13
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5210.00 = 4895.00 FEET.

 FLOW PROCESS FROM NODE 5210.00 TO NODE 5210.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 30.13
 RAINFALL INTENSITY(INCH/HR) = 1.81
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 74.12
 TOTAL STREAM AREA(ACRES) = 74.12
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 119.70

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	129.36	27.81	1.904	0.98(0.10)	0.10	79.6	5120.00
2	119.70	30.13	1.814	0.98(0.10)	0.10	74.1	5170.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	245.60	27.81	1.904	0.98(0.10)	0.10	148.0	5120.00
2	242.66	30.13	1.814	0.98(0.10)	0.10	153.7	5170.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 245.60 Tc(MIN.) = 27.81
 EFFECTIVE AREA(ACRES) = 147.99 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 153.7
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5210.00 = 4895.00 FEET.

 FLOW PROCESS FROM NODE 5210.00 TO NODE 5215.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 728.80 DOWNSTREAM(FEET) = 721.08
 FLOW LENGTH(FEET) = 420.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 54.0 INCH PIPE IS 42.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.12
 ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 245.60
 PIPE TRAVEL TIME(MIN.) = 0.39 Tc(MIN.) = 28.19
 LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5215.00 = 5315.00 FEET.

 FLOW PROCESS FROM NODE 5215.00 TO NODE 5215.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	245.60	28.19	1.888	0.98(0.10)	0.10	148.0	5120.00
2	242.66	30.51	1.800	0.98(0.10)	0.10	153.7	5170.00

LONGEST FLOWPATH FROM NODE 5170.00 TO NODE 5215.00 = 5315.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	449.78	15.52	2.701	0.97(0.31)	0.32	191.6	5043.00
2	473.06	17.93	2.477	0.98(0.30)	0.31	224.0	5080.00
3	473.06	17.94	2.477	0.97(0.30)	0.31	224.0	5050.00
4	472.84	18.49	2.432	0.97(0.30)	0.30	228.7	5142.00
5	448.54	22.93	2.137	0.97(0.28)	0.29	253.6	5025.00
6	433.09	24.26	2.066	0.98(0.28)	0.29	256.5	5070.00
7	367.43	33.16	1.713	0.98(0.27)	0.28	273.8	5000.00

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5215.00 = 9970.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	646.41	15.52	2.701	0.97(0.24)	0.25	273.1	5043.00
2	680.66	17.93	2.477	0.98(0.24)	0.24	318.2	5080.00
3	680.67	17.94	2.477	0.98(0.24)	0.24	318.2	5050.00
4	682.84	18.49	2.432	0.98(0.24)	0.24	325.8	5142.00
5	676.11	22.93	2.137	0.97(0.22)	0.23	374.0	5025.00

6 665.47 24.26 2.066 0.98(0.22) 0.22 383.8 5070.00
 7 649.69 28.19 1.888 0.98(0.21) 0.22 412.1 5120.00
 8 629.62 30.51 1.800 0.98(0.21) 0.22 422.4 5170.00
 9 597.62 33.16 1.713 0.98(0.21) 0.22 427.5 5000.00
 TOTAL AREA(ACRES) = 427.5

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 682.84 Tc(MIN.) = 18.491
 EFFECTIVE AREA(ACRES) = 325.77 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.22
 TOTAL AREA(ACRES) = 427.5
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5215.00 = 9970.00 FEET.

FLOW PROCESS FROM NODE 5215.00 TO NODE 5215.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 5215.00 TO NODE 5248.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 720.42 DOWNSTREAM(FEET) = 708.36
 FLOW LENGTH(FEET) = 1194.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 90.0 INCH PIPE IS 68.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.83
 ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 682.84
 PIPE TRAVEL TIME(MIN.) = 1.06 Tc(MIN.) = 19.55
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5248.00 = 11164.00 FEET.

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 19.55
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.352
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.92	0.98	0.100	32
PUBLIC PARK	A	0.76	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.439
 SUBAREA AREA(ACRES) = 1.68 SUBAREA RUNOFF(CFS) = 2.91
 EFFECTIVE AREA(ACRES) = 327.45 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.24
 TOTAL AREA(ACRES) = 429.2 PEAK FLOW RATE(CFS) = 682.84
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 19.55
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.352
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	1.10	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 1.10 SUBAREA RUNOFF(CFS) = 2.23
 EFFECTIVE AREA(ACRES) = 328.55 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.24
 TOTAL AREA(ACRES) = 430.3 PEAK FLOW RATE(CFS) = 682.84
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 19.55
 RAINFALL INTENSITY(INCH/HR) = 2.35
 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.97
 AREA-AVERAGED Ap = 0.24
 EFFECTIVE STREAM AREA(ACRES) = 328.55
 TOTAL STREAM AREA(ACRES) = 430.29
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 682.84

FLOW PROCESS FROM NODE 5220.00 TO NODE 5225.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 614.00
 ELEVATION DATA: UPSTREAM(FEET) = 751.70 DOWNSTREAM(FEET) = 750.20
 $Tc = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 14.068
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.865
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL "11+ DWELLINGS/ACRE"	A	4.53	0.98	0.200	32	14.07

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA RUNOFF(CFS) = 10.89
 TOTAL AREA(ACRES) = 4.53 PEAK FLOW RATE(CFS) = 10.89

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*****
FLOW PROCESS FROM NODE 5225.00 TO NODE 5230.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 750.20 DOWNSTREAM ELEVATION(FEET) = 745.90
STREET LENGTH(FEET) = 345.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 27.60
***STREET FLOWING FULL***
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.53
HALFSTREET FLOOD WIDTH(FEET) = 18.00
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.75
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 2.00
STREET FLOW TRAVEL TIME(MIN.) = 1.53 Tc(MIN.) = 15.60
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.693
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE" A 14.86 0.98 0.200 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA(ACRES) = 14.86 SUBAREA RUNOFF(CFS) = 33.40
EFFECTIVE AREA(ACRES) = 19.39 AREA-AVERAGED Fm(INCH/HR) = 0.20
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.20
TOTAL AREA(ACRES) = 19.4 PEAK FLOW RATE(CFS) = 43.59

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.60 HALFSTREET FLOOD WIDTH(FEET) = 18.00
FLOW VELOCITY(FEET/SEC.) = 4.50 DEPTH*VELOCITY(FT*FT/SEC.) = 2.68
LONGEST FLOWPATH FROM NODE 5220.00 TO NODE 5230.00 = 959.00 FEET.

*****
FLOW PROCESS FROM NODE 5230.00 TO NODE 5235.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 738.90 DOWNSTREAM(FEET) = 738.10
FLOW LENGTH(FEET) = 265.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.2 INCHES

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PIPE-FLOW VELOCITY(FEET/SEC.) = 6.10
ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 43.59
PIPE TRAVEL TIME(MIN.) = 0.72 Tc(MIN.) = 16.33
LONGEST FLOWPATH FROM NODE 5220.00 TO NODE 5235.00 = 1224.00 FEET.

*****
FLOW PROCESS FROM NODE 5235.00 TO NODE 5235.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 16.33
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.620
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK A 7.86 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
SUBAREA AREA(ACRES) = 7.86 SUBAREA RUNOFF(CFS) = 12.67
EFFECTIVE AREA(ACRES) = 27.25 AREA-AVERAGED Fm(INCH/HR) = 0.38
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.39
TOTAL AREA(ACRES) = 27.2 PEAK FLOW RATE(CFS) = 55.00

*****
FLOW PROCESS FROM NODE 5235.00 TO NODE 5240.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 738.10 DOWNSTREAM(FEET) = 733.00
FLOW LENGTH(FEET) = 514.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 26.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.05
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 55.00
PIPE TRAVEL TIME(MIN.) = 0.85 Tc(MIN.) = 17.18
LONGEST FLOWPATH FROM NODE 5220.00 TO NODE 5240.00 = 1738.00 FEET.

*****
FLOW PROCESS FROM NODE 5240.00 TO NODE 5240.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 17.18
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.541
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 5.55 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 5.55 SUBAREA RUNOFF(CFS) = 12.21
EFFECTIVE AREA(ACRES) = 32.80 AREA-AVERAGED Fm(INCH/HR) = 0.33
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.34

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TOTAL AREA(ACRES) = 32.8 PEAK FLOW RATE(CFS) = 65.27

FLOW PROCESS FROM NODE 5240.00 TO NODE 5247.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 733.00 DOWNSTREAM(FEET) = 717.05
FLOW LENGTH(FEET) = 800.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 25.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.53
ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 65.27
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 18.16
LONGEST FLOWPATH FROM NODE 5220.00 TO NODE 5247.00 = 2538.00 FEET.

FLOW PROCESS FROM NODE 5247.00 TO NODE 5247.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 18.16
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.458
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 15.35 0.98 0.100 32
PUBLIC PARK A 3.86 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.251
SUBAREA AREA(ACRES) = 19.21 SUBAREA RUNOFF(CFS) = 38.27
EFFECTIVE AREA(ACRES) = 52.01 AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.31
TOTAL AREA(ACRES) = 52.0 PEAK FLOW RATE(CFS) = 101.07

FLOW PROCESS FROM NODE 5247.00 TO NODE 5248.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 717.05 DOWNSTREAM(FEET) = 710.66
FLOW LENGTH(FEET) = 950.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 35.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.07
ESTIMATED PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 101.07
PIPE TRAVEL TIME(MIN.) = 1.57 Tc(MIN.) = 19.74
LONGEST FLOWPATH FROM NODE 5220.00 TO NODE 5248.00 = 3488.00 FEET.

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 19.74
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.338
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 1.56 0.98 0.100 32
PUBLIC PARK A 1.20 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.426
SUBAREA AREA(ACRES) = 2.76 SUBAREA RUNOFF(CFS) = 4.78
EFFECTIVE AREA(ACRES) = 54.77 AREA-AVERAGED Fm(INCH/HR) = 0.30
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.31
TOTAL AREA(ACRES) = 54.8 PEAK FLOW RATE(CFS) = 101.07
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 19.74
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.338
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 1.56 0.98 0.100 32
PUBLIC PARK A 1.20 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.426
SUBAREA AREA(ACRES) = 2.76 SUBAREA RUNOFF(CFS) = 4.78
EFFECTIVE AREA(ACRES) = 57.53 AREA-AVERAGED Fm(INCH/HR) = 0.31
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.32
TOTAL AREA(ACRES) = 57.5 PEAK FLOW RATE(CFS) = 105.03

FLOW PROCESS FROM NODE 5248.00 TO NODE 5248.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 19.74
RAINFALL INTENSITY(INCH/HR) = 2.34
AREA-AVERAGED Fm(INCH/HR) = 0.31
AREA-AVERAGED Fp(INCH/HR) = 0.97
AREA-AVERAGED Ap = 0.32
EFFECTIVE STREAM AREA(ACRES) = 57.53
TOTAL STREAM AREA(ACRES) = 57.53
PEAK FLOW RATE(CFS) AT CONFLUENCE = 105.03

** CONFLUENCE DATA **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE

1	646.41	16.60	2.594	0.97(0.25)	0.25	275.9	5043.00
1	680.66	18.99	2.393	0.98(0.24)	0.25	321.0	5080.00
1	680.67	18.99	2.393	0.98(0.24)	0.25	321.0	5050.00
1	682.84	19.55	2.352	0.97(0.24)	0.24	328.6	5142.00
1	676.11	23.99	2.080	0.97(0.22)	0.23	376.8	5025.00
1	665.47	25.34	2.013	0.97(0.22)	0.22	386.6	5070.00
1	649.69	29.27	1.846	0.98(0.21)	0.22	414.9	5120.00
1	629.62	31.59	1.763	0.98(0.21)	0.22	425.1	5170.00
1	597.62	34.26	1.680	0.98(0.21)	0.22	430.3	5000.00
2	105.03	19.74	2.338	0.97(0.31)	0.32	57.5	5220.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	745.89	16.60	2.594	0.97(0.26)	0.26	324.3	5043.00
2	784.45	18.99	2.393	0.98(0.25)	0.26	376.3	5080.00
3	784.46	18.99	2.393	0.97(0.25)	0.26	376.3	5050.00
4	787.56	19.55	2.352	0.97(0.25)	0.25	385.5	5142.00
5	787.58	19.74	2.338	0.98(0.25)	0.25	388.1	5220.00
6	767.75	23.99	2.080	0.97(0.23)	0.24	434.3	5025.00
7	753.64	25.34	2.013	0.97(0.23)	0.24	444.2	5070.00
8	729.23	29.27	1.846	0.98(0.22)	0.23	472.4	5120.00
9	704.87	31.59	1.763	0.98(0.22)	0.23	482.7	5170.00
10	668.54	34.26	1.680	0.98(0.22)	0.23	487.8	5000.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 787.58 Tc(MIN.) = 19.74
EFFECTIVE AREA(ACRES) = 388.13 AREA-AVERAGED Fm(INCH/HR) = 0.25
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.25
TOTAL AREA(ACRES) = 487.8
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5248.00 = 11164.00 FEET.

FLOW PROCESS FROM NODE 5248.00 TO NODE 5300.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 708.36 DOWNSTREAM(FEET) = 707.99
FLOW LENGTH(FEET) = 31.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 90.0 INCH PIPE IS 72.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 20.54
ESTIMATED PIPE DIAMETER(INCH) = 90.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 787.58
PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 19.76
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5300.00 = 11195.00 FEET.

FLOW PROCESS FROM NODE 5300.00 TO NODE 5300.00 IS CODE = 10

>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 5273.00 TO NODE 5274.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 274.00
ELEVATION DATA: UPSTREAM(FEET) = 737.00 DOWNSTREAM(FEET) = 735.88

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.623
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.843
SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	0.46	0.98	0.100	32	8.62
PUBLIC PARK	A	0.35	0.98	0.850	32	13.70

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.424
SUBAREA RUNOFF(CFS) = 2.50
TOTAL AREA(ACRES) = 0.81 PEAK FLOW RATE(CFS) = 2.50

FLOW PROCESS FROM NODE 5274.00 TO NODE 5274.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 8.62
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.843
SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.46	0.98	0.100	32
PUBLIC PARK	A	0.35	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.424
SUBAREA AREA(ACRES) = 0.81 SUBAREA RUNOFF(CFS) = 2.50
EFFECTIVE AREA(ACRES) = 1.62 AREA-AVERAGED Fm(INCH/HR) = 0.41
AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.42
TOTAL AREA(ACRES) = 1.6 PEAK FLOW RATE(CFS) = 5.00

FLOW PROCESS FROM NODE 5274.00 TO NODE 5275.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 725.22 DOWNSTREAM(FEET) = 722.55
FLOW LENGTH(FEET) = 510.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 4.41
ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 5.00
PIPE TRAVEL TIME(MIN.) = 1.93 Tc(MIN.) = 10.55

LONGEST FLOWPATH FROM NODE 5273.00 TO NODE 5275.00 = 784.00 FEET.

FLOW PROCESS FROM NODE 5275.00 TO NODE 5275.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 10.55
RAINFALL INTENSITY(INCH/HR) = 3.40
AREA-AVERAGED Fm(INCH/HR) = 0.41
AREA-AVERAGED Fp(INCH/HR) = 0.97
AREA-AVERAGED Ap = 0.42
EFFECTIVE STREAM AREA(ACRES) = 1.62
TOTAL STREAM AREA(ACRES) = 1.62
PEAK FLOW RATE(CFS) AT CONFLUENCE = 5.00

FLOW PROCESS FROM NODE 5250.00 TO NODE 5255.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 502.00
ELEVATION DATA: UPSTREAM(FEET) = 747.50 DOWNSTREAM(FEET) = 745.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.408
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.249
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
COMMERCIAL A 3.72 0.98 0.100 32 11.41
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA RUNOFF(CFS) = 10.55
TOTAL AREA(ACRES) = 3.72 PEAK FLOW RATE(CFS) = 10.55

FLOW PROCESS FROM NODE 5255.00 TO NODE 5260.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<

=====

UPSTREAM ELEVATION(FEET) = 745.80 DOWNSTREAM ELEVATION(FEET) = 741.80
STREET LENGTH(FEET) = 615.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 23.06

STREET FLOWING FULL

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.55
HALFSTREET FLOOD WIDTH(FEET) = 18.00
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.87
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.58
STREET FLOW TRAVEL TIME(MIN.) = 3.57 Tc(MIN.) = 14.97
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.760

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 10.38 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
SUBAREA AREA(ACRES) = 10.38 SUBAREA RUNOFF(CFS) = 24.87
EFFECTIVE AREA(ACRES) = 14.10 AREA-AVERAGED Fm(INCH/HR) = 0.10
AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
TOTAL AREA(ACRES) = 14.1 PEAK FLOW RATE(CFS) = 33.78

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.61 HALFSTREET FLOOD WIDTH(FEET) = 18.00
FLOW VELOCITY(FEET/SEC.) = 3.35 DEPTH*VELOCITY(FT*FT/SEC.) = 2.04
LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5260.00 = 1117.00 FEET.

FLOW PROCESS FROM NODE 5260.00 TO NODE 5265.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 734.40 DOWNSTREAM(FEET) = 733.30
FLOW LENGTH(FEET) = 364.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 28.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.61
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 33.78
PIPE TRAVEL TIME(MIN.) = 1.08 Tc(MIN.) = 16.06
LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5265.00 = 1481.00 FEET.

FLOW PROCESS FROM NODE 5265.00 TO NODE 5265.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 16.06
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.647
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 6.97 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 6.97 SUBAREA RUNOFF(CFS) = 15.99
 EFFECTIVE AREA(ACRES) = 21.07 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 21.1 PEAK FLOW RATE(CFS) = 48.34

 FLOW PROCESS FROM NODE 5265.00 TO NODE 5270.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 733.30 DOWNSTREAM(FEET) = 732.60
 FLOW LENGTH(FEET) = 183.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 39.0 INCH PIPE IS 31.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.66
 ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 48.34
 PIPE TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 16.51
 LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5270.00 = 1664.00 FEET.

 FLOW PROCESS FROM NODE 5270.00 TO NODE 5270.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 16.51
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.602
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 7.85 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 7.85 SUBAREA RUNOFF(CFS) = 17.70
 EFFECTIVE AREA(ACRES) = 28.92 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.10
 TOTAL AREA(ACRES) = 28.9 PEAK FLOW RATE(CFS) = 65.19

 FLOW PROCESS FROM NODE 5270.00 TO NODE 5275.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 732.60 DOWNSTREAM(FEET) = 722.55
 FLOW LENGTH(FEET) = 322.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 15.93
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 65.19
 PIPE TRAVEL TIME(MIN.) = 0.34 Tc(MIN.) = 16.85
 LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5275.00 = 1986.00 FEET.

FLOW PROCESS FROM NODE 5275.00 TO NODE 5275.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 16.85
 RAINFALL INTENSITY(INCH/HR) = 2.57
 AREA-AVERAGED Fm(INCH/HR) = 0.10
 AREA-AVERAGED Fp(INCH/HR) = 0.98
 AREA-AVERAGED Ap = 0.10
 EFFECTIVE STREAM AREA(ACRES) = 28.92
 TOTAL STREAM AREA(ACRES) = 28.92
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 65.19

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	5.00	10.55	3.405	0.97(0.41)	0.42	1.6	5273.00
2	65.19	16.85	2.571	0.98(0.10)	0.10	28.9	5250.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	59.58	10.55	3.405	0.98(0.12)	0.13	19.7	5273.00
2	68.80	16.85	2.571	0.98(0.11)	0.12	30.5	5250.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 68.80 Tc(MIN.) = 16.85
 EFFECTIVE AREA(ACRES) = 30.54 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.12
 TOTAL AREA(ACRES) = 30.5
 LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5275.00 = 1986.00 FEET.

 FLOW PROCESS FROM NODE 5275.00 TO NODE 5295.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 722.55 DOWNSTREAM(FEET) = 717.86
 FLOW LENGTH(FEET) = 722.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 31.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.04
 ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 68.80
 PIPE TRAVEL TIME(MIN.) = 1.33 Tc(MIN.) = 18.18
 LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5295.00 = 2708.00 FEET.

 FLOW PROCESS FROM NODE 5295.00 TO NODE 5295.00 IS CODE = 1

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>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 18.18
RAINFALL INTENSITY(INCH/HR) = 2.46
AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.98
AREA-AVERAGED Ap = 0.12
EFFECTIVE STREAM AREA(ACRES) = 30.54
TOTAL STREAM AREA(ACRES) = 30.54
PEAK FLOW RATE(CFS) AT CONFLUENCE = 68.80

*****
FLOW PROCESS FROM NODE 5280.00 TO NODE 5285.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 556.00
ELEVATION DATA: UPSTREAM(FEET) = 737.20 DOWNSTREAM(FEET) = 736.30

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.774
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.902
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
PUBLIC PARK A 1.60 0.98 0.850 32 21.88
COMMERCIAL A 5.83 0.98 0.100 32 13.77
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.262
SUBAREA RUNOFF(CFS) = 17.70
TOTAL AREA(ACRES) = 7.43 PEAK FLOW RATE(CFS) = 17.70

*****
FLOW PROCESS FROM NODE 5285.00 TO NODE 5290.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 736.30 DOWNSTREAM(FEET) = 728.30
FLOW LENGTH(FEET) = 347.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.48
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 17.70
PIPE TRAVEL TIME(MIN.) = 0.55 Tc(MIN.) = 14.33
LONGEST FLOWPATH FROM NODE 5280.00 TO NODE 5290.00 = 903.00 FEET.

*****
FLOW PROCESS FROM NODE 5290.00 TO NODE 5290.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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=====
MAINLINE Tc(MIN.) = 14.33
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.834
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL C 4.80 0.57 0.100 69
PUBLIC PARK A 7.67 0.98 0.850 32
COMMERCIAL A 8.90 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.95
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.369
SUBAREA AREA(ACRES) = 21.37 SUBAREA RUNOFF(CFS) = 47.76
EFFECTIVE AREA(ACRES) = 28.80 AREA-AVERAGED Fm(INCH/HR) = 0.33
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.34
TOTAL AREA(ACRES) = 28.8 PEAK FLOW RATE(CFS) = 65.01

*****
FLOW PROCESS FROM NODE 5290.00 TO NODE 5295.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 728.30 DOWNSTREAM(FEET) = 724.50
FLOW LENGTH(FEET) = 374.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 27.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.63
ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 65.01
PIPE TRAVEL TIME(MIN.) = 0.59 Tc(MIN.) = 14.91
LONGEST FLOWPATH FROM NODE 5280.00 TO NODE 5295.00 = 1277.00 FEET.

*****
FLOW PROCESS FROM NODE 5295.00 TO NODE 5295.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 14.91
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.767
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
PUBLIC PARK C 0.32 0.57 0.850 69
COMMERCIAL C 3.80 0.57 0.100 69
COMMERCIAL A 6.20 0.98 0.100 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.77
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.123
SUBAREA AREA(ACRES) = 10.32 SUBAREA RUNOFF(CFS) = 24.82
EFFECTIVE AREA(ACRES) = 39.12 AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.93 AREA-AVERAGED Ap = 0.28
TOTAL AREA(ACRES) = 39.1 PEAK FLOW RATE(CFS) = 88.08

*****
FLOW PROCESS FROM NODE 5295.00 TO NODE 5295.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

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>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 14.91
RAINFALL INTENSITY(INCH/HR) = 2.77
AREA-AVERAGED Fm(INCH/HR) = 0.26
AREA-AVERAGED Fp(INCH/HR) = 0.93
AREA-AVERAGED Ap = 0.28
EFFECTIVE STREAM AREA(ACRES) = 39.12
TOTAL STREAM AREA(ACRES) = 39.12
PEAK FLOW RATE(CFS) AT CONFLUENCE = 88.08

** CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1, 1, 2.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1, 2, 3.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 152.05 Tc(MIN.) = 14.91
EFFECTIVE AREA(ACRES) = 63.99 AREA-AVERAGED Fm(INCH/HR) = 0.21
AREA-AVERAGED Fp(INCH/HR) = 0.94 AREA-AVERAGED Ap = 0.22
TOTAL AREA(ACRES) = 69.7
LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5295.00 = 2708.00 FEET.

FLOW PROCESS FROM NODE 5295.00 TO NODE 5300.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 717.86 DOWNSTREAM(FEET) = 710.30
FLOW LENGTH(FEET) = 921.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 40.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.03
ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 152.05
PIPE TRAVEL TIME(MIN.) = 1.28 Tc(MIN.) = 16.19
LONGEST FLOWPATH FROM NODE 5250.00 TO NODE 5300.00 = 3629.00 FEET.

FLOW PROCESS FROM NODE 5300.00 TO NODE 5300.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 16.19
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.634
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL C 1.50 0.57 0.100 69
PUBLIC PARK C 1.16 0.57 0.850 69
COMMERCIAL A 1.58 0.98 0.100 32
PUBLIC PARK A 1.22 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.78
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.427
SUBAREA AREA(ACRES) = 5.46 SUBAREA RUNOFF(CFS) = 11.31
EFFECTIVE AREA(ACRES) = 69.45 AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.92 AREA-AVERAGED Ap = 0.24
TOTAL AREA(ACRES) = 75.1 PEAK FLOW RATE(CFS) = 152.05
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5300.00 TO NODE 5300.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 16.19
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.634
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL C 1.46 0.57 0.100 69
PUBLIC PARK C 1.12 0.57 0.850 69
COMMERCIAL A 1.64 0.98 0.100 32
PUBLIC PARK A 1.26 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.78
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.426
SUBAREA AREA(ACRES) = 5.48 SUBAREA RUNOFF(CFS) = 11.35
EFFECTIVE AREA(ACRES) = 74.93 AREA-AVERAGED Fm(INCH/HR) = 0.23
AREA-AVERAGED Fp(INCH/HR) = 0.90 AREA-AVERAGED Ap = 0.25
TOTAL AREA(ACRES) = 80.6 PEAK FLOW RATE(CFS) = 162.35

FLOW PROCESS FROM NODE 5300.00 TO NODE 5300.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

Table with 8 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1, 2, 3.

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	745.89	16.63	2.592	0.97(0.26)	0.26	324.3	5043.00
2	784.45	19.02	2.391	0.98(0.25)	0.26	376.3	5080.00
3	784.46	19.02	2.391	0.97(0.25)	0.26	376.3	5050.00
4	787.56	19.57	2.350	0.97(0.25)	0.25	385.5	5142.00
5	787.58	19.76	2.337	0.98(0.25)	0.25	388.1	5220.00
6	767.75	24.02	2.079	0.97(0.23)	0.24	434.3	5025.00
7	753.64	25.37	2.011	0.97(0.23)	0.24	444.2	5070.00
8	729.23	29.29	1.845	0.98(0.22)	0.23	472.4	5120.00
9	704.87	31.62	1.762	0.98(0.22)	0.23	482.7	5170.00
10	668.54	34.29	1.679	0.98(0.22)	0.23	487.8	5000.00

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5300.00 = 11195.00 FEET.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	843.48	13.26	2.969	0.96(0.25)	0.26	320.6	5273.00
2	901.58	16.19	2.634	0.96(0.25)	0.26	390.6	5280.00
3	907.30	16.63	2.592	0.96(0.25)	0.26	400.0	5043.00
4	940.71	19.02	2.391	0.96(0.24)	0.25	456.1	5080.00
5	940.72	19.02	2.391	0.96(0.24)	0.25	456.2	5050.00
6	942.26	19.47	2.358	0.96(0.24)	0.25	464.4	5250.00
7	942.29	19.57	2.350	0.96(0.24)	0.25	466.1	5142.00
8	941.34	19.76	2.337	0.96(0.24)	0.25	468.7	5220.00
9	902.79	24.02	2.079	0.96(0.23)	0.24	514.9	5025.00
10	883.81	25.37	2.011	0.96(0.23)	0.24	524.8	5070.00
11	847.33	29.29	1.845	0.96(0.22)	0.23	553.0	5120.00
12	816.98	31.62	1.762	0.96(0.22)	0.23	563.3	5170.00
13	774.58	34.29	1.679	0.96(0.22)	0.23	568.4	5000.00

TOTAL AREA(ACRES) = 568.4

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 942.29 Tc(MIN.) = 19.573
EFFECTIVE AREA(ACRES) = 466.13 AREA-AVERAGED Fm(INCH/HR) = 0.24
AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.25
TOTAL AREA(ACRES) = 568.4
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5300.00 = 11195.00 FEET.

FLOW PROCESS FROM NODE 5300.00 TO NODE 5300.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 5300.00 TO NODE 5305.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 707.90 DOWNSTREAM(FEET) = 704.64

FLOW LENGTH(FEET) = 652.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 114.0 INCH PIPE IS 90.9 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 15.56

ESTIMATED PIPE DIAMETER(INCH) = 114.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 942.29
PIPE TRAVEL TIME(MIN.) = 0.70 Tc(MIN.) = 20.27
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5305.00 = 11847.00 FEET.

FLOW PROCESS FROM NODE 5305.00 TO NODE 5305.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 20.27

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.301

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	9.55	0.57	0.100	69
COMMERCIAL	A	11.87	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.79

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100

SUBAREA AREA(ACRES) = 21.42 SUBAREA RUNOFF(CFS) = 42.83

EFFECTIVE AREA(ACRES) = 487.55 AREA-AVERAGED Fm(INCH/HR) = 0.24

AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.24

TOTAL AREA(ACRES) = 589.8 PEAK FLOW RATE(CFS) = 942.29

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5305.00 TO NODE 5305.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 20.27

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.301

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	0.54	0.57	0.100	69
PUBLIC PARK	C	0.41	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.424

SUBAREA AREA(ACRES) = 0.95 SUBAREA RUNOFF(CFS) = 1.76

EFFECTIVE AREA(ACRES) = 488.50 AREA-AVERAGED Fm(INCH/HR) = 0.24

AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.25

TOTAL AREA(ACRES) = 590.8 PEAK FLOW RATE(CFS) = 942.29

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5305.00 TO NODE 5330.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 704.64 DOWNSTREAM(FEET) = 701.26

FLOW LENGTH(FEET) = 675.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 114.0 INCH PIPE IS 90.8 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 15.57

ESTIMATED PIPE DIAMETER(INCH) = 114.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 942.29
 PIPE TRAVEL TIME(MIN.) = 0.72 Tc(MIN.) = 20.99
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5330.00 = 12522.00 FEET.

FLOW PROCESS FROM NODE 5330.00 TO NODE 5330.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 20.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.253
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	0.54	0.57	0.100	69
PUBLIC PARK	C	0.41	0.57	0.850	69
COMMERCIAL	A	0.51	0.98	0.100	32
PUBLIC PARK	A	0.48	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.78
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.444
 SUBAREA AREA(ACRES) = 1.94 SUBAREA RUNOFF(CFS) = 3.33
 EFFECTIVE AREA(ACRES) = 490.44 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.25
 TOTAL AREA(ACRES) = 592.7 PEAK FLOW RATE(CFS) = 942.29
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5330.00 TO NODE 5330.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 20.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.253
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.51	0.98	0.100	32
PUBLIC PARK	A	0.48	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.464
 SUBAREA AREA(ACRES) = 0.99 SUBAREA RUNOFF(CFS) = 1.60
 EFFECTIVE AREA(ACRES) = 491.43 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.25
 TOTAL AREA(ACRES) = 593.7 PEAK FLOW RATE(CFS) = 942.29
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5330.00 TO NODE 5330.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 20.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.253
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	23.78	0.98	0.100	32
COMMERCIAL	C	5.56	0.57	0.100	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.90
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 29.34 SUBAREA RUNOFF(CFS) = 57.13
 EFFECTIVE AREA(ACRES) = 520.77 AREA-AVERAGED Fm(INCH/HR) = 0.23
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.24
 TOTAL AREA(ACRES) = 623.1 PEAK FLOW RATE(CFS) = 949.30

FLOW PROCESS FROM NODE 5330.00 TO NODE 5330.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 20.99
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.253
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	32.80	0.98	0.100	32
COMMERCIAL	C	4.08	0.57	0.100	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.93
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 36.88 SUBAREA RUNOFF(CFS) = 71.70
 EFFECTIVE AREA(ACRES) = 557.65 AREA-AVERAGED Fm(INCH/HR) = 0.22
 AREA-AVERAGED Fp(INCH/HR) = 0.96 AREA-AVERAGED Ap = 0.23
 TOTAL AREA(ACRES) = 659.9 PEAK FLOW RATE(CFS) = 1021.01

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	954.61	14.70	2.791	0.95(0.22)	0.23	412.1	5273.00
2	990.46	17.61	2.504	0.95(0.22)	0.23	482.2	5280.00
3	993.22	18.05	2.467	0.95(0.22)	0.23	491.5	5043.00
4	1020.30	20.44	2.290	0.96(0.22)	0.23	547.7	5080.00
5	1020.30	20.44	2.290	0.96(0.22)	0.23	547.7	5050.00
6	1021.15	20.89	2.260	0.96(0.22)	0.23	555.9	5250.00
7	1021.01	20.99	2.253	0.96(0.22)	0.23	557.7	5142.00
8	1019.80	21.18	2.241	0.96(0.22)	0.23	560.2	5220.00
9	980.89	25.44	2.008	0.96(0.21)	0.22	606.4	5025.00
10	963.41	26.80	1.946	0.96(0.21)	0.22	616.3	5070.00
11	920.53	30.73	1.793	0.96(0.21)	0.22	644.5	5120.00
12	889.79	33.09	1.715	0.96(0.20)	0.21	654.8	5170.00
13	850.40	35.77	1.637	0.96(0.21)	0.21	659.9	5000.00

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 1021.15 Tc(MIN.) = 20.89
 AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.96
 AREA-AVERAGED Ap = 0.23 EFFECTIVE AREA(ACRES) = 555.88

FLOW PROCESS FROM NODE 5330.00 TO NODE 5330.00 IS CODE = 7

>>>>PEAK FLOW RATE ESTIMATOR CHANGED TO UNIT-HYDROGRAPH METHOD<<<<<

```
>>>>USING TIME-OF-CONCENTRATION OF LONGEST FLOWPATH<<<<<
=====
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
          MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.60; LAG(HR) = 0.48; Fm(INCH/HR) = 0.21; Ybar = 0.24
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
3HR = 1.00; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 659.9
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5330.00 = 12522.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0337; Lca/L=0.4,n=.0302; Lca/L=0.5,n=.0277;Lca/L=0.6,n=.0259
TIME OF PEAK FLOW(HR) = 16.50 RUNOFF VOLUME(AF) = 257.40
UNIT-HYDROGRAPH METHOD PEAK FLOW RATE(CFS) = 914.56
TOTAL PEAK FLOW RATE(CFS) = 914.56 (SOURCE FLOW INCLUDED)
RATIONAL METHOD PEAK FLOW RATE(CFS) = 1021.15
(UPSTREAM NODE PEAK FLOW RATE(CFS) = 1021.15)
PEAK FLOW RATE(CFS) USED = 1021.15
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*****
FLOW PROCESS FROM NODE 5330.00 TO NODE 5335.00 IS CODE = 31
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```
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
```

```
ELEVATION DATA: UPSTREAM(FEET) = 701.26 DOWNSTREAM(FEET) = 697.78
FLOW LENGTH(FEET) = 687.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 120.0 INCH PIPE IS 90.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.12
ESTIMATED PIPE DIAMETER(INCH) = 120.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 1021.15
PIPE TRAVEL TIME(MIN.) = 0.71 Tc(MIN.) = 36.48
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5335.00 = 13209.00 FEET.
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*****
FLOW PROCESS FROM NODE 5335.00 TO NODE 5335.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
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```
MAINLINE Tc(MIN.) = 36.48
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.618
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 0.54 0.98 0.100 32
PUBLIC PARK A 0.50 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.461
SUBAREA AREA(ACRES) = 1.04
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
          MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
```

```
Tc(HR) = 0.61; LAG(HR) = 0.49; Fm(INCH/HR) = 0.21; Ybar = 0.24
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
3HR = 1.00; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 661.0
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5335.00 = 13209.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0329; Lca/L=0.4,n=.0295; Lca/L=0.5,n=.0271;Lca/L=0.6,n=.0252
TIME OF PEAK FLOW(HR) = 16.50 RUNOFF VOLUME(AF) = 257.69
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 897.46
TOTAL AREA(ACRES) = 661.0 PEAK FLOW RATE(CFS) = 1021.15
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE
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*****
FLOW PROCESS FROM NODE 5335.00 TO NODE 5335.00 IS CODE = 81
-----
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```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
```

```
MAINLINE Tc(MIN.) = 36.48
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.618
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL A 0.54 0.98 0.100 32
PUBLIC PARK A 0.50 0.98 0.850 32
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.461
SUBAREA AREA(ACRES) = 1.04
```

```
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
          MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.61; LAG(HR) = 0.49; Fm(INCH/HR) = 0.21; Ybar = 0.24
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
3HR = 1.00; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 662.0
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5335.00 = 13209.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0329; Lca/L=0.4,n=.0295; Lca/L=0.5,n=.0271;Lca/L=0.6,n=.0252
TIME OF PEAK FLOW(HR) = 16.50 RUNOFF VOLUME(AF) = 257.98
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 898.62
TOTAL AREA(ACRES) = 662.0 PEAK FLOW RATE(CFS) = 1021.15
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE
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*****
FLOW PROCESS FROM NODE 5335.00 TO NODE 5335.00 IS CODE = 81
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
```

```
MAINLINE Tc(MIN.) = 36.48
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.618
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
```

RESIDENTIAL
 "11+ DWELLINGS/ACRE" C 1.40 0.57 0.200 69
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" A 17.88 0.98 0.200 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.95
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 19.28
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.61; LAG(HR) = 0.49; Fm(INCH/HR) = 0.21; Ybar = 0.24
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
 3HR = 1.00; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 681.3
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5335.00 = 13209.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0329; Lca/L=0.4,n=.0295; Lca/L=0.5,n=.0271;Lca/L=0.6,n=.0252
 TIME OF PEAK FLOW(HR) = 16.50 RUNOFF VOLUME(AF) = 265.62
 UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 924.20
 TOTAL AREA(ACRES) = 681.3 PEAK FLOW RATE(CFS) = 1021.15
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5335.00 TO NODE 5375.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 697.79 DOWNSTREAM(FEET) = 682.26
 FLOW LENGTH(FEET) = 1386.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 102.0 INCH PIPE IS 79.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 21.59
 ESTIMATED PIPE DIAMETER(INCH) = 102.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1021.15
 PIPE TRAVEL TIME(MIN.) = 1.07 Tc(MIN.) = 37.55
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5375.00 = 14595.00 FEET.

 FLOW PROCESS FROM NODE 5375.00 TO NODE 5375.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 PEAK FLOW RATE(CFS) = 1021.15 Tc(MIN.) = 37.55
 AREA-AVERAGED Fm(INCH/HR) = 0.21 Ybar = 0.24
 TOTAL AREA(ACRES) = 681.3

 FLOW PROCESS FROM NODE 5355.00 TO NODE 5360.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 410.00
 ELEVATION DATA: UPSTREAM(FEET) = 717.00 DOWNSTREAM(FEET) = 712.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 13.212
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.975
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
PUBLIC PARK	A	2.36	0.98	0.850	32	13.21

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
 SUBAREA RUNOFF(CFS) = 4.56
 TOTAL AREA(ACRES) = 2.36 PEAK FLOW RATE(CFS) = 4.56

 FLOW PROCESS FROM NODE 5360.00 TO NODE 5365.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 712.50 DOWNSTREAM ELEVATION(FEET) = 710.30
 STREET LENGTH(FEET) = 394.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.27
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.43
 HALFSTREET FLOOD WIDTH(FEET) = 13.55
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.04
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.88
 STREET FLOW TRAVEL TIME(MIN.) = 3.22 Tc(MIN.) = 16.43
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.611

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	A	4.61	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850
 SUBAREA AREA(ACRES) = 4.61 SUBAREA RUNOFF(CFS) = 7.39
 EFFECTIVE AREA(ACRES) = 6.97 AREA-AVERAGED Fm(INCH/HR) = 0.83
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.85
 TOTAL AREA(ACRES) = 7.0 PEAK FLOW RATE(CFS) = 11.18

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.46 HALFSTREET FLOOD WIDTH(FEET) = 15.33
 FLOW VELOCITY(FEET/SEC.) = 2.20 DEPTH*VELOCITY(FT*FT/SEC.) = 1.02
 LONGEST FLOWPATH FROM NODE 5355.00 TO NODE 5365.00 = 804.00 FEET.

 FLOW PROCESS FROM NODE 5365.00 TO NODE 5370.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 703.30 DOWNSTREAM(FEET) = 702.40
 FLOW LENGTH(FEET) = 287.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 18.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.34
 ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 11.18
 PIPE TRAVEL TIME(MIN.) = 1.10 Tc(MIN.) = 17.53
 LONGEST FLOWPATH FROM NODE 5355.00 TO NODE 5370.00 = 1091.00 FEET.

 FLOW PROCESS FROM NODE 5370.00 TO NODE 5370.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 17.53
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.511
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	6.55	0.57	0.500	69
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	5.60	0.98	0.500	32

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.75
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 12.15 SUBAREA RUNOFF(CFS) = 23.33
 EFFECTIVE AREA(ACRES) = 19.12 AREA-AVERAGED Fm(INCH/HR) = 0.54
 AREA-AVERAGED Fp(INCH/HR) = 0.86 AREA-AVERAGED Ap = 0.63
 TOTAL AREA(ACRES) = 19.1 PEAK FLOW RATE(CFS) = 33.88

 FLOW PROCESS FROM NODE 5370.00 TO NODE 5375.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 702.40 DOWNSTREAM(FEET) = 685.59
 FLOW LENGTH(FEET) = 917.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.23
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 33.88
 PIPE TRAVEL TIME(MIN.) = 1.36 Tc(MIN.) = 18.89
 LONGEST FLOWPATH FROM NODE 5355.00 TO NODE 5375.00 = 2008.00 FEET.

 FLOW PROCESS FROM NODE 5375.00 TO NODE 5375.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 18.89
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.401
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	9.45	0.57	0.500	69

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 9.45 SUBAREA RUNOFF(CFS) = 18.01
 EFFECTIVE AREA(ACRES) = 28.57 AREA-AVERAGED Fm(INCH/HR) = 0.46
 AREA-AVERAGED Fp(INCH/HR) = 0.78 AREA-AVERAGED Ap = 0.59
 TOTAL AREA(ACRES) = 28.6 PEAK FLOW RATE(CFS) = 50.00

 FLOW PROCESS FROM NODE 5375.00 TO NODE 5375.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 18.89
 RAINFALL INTENSITY(INCH/HR) = 2.40
 AREA-AVERAGED Fm(INCH/HR) = 0.46
 AREA-AVERAGED Fp(INCH/HR) = 0.78
 AREA-AVERAGED Ap = 0.59
 EFFECTIVE STREAM AREA(ACRES) = 28.57
 TOTAL STREAM AREA(ACRES) = 28.57
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 50.00
 ** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	AREA (ACRES)	HEADWATER NODE
1	1021.15	37.55	681.30	5000.00
2	50.00	18.89	28.57	5355.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.63; LAG(HR) = 0.50; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
 3HR = 1.00; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 709.9
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5375.00 = 14595.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0314; Lca/L=0.4,n=.0281; Lca/L=0.5,n=.0258;Lca/L=0.6,n=.0241
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 273.63

PEAK FLOW RATE(CFS) = 949.39
 (UPSTREAM NODE PEAK FLOW RATE(CFS) = 1021.15)
 PEAK FLOW RATE(CFS) USED = 1021.15

 FLOW PROCESS FROM NODE 5375.00 TO NODE 5383.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 682.26 DOWNSTREAM(FEET) = 676.82
 FLOW LENGTH(FEET) = 526.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 102.0 INCH PIPE IS 82.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 20.78
 ESTIMATED PIPE DIAMETER(INCH) = 102.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1021.15
 PIPE TRAVEL TIME(MIN.) = 0.42 Tc(MIN.) = 37.97
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5383.00 = 15121.00 FEET.

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 37.97
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.579
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	1.31	0.57	0.100	69
PUBLIC PARK	C	1.23	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.463
 SUBAREA AREA(ACRES) = 2.54

UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.63; LAG(HR) = 0.51; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
 3HR = 1.00; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 712.4
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5383.00 = 15121.00 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0311; Lca/L=0.4,n=.0279; Lca/L=0.5,n=.0256;Lca/L=0.6,n=.0239
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 274.58

UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 949.92
 TOTAL AREA(ACRES) = 712.4 PEAK FLOW RATE(CFS) = 1021.15
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 37.97
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.579

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	1.31	0.57	0.100	69
PUBLIC PARK	C	1.23	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.463
 SUBAREA AREA(ACRES) = 2.54

UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.63; LAG(HR) = 0.51; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.97; 30M = 0.97; 1HR = 0.97;
 3HR = 1.00; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 715.0
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5383.00 = 15121.00 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0311; Lca/L=0.4,n=.0279; Lca/L=0.5,n=.0256;Lca/L=0.6,n=.0239
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 275.52

UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 953.11
 TOTAL AREA(ACRES) = 715.0 PEAK FLOW RATE(CFS) = 1021.15
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 PEAK FLOW RATE(CFS) = 1021.15 Tc(MIN.) = 37.97
 AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
 TOTAL AREA(ACRES) = 715.0

 FLOW PROCESS FROM NODE 5380.00 TO NODE 5381.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 594.00
 ELEVATION DATA: UPSTREAM(FEET) = 702.90 DOWNSTREAM(FEET) = 700.51

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.789
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.186

SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	0.54	0.98	0.100	32	11.79

PUBLIC PARK A 0.45 0.98 0.850 32 18.73
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.441
 SUBAREA RUNOFF(CFS) = 2.46
 TOTAL AREA(ACRES) = 0.99 PEAK FLOW RATE(CFS) = 2.46

 FLOW PROCESS FROM NODE 5381.00 TO NODE 5381.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 11.79
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.186
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 0.54 0.98 0.100 32
 PUBLIC PARK A 0.45 0.98 0.850 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.441
 SUBAREA AREA(ACRES) = 0.99 SUBAREA RUNOFF(CFS) = 2.46
 EFFECTIVE AREA(ACRES) = 1.98 AREA-AVERAGED Fm(INCH/HR) = 0.43
 AREA-AVERAGED Fp(INCH/HR) = 0.97 AREA-AVERAGED Ap = 0.44
 TOTAL AREA(ACRES) = 2.0 PEAK FLOW RATE(CFS) = 4.91

 FLOW PROCESS FROM NODE 5381.00 TO NODE 5382.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 690.51 DOWNSTREAM(FEET) = 685.38
 FLOW LENGTH(FEET) = 1020.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 11.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 4.32
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 4.91
 PIPE TRAVEL TIME(MIN.) = 3.94 Tc(MIN.) = 15.72
 LONGEST FLOWPATH FROM NODE 5380.00 TO NODE 5382.00 = 1614.00 FEET.

 FLOW PROCESS FROM NODE 5382.00 TO NODE 5382.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 15.72
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.680
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 1.20 0.57 0.100 69
 PUBLIC PARK C 1.00 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.441
 SUBAREA AREA(ACRES) = 2.20 SUBAREA RUNOFF(CFS) = 4.81

EFFECTIVE AREA(ACRES) = 4.18 AREA-AVERAGED Fm(INCH/HR) = 0.33
 AREA-AVERAGED Fp(INCH/HR) = 0.76 AREA-AVERAGED Ap = 0.44
 TOTAL AREA(ACRES) = 4.2 PEAK FLOW RATE(CFS) = 8.82

 FLOW PROCESS FROM NODE 5382.00 TO NODE 5382.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 15.72
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.680
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 1.20 0.57 0.100 69
 PUBLIC PARK C 1.00 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.441
 SUBAREA AREA(ACRES) = 2.20 SUBAREA RUNOFF(CFS) = 4.81
 EFFECTIVE AREA(ACRES) = 6.38 AREA-AVERAGED Fm(INCH/HR) = 0.31
 AREA-AVERAGED Fp(INCH/HR) = 0.69 AREA-AVERAGED Ap = 0.44
 TOTAL AREA(ACRES) = 6.4 PEAK FLOW RATE(CFS) = 13.63

 FLOW PROCESS FROM NODE 5382.00 TO NODE 5383.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 685.38 DOWNSTREAM(FEET) = 680.32
 FLOW LENGTH(FEET) = 770.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.13
 ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 13.63
 PIPE TRAVEL TIME(MIN.) = 2.09 Tc(MIN.) = 17.82
 LONGEST FLOWPATH FROM NODE 5380.00 TO NODE 5383.00 = 2384.00 FEET.

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 17.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.486
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 0.47 0.57 0.100 69
 PUBLIC PARK C 0.39 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.440
 SUBAREA AREA(ACRES) = 0.86 SUBAREA RUNOFF(CFS) = 1.73
 EFFECTIVE AREA(ACRES) = 7.24 AREA-AVERAGED Fm(INCH/HR) = 0.30
 AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.44

TOTAL AREA(ACRES) = 7.2 PEAK FLOW RATE(CFS) = 14.25

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 17.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.486
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 0.47 0.57 0.100 69
 PUBLIC PARK C 0.39 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.440
 SUBAREA AREA(ACRES) = 0.86 SUBAREA RUNOFF(CFS) = 1.73
 EFFECTIVE AREA(ACRES) = 8.10 AREA-AVERAGED Fm(INCH/HR) = 0.29
 AREA-AVERAGED Fp(INCH/HR) = 0.67 AREA-AVERAGED Ap = 0.44
 TOTAL AREA(ACRES) = 8.1 PEAK FLOW RATE(CFS) = 15.98

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 17.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.486
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" A 30.37 0.98 0.200 32
 RESIDENTIAL
 "11+ DWELLINGS/ACRE" C 45.30 0.57 0.200 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.73
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 75.67 SUBAREA RUNOFF(CFS) = 159.38
 EFFECTIVE AREA(ACRES) = 83.77 AREA-AVERAGED Fm(INCH/HR) = 0.16
 AREA-AVERAGED Fp(INCH/HR) = 0.72 AREA-AVERAGED Ap = 0.22
 TOTAL AREA(ACRES) = 83.8 PEAK FLOW RATE(CFS) = 175.36

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5383.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<
 =====
 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 17.82
 RAINFALL INTENSITY(INCH/HR) = 2.49
 AREA-AVERAGED Fm(INCH/HR) = 0.16
 AREA-AVERAGED Fp(INCH/HR) = 0.72
 AREA-AVERAGED Ap = 0.22

EFFECTIVE STREAM AREA(ACRES) = 83.77
 TOTAL STREAM AREA(ACRES) = 83.77
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 175.36
 ** CONFLUENCE DATA **
 STREAM Q Tc AREA HEADWATER
 NUMBER (CFS) (MIN.) (ACRES) NODE
 1 1021.15 37.97 714.95 5000.00
 2 175.36 17.82 83.77 5380.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.63; LAG(HR) = 0.51; Fm(INCH/HR) = 0.21; Ybar = 0.24
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
 3HR = 0.99; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 798.7
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5383.00 = 15121.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0309; Lca/L=0.4,n=.0277; Lca/L=0.5,n=.0255;Lca/L=0.6,n=.0238
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 309.93
 PEAK FLOW RATE(CFS) = 1064.45

 FLOW PROCESS FROM NODE 5383.00 TO NODE 5425.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 676.82 DOWNSTREAM(FEET) = 676.55
 FLOW LENGTH(FEET) = 38.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 114.0 INCH PIPE IS 86.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.47
 ESTIMATED PIPE DIAMETER(INCH) = 114.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1064.45
 PIPE TRAVEL TIME(MIN.) = 0.03 Tc(MIN.) = 38.00
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5425.00 = 15159.00 FEET.

 FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 10

 >>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<
 =====

 FLOW PROCESS FROM NODE 5048.00 TO NODE 5049.00 IS CODE = 21

 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 200.00
 ELEVATION DATA: UPSTREAM(FEET) = 704.37 DOWNSTREAM(FEET) = 703.86

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 8.356
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.916
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	C	0.17	0.57	0.100	69	8.36
PUBLIC PARK	C	0.13	0.57	0.850	69	13.28

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.425
 SUBAREA RUNOFF(CFS) = 0.99
 TOTAL AREA(ACRES) = 0.30 PEAK FLOW RATE(CFS) = 0.99

FLOW PROCESS FROM NODE 5049.00 TO NODE 5049.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 8.36
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.916
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	0.15	0.57	0.100	69
PUBLIC PARK	C	0.12	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.433
 SUBAREA AREA(ACRES) = 0.27 SUBAREA RUNOFF(CFS) = 0.89
 EFFECTIVE AREA(ACRES) = 0.57 AREA-AVERAGED Fm(INCH/HR) = 0.24
 AREA-AVERAGED Fp(INCH/HR) = 0.57 AREA-AVERAGED Ap = 0.43
 TOTAL AREA(ACRES) = 0.6 PEAK FLOW RATE(CFS) = 1.88

FLOW PROCESS FROM NODE 5049.00 TO NODE 5410.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 694.03 DOWNSTREAM(FEET) = 692.13
 FLOW LENGTH(FEET) = 431.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 18.000
 DEPTH OF FLOW IN 18.0 INCH PIPE IS 6.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 3.23
 ESTIMATED PIPE DIAMETER(INCH) = 18.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1.88
 PIPE TRAVEL TIME(MIN.) = 2.23 Tc(MIN.) = 10.58
 LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5410.00 = 631.00 FEET.

FLOW PROCESS FROM NODE 5410.00 TO NODE 5410.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 10.58
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.399

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.27	0.98	0.100	32
PUBLIC PARK	A	0.27	0.98	0.850	32
COMMERCIAL	C	0.27	0.57	0.100	69
PUBLIC PARK	C	0.27	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.77
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.475
 SUBAREA AREA(ACRES) = 1.08 SUBAREA RUNOFF(CFS) = 2.95
 EFFECTIVE AREA(ACRES) = 1.65 AREA-AVERAGED Fm(INCH/HR) = 0.32
 AREA-AVERAGED Fp(INCH/HR) = 0.70 AREA-AVERAGED Ap = 0.46
 TOTAL AREA(ACRES) = 1.7 PEAK FLOW RATE(CFS) = 4.57

FLOW PROCESS FROM NODE 5410.00 TO NODE 5410.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 10.58
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.399
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.27	0.98	0.100	32
PUBLIC PARK	A	0.26	0.98	0.850	32
COMMERCIAL	C	0.26	0.57	0.100	69
PUBLIC PARK	C	0.26	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.77
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.471
 SUBAREA AREA(ACRES) = 1.05 SUBAREA RUNOFF(CFS) = 2.87
 EFFECTIVE AREA(ACRES) = 2.70 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.73 AREA-AVERAGED Ap = 0.46
 TOTAL AREA(ACRES) = 2.7 PEAK FLOW RATE(CFS) = 7.44

FLOW PROCESS FROM NODE 5410.00 TO NODE 5410.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 10.58
 RAINFALL INTENSITY(INCH/HR) = 3.40
 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.73
 AREA-AVERAGED Ap = 0.46
 EFFECTIVE STREAM AREA(ACRES) = 2.70
 TOTAL STREAM AREA(ACRES) = 2.70
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 7.44

FLOW PROCESS FROM NODE 5400.00 TO NODE 5405.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

INITIAL SUBAREA FLOW-LENGTH(FEET) = 295.00
ELEVATION DATA: UPSTREAM(FEET) = 709.30 DOWNSTREAM(FEET) = 707.30

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.879

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.343

SUBAREA Tc AND LOSS RATE DATA(AMC II):

Table with 7 columns: DEVELOPMENT TYPE/ LAND USE, SCS SOIL GROUP, AREA (ACRES), Fp (INCH/HR), Ap (DECIMAL), SCS CN, Tc (MIN.). Row 1: SCHOOL, C, 1.07, 0.57, 0.600, 69, 10.88

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600

SUBAREA RUNOFF(CFS) = 2.89

TOTAL AREA(ACRES) = 1.07 PEAK FLOW RATE(CFS) = 2.89

FLOW PROCESS FROM NODE 5405.00 TO NODE 5410.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>(STREET TABLE SECTION # 1 USED)<<<<

UPSTREAM ELEVATION(FEET) = 707.30 DOWNSTREAM ELEVATION(FEET) = 703.70
STREET LENGTH(FEET) = 240.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 18.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 12.00

INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 6.90

STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:

STREET FLOW DEPTH(FEET) = 0.36

HALFSTREET FLOOD WIDTH(FEET) = 10.08

AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.86

PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.03

STREET FLOW TRAVEL TIME(MIN.) = 1.40 Tc(MIN.) = 12.28

* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.109

SUBAREA LOSS RATE DATA(AMC II):

Table with 7 columns: DEVELOPMENT TYPE/ LAND USE, SCS SOIL GROUP, AREA (ACRES), Fp (INCH/HR), Ap (DECIMAL), SCS CN. Rows: PUBLIC PARK A 0.72 0.98 0.850 32; PUBLIC PARK A 3.17 0.98 0.850 32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.850

SUBAREA AREA(ACRES) = 3.89 SUBAREA RUNOFF(CFS) = 7.98

EFFECTIVE AREA(ACRES) = 4.96 AREA-AVERAGED Fm(INCH/HR) = 0.72

AREA-AVERAGED Fp(INCH/HR) = 0.91 AREA-AVERAGED Ap = 0.80

TOTAL AREA(ACRES) = 5.0 PEAK FLOW RATE(CFS) = 10.65

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.40 HALFSTREET FLOOD WIDTH(FEET) = 12.23

FLOW VELOCITY(FEET/SEC.) = 3.16 DEPTH*VELOCITY(FT*FT/SEC.) = 1.27

LONGEST FLOWPATH FROM NODE 5400.00 TO NODE 5410.00 = 535.00 FEET.

FLOW PROCESS FROM NODE 5410.00 TO NODE 5410.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<

>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<

TOTAL NUMBER OF STREAMS = 2

CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:

TIME OF CONCENTRATION(MIN.) = 12.28

RAINFALL INTENSITY(INCH/HR) = 3.11

AREA-AVERAGED Fm(INCH/HR) = 0.72

AREA-AVERAGED Fp(INCH/HR) = 0.91

AREA-AVERAGED Ap = 0.80

EFFECTIVE STREAM AREA(ACRES) = 4.96

TOTAL STREAM AREA(ACRES) = 4.96

PEAK FLOW RATE(CFS) AT CONFLUENCE = 10.65

** CONFLUENCE DATA **

Table with 9 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1 and 2

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

Table with 9 columns: STREAM NUMBER, Q (CFS), Tc (MIN.), Intensity (INCH/HR), Fp(Fm) (INCH/HR), Ap, Ae (ACRES), HEADWATER NODE. Rows 1 and 2

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 17.73 Tc(MIN.) = 10.58

EFFECTIVE AREA(ACRES) = 6.98 AREA-AVERAGED Fm(INCH/HR) = 0.57

AREA-AVERAGED Fp(INCH/HR) = 0.86 AREA-AVERAGED Ap = 0.67

TOTAL AREA(ACRES) = 7.7

LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5410.00 = 631.00 FEET.

FLOW PROCESS FROM NODE 5410.00 TO NODE 5411.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

ELEVATION DATA: UPSTREAM(FEET) = 692.13 DOWNSTREAM(FEET) = 690.07

FLOW LENGTH(FEET) = 544.00 MANNING'S N = 0.013

DEPTH OF FLOW IN 27.0 INCH PIPE IS 21.7 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 5.18

ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 17.73
 PIPE TRAVEL TIME(MIN.) = 1.75 Tc(MIN.) = 12.33
 LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5411.00 = 1175.00 FEET.

FLOW PROCESS FROM NODE 5411.00 TO NODE 5411.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 12.33
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.101
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.40	0.98	0.100	32
PUBLIC PARK	A	0.25	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.388
 SUBAREA AREA(ACRES) = 0.65 SUBAREA RUNOFF(CFS) = 1.59
 EFFECTIVE AREA(ACRES) = 7.63 AREA-AVERAGED Fm(INCH/HR) = 0.56
 AREA-AVERAGED Fp(INCH/HR) = 0.87 AREA-AVERAGED Ap = 0.64
 TOTAL AREA(ACRES) = 8.3 PEAK FLOW RATE(CFS) = 17.73
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5411.00 TO NODE 5411.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 12.33
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.101
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.41	0.98	0.100	32
PUBLIC PARK	A	0.27	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.398
 SUBAREA AREA(ACRES) = 0.68 SUBAREA RUNOFF(CFS) = 1.66
 EFFECTIVE AREA(ACRES) = 8.31 AREA-AVERAGED Fm(INCH/HR) = 0.54
 AREA-AVERAGED Fp(INCH/HR) = 0.87 AREA-AVERAGED Ap = 0.62
 TOTAL AREA(ACRES) = 9.0 PEAK FLOW RATE(CFS) = 19.11

FLOW PROCESS FROM NODE 5411.00 TO NODE 5411.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 12.33
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.101
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL	A				

"5-7 DWELLINGS/ACRE"	A	1.71	0.98	0.500	32
SCHOOL	A	5.80	0.98	0.600	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.577
 SUBAREA AREA(ACRES) = 7.51 SUBAREA RUNOFF(CFS) = 17.15
 EFFECTIVE AREA(ACRES) = 15.82 AREA-AVERAGED Fm(INCH/HR) = 0.55
 AREA-AVERAGED Fp(INCH/HR) = 0.92 AREA-AVERAGED Ap = 0.60
 TOTAL AREA(ACRES) = 16.5 PEAK FLOW RATE(CFS) = 36.27

FLOW PROCESS FROM NODE 5411.00 TO NODE 5415.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 690.07 DOWNSTREAM(FEET) = 689.61
 FLOW LENGTH(FEET) = 92.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.6 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.05
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 36.27
 PIPE TRAVEL TIME(MIN.) = 0.22 Tc(MIN.) = 12.55
 LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5415.00 = 1267.00 FEET.

FLOW PROCESS FROM NODE 5415.00 TO NODE 5415.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 12.55
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.068
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.21	0.98	0.100	32
PUBLIC PARK	A	0.14	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.400
 SUBAREA AREA(ACRES) = 0.35 SUBAREA RUNOFF(CFS) = 0.84
 EFFECTIVE AREA(ACRES) = 16.17 AREA-AVERAGED Fm(INCH/HR) = 0.55
 AREA-AVERAGED Fp(INCH/HR) = 0.92 AREA-AVERAGED Ap = 0.60
 TOTAL AREA(ACRES) = 16.9 PEAK FLOW RATE(CFS) = 36.65

FLOW PROCESS FROM NODE 5415.00 TO NODE 5415.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

=====

MAINLINE Tc(MIN.) = 12.55
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.068
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.21	0.98	0.100	32
PUBLIC PARK	A	0.18	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.446
 SUBAREA AREA(ACRES) = 0.39 SUBAREA RUNOFF(CFS) = 0.92
 EFFECTIVE AREA(ACRES) = 16.56 AREA-AVERAGED Fm(INCH/HR) = 0.55
 AREA-AVERAGED Fp(INCH/HR) = 0.92 AREA-AVERAGED Ap = 0.59
 TOTAL AREA(ACRES) = 17.2 PEAK FLOW RATE(CFS) = 37.57

FLOW PROCESS FROM NODE 5415.00 TO NODE 5420.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 689.61 DOWNSTREAM(FEET) = 687.37
 FLOW LENGTH(FEET) = 595.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 28.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.26
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 37.57
 PIPE TRAVEL TIME(MIN.) = 1.58 Tc(MIN.) = 14.13
 LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5420.00 = 1862.00 FEET.

FLOW PROCESS FROM NODE 5420.00 TO NODE 5420.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 14.13
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.857
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	10.00	0.57	0.500	69
RESIDENTIAL "5-7 DWELLINGS/ACRE"	A	4.69	0.98	0.500	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.70
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 14.69 SUBAREA RUNOFF(CFS) = 33.17
 EFFECTIVE AREA(ACRES) = 31.25 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.82 AREA-AVERAGED Ap = 0.55
 TOTAL AREA(ACRES) = 31.9 PEAK FLOW RATE(CFS) = 67.59

FLOW PROCESS FROM NODE 5420.00 TO NODE 5420.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 14.13
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.857
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.30	0.98	0.100	32
PUBLIC PARK	A	0.26	0.98	0.850	32

COMMERCIAL C 0.15 0.57 0.100 69
 PUBLIC PARK C 0.12 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.84
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.443
 SUBAREA AREA(ACRES) = 0.83 SUBAREA RUNOFF(CFS) = 1.85
 EFFECTIVE AREA(ACRES) = 32.08 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.83 AREA-AVERAGED Ap = 0.55
 TOTAL AREA(ACRES) = 32.8 PEAK FLOW RATE(CFS) = 69.45

FLOW PROCESS FROM NODE 5420.00 TO NODE 5420.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 14.13
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.857
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.32	0.98	0.100	32
PUBLIC PARK	A	0.28	0.98	0.850	32
COMMERCIAL	C	0.17	0.57	0.100	69
PUBLIC PARK	C	0.12	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.85
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.437
 SUBAREA AREA(ACRES) = 0.89 SUBAREA RUNOFF(CFS) = 1.99
 EFFECTIVE AREA(ACRES) = 32.97 AREA-AVERAGED Fm(INCH/HR) = 0.45
 AREA-AVERAGED Fp(INCH/HR) = 0.83 AREA-AVERAGED Ap = 0.54
 TOTAL AREA(ACRES) = 33.7 PEAK FLOW RATE(CFS) = 71.44

FLOW PROCESS FROM NODE 5420.00 TO NODE 5425.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 687.37 DOWNSTREAM(FEET) = 679.09
 FLOW LENGTH(FEET) = 640.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 36.0 INCH PIPE IS 29.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.61
 ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 71.44
 PIPE TRAVEL TIME(MIN.) = 0.92 Tc(MIN.) = 15.05
 LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5425.00 = 2502.00 FEET.

FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 15.05
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.751
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	0.30	0.98	0.100	32
PUBLIC PARK	A	0.26	0.98	0.850	32

COMMERCIAL C 0.53 0.57 0.100 69
 PUBLIC PARK C 0.44 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.440
 SUBAREA AREA(ACRES) = 0.97 SUBAREA RUNOFF(CFS) = 2.18
 EFFECTIVE AREA(ACRES) = 33.94 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.82 AREA-AVERAGED Ap = 0.54
 TOTAL AREA(ACRES) = 34.6 PEAK FLOW RATE(CFS) = 71.44
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 15.05
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.751
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	0.53	0.57	0.100	69
PUBLIC PARK	C	0.44	0.57	0.850	69

 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.440
 SUBAREA AREA(ACRES) = 0.97 SUBAREA RUNOFF(CFS) = 2.18
 EFFECTIVE AREA(ACRES) = 34.91 AREA-AVERAGED Fm(INCH/HR) = 0.44
 AREA-AVERAGED Fp(INCH/HR) = 0.81 AREA-AVERAGED Ap = 0.54
 TOTAL AREA(ACRES) = 35.6 PEAK FLOW RATE(CFS) = 72.66

FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 11

>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

** MAIN STREAM CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	72.66	15.05	2.751	0.81(0.44)	0.54	34.9	5048.00
2	68.39	16.77	2.579	0.82(0.44)	0.54	35.6	5400.00

LONGEST FLOWPATH FROM NODE 5048.00 TO NODE 5425.00 = 2502.00 FEET.

** MEMORY BANK # 1 CONFLUENCE DATA **

PEAK FLOW RATE(CFS) = 1064.45 Tc(MIN.) = 38.00
 AREA-AVERAGED Fm(INCH/HR) = 0.21 Ybar = 0.24
 TOTAL AREA(ACRES) = 798.7
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5425.00 = 15159.00 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00

S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.63; LAG(HR) = 0.51; Fm(INCH/HR) = 0.22; Ybar = 0.25

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
3HR = 0.99; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 834.3

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5425.00 = 15159.00 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0309; Lca/L=0.4,n=.0277; Lca/L=0.5,n=.0255;Lca/L=0.6,n=.0238

TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 320.07

PEAK FLOW RATE(CFS) = 1102.99

=====

END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 834.3 TC(MIN.) = 38.00

AREA-AVERAGED Fm(INCH/HR)= 0.22 Ybar = 0.25

PEAK FLOW RATE(CFS) = 1102.99

=====

END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS



UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:
 WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF
 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH
 FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
 SIERRA MADRE DEPTH-AREA FACTORS USED.

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 (Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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 Ver. 22.0 Release Date: 07/01/2015 License ID 1673

Analysis prepared by:

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

Mill Creek - Proposed Condition

***** DESCRIPTION OF STUDY *****

- * SUBAREA 29 *
- * MILL CREEK SD *
- * PROPOSED CONDITION 100YR *

FILE NAME: PRMCRK.DAT
 TIME/DATE OF STUDY: 11:02 04/22/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL IN- / OUT- / SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GEOMETRIES LIP (FT)	MANNING HIKE (FT)	STREETFLOW FACTOR (n)
1	18.0	12.0	0.020/0.020/0.020	0.67	2.00	0.0312	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<

=====

PEAK FLOWRATE TABLE FILE NAME: MPMCRK.DNA MP Node 5425 per 2012 MP
 MEMORY BANK # 1 DEFINED AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 1102.99 Tc(MIN.) = 38.00
 AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
 TOTAL AREA(ACRES) = 834.3
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5425.00 = 15159.00 FEET.

FLOW PROCESS FROM NODE 5425.00 TO NODE 5425.00 IS CODE = 14.0

>>>>MEMORY BANK # 1 COPIED ONTO MAIN-STREAM MEMORY<<<<<

=====

MAIN-STREAM MEMORY DEFINED AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 1102.99 Tc(MIN.) = 38.00
 AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
 TOTAL AREA(ACRES) = 834.3
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5425.00 = 15159.00 FEET.

FLOW PROCESS FROM NODE 5425.00 TO NODE 5455.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 676.55 DOWNSTREAM(FEET) = 671.57
 FLOW LENGTH(FEET) = 712.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 114.0 INCH PIPE IS 89.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.39
 ESTIMATED PIPE DIAMETER(INCH) = 114.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1102.99
 PIPE TRAVEL TIME(MIN.) = 0.65 Tc(MIN.) = 38.65
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5455.00 = 15871.00 FEET.

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*****
FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 38.65
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.562 P Area per 2012 MP
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
COMMERCIAL C 0.66 0.57 0.100 69
PUBLIC PARK C 0.46 0.57 0.850 69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.408
SUBAREA AREA(ACRES) = 1.12
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.64; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
3HR = 0.99; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 835.4
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5455.00 = 15871.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0304; Lca/L=0.4,n=.0273; Lca/L=0.5,n=.0251;Lca/L=0.6,n=.0234
TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 320.50
UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1097.28
TOTAL AREA(ACRES) = 835.4 PEAK FLOW RATE(CFS) = 1102.99
NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

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*****
FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
PEAK FLOW RATE(CFS) = 1102.99 Tc(MIN.) = 38.65
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 835.4

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*****
FLOW PROCESS FROM NODE 5430.00 TO NODE 5431.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 727.00
ELEVATION DATA: UPSTREAM(FEET) = 703.40 DOWNSTREAM(FEET) = 687.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 11.657

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* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.207
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"5-7 DWELLINGS/ACRE" C 1.00 0.57 0.500 69 11.66
RESIDENTIAL
"5-7 DWELLINGS/ACRE" A 2.95 0.98 0.500 32 11.66
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.87
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500 P Area per 2012 MP
SUBAREA RUNOFF(CFS) = 9.85
TOTAL AREA(ACRES) = 3.95 PEAK FLOW RATE(CFS) = 9.85

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*****
FLOW PROCESS FROM NODE 5431.00 TO NODE 5432.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 687.50 DOWNSTREAM(FEET) = 682.10
FLOW LENGTH(FEET) = 749.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.86
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 9.85
PIPE TRAVEL TIME(MIN.) = 2.13 Tc(MIN.) = 13.79
LONGEST FLOWPATH FROM NODE 5430.00 TO NODE 5432.00 = 1476.00 FEET.

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*****
FLOW PROCESS FROM NODE 5432.00 TO NODE 5432.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 13.79 P Area per 2012 MP
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.900
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE" C 3.81 0.57 0.500 69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 3.81 SUBAREA RUNOFF(CFS) = 8.97
EFFECTIVE AREA(ACRES) = 7.76 AREA-AVERAGED Fm(INCH/HR) = 0.36
AREA-AVERAGED Fp(INCH/HR) = 0.72 AREA-AVERAGED Ap = 0.50
TOTAL AREA(ACRES) = 7.8 PEAK FLOW RATE(CFS) = 17.73

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*****
FLOW PROCESS FROM NODE 5432.00 TO NODE 5435.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 682.10 DOWNSTREAM(FEET) = 681.00
FLOW LENGTH(FEET) = 166.00 MANNING'S N = 0.013

```


DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.58
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 17.73
 PIPE TRAVEL TIME(MIN.) = 0.42 Tc(MIN.) = 14.21
 LONGEST FLOWPATH FROM NODE 5430.00 TO NODE 5435.00 = 1642.00 FEET.

FLOW PROCESS FROM NODE 5435.00 TO NODE 5435.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 14.21
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.848 P Area per 2012 MP
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	1.85	0.57	0.500	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 1.85 SUBAREA RUNOFF(CFS) = 4.27
 EFFECTIVE AREA(ACRES) = 9.61 AREA-AVERAGED Fm(INCH/HR) = 0.35
 AREA-AVERAGED Fp(INCH/HR) = 0.69 AREA-AVERAGED Ap = 0.50
 TOTAL AREA(ACRES) = 9.6 PEAK FLOW RATE(CFS) = 21.64

FLOW PROCESS FROM NODE 5435.00 TO NODE 5455.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 681.00 DOWNSTREAM(FEET) = 675.09
 FLOW LENGTH(FEET) = 357.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 16.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.73
 ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 21.64
 PIPE TRAVEL TIME(MIN.) = 0.61 Tc(MIN.) = 14.82
 LONGEST FLOWPATH FROM NODE 5430.00 TO NODE 5455.00 = 1999.00 FEET.

FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 14.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.777 P Area per 2012 MP
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	2.53	0.57	0.500	69
PUBLIC PARK	C	0.90	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.592
 SUBAREA AREA(ACRES) = 3.43 SUBAREA RUNOFF(CFS) = 7.54
 EFFECTIVE AREA(ACRES) = 13.04 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.52
 TOTAL AREA(ACRES) = 13.0 PEAK FLOW RATE(CFS) = 28.56

FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 14.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.777 P Area per 2012 MP
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	0.96	0.57	0.500	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 0.96 SUBAREA RUNOFF(CFS) = 2.15
 EFFECTIVE AREA(ACRES) = 14.00 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.52
 TOTAL AREA(ACRES) = 14.0 PEAK FLOW RATE(CFS) = 30.72

FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 14.82
 RAINFALL INTENSITY(INCH/HR) = 2.78
 AREA-AVERAGED Fm(INCH/HR) = 0.34
 AREA-AVERAGED Fp(INCH/HR) = 0.65
 AREA-AVERAGED Ap = 0.52
 EFFECTIVE STREAM AREA(ACRES) = 14.00
 TOTAL STREAM AREA(ACRES) = 14.00
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 30.72
 ** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	AREA (ACRES)	HEADWATER NODE
1	1102.99	38.65	835.43	5000.00
2	30.72	14.82	14.00	5430.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(UR) = 0.64; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;

3HR = 0.99; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 849.4
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5455.00 = 15871.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0303; Lca/L=0.4,n=.0271; Lca/L=0.5,n=.0249;Lca/L=0.6,n=.0233
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 325.19
 PEAK FLOW RATE(CFS) = 1113.63

 FLOW PROCESS FROM NODE 5455.00 TO NODE 5455.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 38.65
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.562
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 APARTMENTS C 12.10 0.57 0.200 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
 SUBAREA AREA(ACRES) = 12.10
 UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.64; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
 3HR = 0.99; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 861.5
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5455.00 = 15871.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0303; Lca/L=0.4,n=.0271; Lca/L=0.5,n=.0249;Lca/L=0.6,n=.0233
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 330.42
 UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1129.93
 TOTAL AREA(ACRES) = 861.5 PEAK FLOW RATE(CFS) = 1129.93

P2

 FLOW PROCESS FROM NODE 5455.00 TO NODE 5470.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 671.57 DOWNSTREAM(FEET) = 666.50
 FLOW LENGTH(FEET) = 767.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 120.0 INCH PIPE IS 87.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 18.34
 ESTIMATED PIPE DIAMETER(INCH) = 120.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 1129.93
 PIPE TRAVEL TIME(MIN.) = 0.70 Tc(MIN.) = 39.34
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.

 FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 39.34
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.546 P Area per 2012 MP
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 1.23 0.57 0.100 69
 PUBLIC PARK C 1.01 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.438
 SUBAREA AREA(ACRES) = 2.24

UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.66; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
 3HR = 0.99; 6HR = 1.00; 24HR= 1.00
 UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 863.8
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.
 EQUIVALENT BASIN FACTOR APPROXIMATIONS:
 Lca/L=0.3,n=.0298; Lca/L=0.4,n=.0267; Lca/L=0.5,n=.0246;Lca/L=0.6,n=.0229
 TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 331.26
 UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1124.30
 TOTAL AREA(ACRES) = 863.8 PEAK FLOW RATE(CFS) = 1129.93
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====

MAINLINE Tc(MIN.) = 39.34
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.546 P Area per 2012 MP
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL C 0.58 0.57 0.100 69
 PUBLIC PARK C 0.52 0.57 0.850 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.455
 SUBAREA AREA(ACRES) = 1.10

UNIT-HYDROGRAPH DATA:
 RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
 S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
 MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
 Tc(HR) = 0.66; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
 USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
 DEPTH-AREA FACTORS: 5M = 0.96; 30M = 0.96; 1HR = 0.96;
 3HR = 0.99; 6HR = 1.00; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 864.9
 LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0298; Lca/L=0.4,n=.0267; Lca/L=0.5,n=.0246;Lca/L=0.6,n=.0229

TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 331.67

UNIT-HYDROGRAPH PEAK FLOW RATE(CFS) = 1125.65

TOTAL AREA(ACRES) = 864.9 PEAK FLOW RATE(CFS) = 1129.93

NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

=====
END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 864.9 TC(MIN.) = 39.34

AREA-AVERAGED Fm(INCH/HR)= 0.22 Ybar = 0.25

PEAK FLOW RATE(CFS) = 1129.93
=====

=====
END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS



 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
 (Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
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 Ver. 22.0 Release Date: 07/01/2015 License ID 1673

Analysis prepared by:

Milliken - Recreated 2012 MP Hydrology Tributary to MP Node 6130

***** DESCRIPTION OF STUDY *****
 * SUBAREA 29 *
 * MILLIKEN WATERSHED *
 * 2017 ONTARIO MP - TRIB TO MP NODE 6130 *

FILE NAME: MILLIKEN.DAT
 TIME/DATE OF STUDY: 12:14 03/15/2022

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
 USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
 USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL IN- / OUT- / SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GUTTER LIP (FT)	GEOMETRIES HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150
2	40.0	25.0	0.020/0.020/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

- Relative Flow-Depth = 0.50 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
- (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:
 WATERSHED LAG = 0.80 * Tc
 USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
 SIERRA MADRE DEPTH-AREA FACTORS USED.

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

FLOW PROCESS FROM NODE 6000.00 TO NODE 6005.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 800.00
 ELEVATION DATA: UPSTREAM(FEET) = 809.00 DOWNSTREAM(FEET) = 800.00

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 10.811
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.355
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
COMMERCIAL	A	3.63	0.98	0.100	32	10.81

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA RUNOFF(CFS) = 10.64
 TOTAL AREA(ACRES) = 3.63 PEAK FLOW RATE(CFS) = 10.64

FLOW PROCESS FROM NODE 6005.00 TO NODE 6010.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 2 USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 800.00 DOWNSTREAM ELEVATION(FEET) = 795.06
 STREET LENGTH(FEET) = 920.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 40.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 17.52
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.65
 HALFSTREET FLOOD WIDTH(FEET) = 24.42
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.85
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.84
 STREET FLOW TRAVEL TIME(MIN.) = 5.39 Tc(MIN.) = 16.20
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.633
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	5.89	0.98	0.100	32
PUBLIC PARK	A	0.15	0.98	0.850	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.119
 SUBAREA AREA(ACRES) = 6.04 SUBAREA RUNOFF(CFS) = 13.68
 EFFECTIVE AREA(ACRES) = 9.67 AREA-AVERAGED Fm(INCH/HR) = 0.11
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.11
 TOTAL AREA(ACRES) = 9.7 PEAK FLOW RATE(CFS) = 21.96

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.69 HALFSTREET FLOOD WIDTH(FEET) = 27.92
 FLOW VELOCITY(FEET/SEC.) = 3.00 DEPTH*VELOCITY(FT*FT/SEC.) = 2.08
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6010.00 = 1720.00 FEET.

 FLOW PROCESS FROM NODE 6010.00 TO NODE 6012.50 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 779.48 DOWNSTREAM(FEET) = 767.69
 FLOW LENGTH(FEET) = 1590.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.17
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 21.96
 PIPE TRAVEL TIME(MIN.) = 3.70 Tc(MIN.) = 19.90
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6012.50 = 3310.00 FEET.

 FLOW PROCESS FROM NODE 6012.50 TO NODE 6012.50 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 19.90
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.327
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	A	1.24	0.98	0.850	32

COMMERCIAL A 2.91 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.324
 SUBAREA AREA(ACRES) = 4.15 SUBAREA RUNOFF(CFS) = 7.51
 EFFECTIVE AREA(ACRES) = 13.82 AREA-AVERAGED Fm(INCH/HR) = 0.17
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.18
 TOTAL AREA(ACRES) = 13.8 PEAK FLOW RATE(CFS) = 26.82

 FLOW PROCESS FROM NODE 6012.50 TO NODE 6015.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 767.69 DOWNSTREAM(FEET) = 762.79
 FLOW LENGTH(FEET) = 917.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 6.59
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 26.82
 PIPE TRAVEL TIME(MIN.) = 2.32 Tc(MIN.) = 22.22
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6015.00 = 4227.00 FEET.

 FLOW PROCESS FROM NODE 6015.00 TO NODE 6015.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

 MAINLINE Tc(MIN.) = 22.22
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.178
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
PUBLIC PARK	A	0.71	0.98	0.850	32
COMMERCIAL	A	1.70	0.98	0.100	32

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.321
 SUBAREA AREA(ACRES) = 2.41 SUBAREA RUNOFF(CFS) = 4.05
 EFFECTIVE AREA(ACRES) = 16.23 AREA-AVERAGED Fm(INCH/HR) = 0.19
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.20
 TOTAL AREA(ACRES) = 16.2 PEAK FLOW RATE(CFS) = 29.01

 FLOW PROCESS FROM NODE 6015.00 TO NODE 6020.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

 ELEVATION DATA: UPSTREAM(FEET) = 762.79 DOWNSTREAM(FEET) = 745.81
 FLOW LENGTH(FEET) = 1290.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 19.3 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.53
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 29.01
 PIPE TRAVEL TIME(MIN.) = 2.26 Tc(MIN.) = 24.47

LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6020.00 = 5517.00 FEET.

 FLOW PROCESS FROM NODE 6020.00 TO NODE 6020.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 24.47
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.055
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 3.35 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.97
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 3.35 SUBAREA RUNOFF(CFS) = 5.90
 EFFECTIVE AREA(ACRES) = 19.58 AREA-AVERAGED Fm(INCH/HR) = 0.18
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.18
 TOTAL AREA(ACRES) = 19.6 PEAK FLOW RATE(CFS) = 33.12

 FLOW PROCESS FROM NODE 6020.00 TO NODE 6020.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 745.81 DOWNSTREAM(FEET) = 736.48
 FLOW LENGTH(FEET) = 1450.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 22.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.57
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 33.12
 PIPE TRAVEL TIME(MIN.) = 3.19 Tc(MIN.) = 27.67
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6020.00 = 6967.00 FEET.

 FLOW PROCESS FROM NODE 6025.00 TO NODE 6025.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 27.67
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.909
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 3.83 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 3.83 SUBAREA RUNOFF(CFS) = 6.25
 EFFECTIVE AREA(ACRES) = 23.41 AREA-AVERAGED Fm(INCH/HR) = 0.16
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.17
 TOTAL AREA(ACRES) = 23.4 PEAK FLOW RATE(CFS) = 36.79

 FLOW PROCESS FROM NODE 6025.00 TO NODE 6030.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 736.48 DOWNSTREAM(FEET) = 724.27
 FLOW LENGTH(FEET) = 1164.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 22.7 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.21
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 36.79
 PIPE TRAVEL TIME(MIN.) = 2.11 Tc(MIN.) = 29.77
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6030.00 = 8131.00 FEET.

 FLOW PROCESS FROM NODE 6030.00 TO NODE 6030.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 29.77
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.827
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 3.06 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 3.06 SUBAREA RUNOFF(CFS) = 4.76
 EFFECTIVE AREA(ACRES) = 26.47 AREA-AVERAGED Fm(INCH/HR) = 0.16
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.16
 TOTAL AREA(ACRES) = 26.5 PEAK FLOW RATE(CFS) = 39.83

 FLOW PROCESS FROM NODE 6030.00 TO NODE 6050.00 IS CODE = 31

 >>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
 =====
 ELEVATION DATA: UPSTREAM(FEET) = 724.27 DOWNSTREAM(FEET) = 713.28
 FLOW LENGTH(FEET) = 769.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 30.0 INCH PIPE IS 21.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.65
 ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 39.83
 PIPE TRAVEL TIME(MIN.) = 1.20 Tc(MIN.) = 30.97
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6050.00 = 8900.00 FEET.

 FLOW PROCESS FROM NODE 6050.00 TO NODE 6050.00 IS CODE = 81

 >>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
 =====
 MAINLINE Tc(MIN.) = 30.97
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.784
 SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS

LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 COMMERCIAL A 12.09 0.98 0.100 32
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.98
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 12.09 SUBAREA RUNOFF(CFS) = 18.35
 EFFECTIVE AREA(ACRES) = 38.56 AREA-AVERAGED Fm(INCH/HR) = 0.14
 AREA-AVERAGED Fp(INCH/HR) = 0.98 AREA-AVERAGED Ap = 0.14
 TOTAL AREA(ACRES) = 38.6 PEAK FLOW RATE(CFS) = 57.16

 FLOW PROCESS FROM NODE 6050.00 TO NODE 6075.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 713.28 DOWNSTREAM(FEET) = 702.49
 FLOW LENGTH(FEET) = 692.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.96
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 57.16
 PIPE TRAVEL TIME(MIN.) = 0.96 Tc(MIN.) = 31.94
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6075.00 = 9592.00 FEET.

 FLOW PROCESS FROM NODE 6075.00 TO NODE 6075.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 31.94
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.752
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	A	6.70	0.98	0.100	32
COMMERCIAL	C	4.77	0.57	0.100	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.80
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.100
 SUBAREA AREA(ACRES) = 11.47 SUBAREA RUNOFF(CFS) = 17.25
 EFFECTIVE AREA(ACRES) = 50.03 AREA-AVERAGED Fm(INCH/HR) = 0.12
 AREA-AVERAGED Fp(INCH/HR) = 0.95 AREA-AVERAGED Ap = 0.13
 TOTAL AREA(ACRES) = 50.0 PEAK FLOW RATE(CFS) = 73.28

 FLOW PROCESS FROM NODE 6075.00 TO NODE 6077.50 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 702.49 DOWNSTREAM(FEET) = 698.66
 FLOW LENGTH(FEET) = 476.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 42.0 INCH PIPE IS 29.9 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.00
 ESTIMATED PIPE DIAMETER(INCH) = 42.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 73.28

PIPE TRAVEL TIME(MIN.) = 0.79 Tc(MIN.) = 32.73
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6077.50 = 10068.00 FEET.

 FLOW PROCESS FROM NODE 6077.50 TO NODE 6077.50 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 32.73
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.726
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	0.90	0.57	0.100	69
PUBLIC PARK	C	0.16	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.213
 SUBAREA AREA(ACRES) = 1.06 SUBAREA RUNOFF(CFS) = 1.53
 EFFECTIVE AREA(ACRES) = 51.09 AREA-AVERAGED Fm(INCH/HR) = 0.12
 AREA-AVERAGED Fp(INCH/HR) = 0.93 AREA-AVERAGED Ap = 0.13
 TOTAL AREA(ACRES) = 51.1 PEAK FLOW RATE(CFS) = 73.66

 FLOW PROCESS FROM NODE 6077.50 TO NODE 6105.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 698.66 DOWNSTREAM(FEET) = 690.85
 FLOW LENGTH(FEET) = 831.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 39.0 INCH PIPE IS 31.0 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.43
 ESTIMATED PIPE DIAMETER(INCH) = 39.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 73.66
 PIPE TRAVEL TIME(MIN.) = 1.33 Tc(MIN.) = 34.06
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6105.00 = 10899.00 FEET.

 FLOW PROCESS FROM NODE 6105.00 TO NODE 6105.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 34.06
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.685
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	1.53	0.57	0.100	69
PUBLIC PARK	C	0.35	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.240
 SUBAREA AREA(ACRES) = 1.88 SUBAREA RUNOFF(CFS) = 2.62
 EFFECTIVE AREA(ACRES) = 52.97 AREA-AVERAGED Fm(INCH/HR) = 0.12
 AREA-AVERAGED Fp(INCH/HR) = 0.91 AREA-AVERAGED Ap = 0.14
 TOTAL AREA(ACRES) = 53.0 PEAK FLOW RATE(CFS) = 74.41

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*****
FLOW PROCESS FROM NODE 6105.00 TO NODE 6105.00 IS CODE = 1
-----
>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 34.06
RAINFALL INTENSITY(INCH/HR) = 1.69
AREA-AVERAGED Fm(INCH/HR) = 0.12
AREA-AVERAGED Fp(INCH/HR) = 0.91
AREA-AVERAGED Ap = 0.14
EFFECTIVE STREAM AREA(ACRES) = 52.97
TOTAL STREAM AREA(ACRES) = 52.97
PEAK FLOW RATE(CFS) AT CONFLUENCE = 74.41

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*****
FLOW PROCESS FROM NODE 6080.00 TO NODE 6085.00 IS CODE = 21
-----
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====
INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
ELEVATION DATA: UPSTREAM(FEET) = 715.50 DOWNSTREAM(FEET) = 714.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.005
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.744
SUBAREA Tc AND LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS Tc
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN (MIN.)
RESIDENTIAL
"11+ DWELLINGS/ACRE" C 1.38 0.57 0.200 69 9.00
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA RUNOFF(CFS) = 4.51
TOTAL AREA(ACRES) = 1.38 PEAK FLOW RATE(CFS) = 4.51

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*****
FLOW PROCESS FROM NODE 6085.00 TO NODE 6090.00 IS CODE = 62
-----
>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>(STREET TABLE SECTION # 1 USED)<<<<
=====
UPSTREAM ELEVATION(FEET) = 714.50 DOWNSTREAM ELEVATION(FEET) = 713.50
STREET LENGTH(FEET) = 847.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.018
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
STREET PARKWAY CROSSFALL(DECIMAL) = 0.020

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Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.50
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.58
HALFSTREET FLOOD WIDTH(FEET) = 23.55
AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.21
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.71
STREET FLOW TRAVEL TIME(MIN.) = 11.62 Tc(MIN.) = 20.63
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.277
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"11+ DWELLINGS/ACRE" C 7.98 0.57 0.200 69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA(ACRES) = 7.98 SUBAREA RUNOFF(CFS) = 15.54
EFFECTIVE AREA(ACRES) = 9.36 AREA-AVERAGED Fm(INCH/HR) = 0.11
AREA-AVERAGED Fp(INCH/HR) = 0.57 AREA-AVERAGED Ap = 0.20
TOTAL AREA(ACRES) = 9.4 PEAK FLOW RATE(CFS) = 18.23

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.65 HALFSTREET FLOOD WIDTH(FEET) = 27.30
FLOW VELOCITY(FEET/SEC.) = 1.33 DEPTH*VELOCITY(FT*FT/SEC.) = 0.86
LONGEST FLOWPATH FROM NODE 6080.00 TO NODE 6090.00 = 1102.00 FEET.

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*****
FLOW PROCESS FROM NODE 6090.00 TO NODE 6095.00 IS CODE = 31
-----
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 713.50 DOWNSTREAM(FEET) = 706.60
FLOW LENGTH(FEET) = 425.00 MANNING'S N = 0.013
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 24.000
DEPTH OF FLOW IN 24.0 INCH PIPE IS 14.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.32
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 18.23
PIPE TRAVEL TIME(MIN.) = 0.76 Tc(MIN.) = 21.39
LONGEST FLOWPATH FROM NODE 6080.00 TO NODE 6095.00 = 1527.00 FEET.

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*****
FLOW PROCESS FROM NODE 6095.00 TO NODE 6095.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
MAINLINE Tc(MIN.) = 21.39
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.228
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL

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"11+ DWELLINGS/ACRE"      A      1.00      0.98      0.200      32
RESIDENTIAL
"11+ DWELLINGS/ACRE"      C      8.67      0.57      0.200      69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.61
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.200
SUBAREA AREA(ACRES) = 9.67      SUBAREA RUNOFF(CFS) = 18.33
EFFECTIVE AREA(ACRES) = 19.03      AREA-AVERAGED Fm(INCH/HR) = 0.12
AREA-AVERAGED Fp(INCH/HR) = 0.59      AREA-AVERAGED Ap = 0.20
TOTAL AREA(ACRES) = 19.0      PEAK FLOW RATE(CFS) = 36.15

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*****
FLOW PROCESS FROM NODE 6095.00 TO NODE 6097.50 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
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ELEVATION DATA: UPSTREAM(FEET) = 706.60 DOWNSTREAM(FEET) = 703.67
FLOW LENGTH(FEET) = 586.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 7.04
ESTIMATED PIPE DIAMETER(INCH) = 36.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 36.15
PIPE TRAVEL TIME(MIN.) = 1.39      Tc(MIN.) = 22.78
LONGEST FLOWPATH FROM NODE 6080.00 TO NODE 6097.50 = 2113.00 FEET.

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*****
FLOW PROCESS FROM NODE 6097.50 TO NODE 6097.50 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 22.78
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.146
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL      AREA      Fp      Ap      SCS
LAND USE      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE"      A      2.30      0.98      0.500      32
RESIDENTIAL
"5-7 DWELLINGS/ACRE"      C      9.59      0.57      0.500      69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.65
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 11.89      SUBAREA RUNOFF(CFS) = 19.51
EFFECTIVE AREA(ACRES) = 30.92      AREA-AVERAGED Fm(INCH/HR) = 0.20
AREA-AVERAGED Fp(INCH/HR) = 0.62      AREA-AVERAGED Ap = 0.32
TOTAL AREA(ACRES) = 30.9      PEAK FLOW RATE(CFS) = 54.25

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*****
FLOW PROCESS FROM NODE 6097.50 TO NODE 6100.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
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ELEVATION DATA: UPSTREAM(FEET) = 703.67 DOWNSTREAM(FEET) = 700.00
FLOW LENGTH(FEET) = 742.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 39.0 INCH PIPE IS 31.4 INCHES

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PIPE-FLOW VELOCITY(FEET/SEC.) = 7.57
ESTIMATED PIPE DIAMETER(INCH) = 39.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 54.25
PIPE TRAVEL TIME(MIN.) = 1.63      Tc(MIN.) = 24.41
LONGEST FLOWPATH FROM NODE 6080.00 TO NODE 6100.00 = 2855.00 FEET.

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FLOW PROCESS FROM NODE 6100.00 TO NODE 6100.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 24.41
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.058
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL      AREA      Fp      Ap      SCS
LAND USE      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE"      C      9.54      0.57      0.500      69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 9.54      SUBAREA RUNOFF(CFS) = 15.24
EFFECTIVE AREA(ACRES) = 40.46      AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.60      AREA-AVERAGED Ap = 0.36
TOTAL AREA(ACRES) = 40.5      PEAK FLOW RATE(CFS) = 67.06

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*****
FLOW PROCESS FROM NODE 6100.00 TO NODE 6105.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
-----

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```

ELEVATION DATA: UPSTREAM(FEET) = 700.00 DOWNSTREAM(FEET) = 690.85
FLOW LENGTH(FEET) = 477.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 33.0 INCH PIPE IS 26.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.33
ESTIMATED PIPE DIAMETER(INCH) = 33.00      NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 67.06
PIPE TRAVEL TIME(MIN.) = 0.60      Tc(MIN.) = 25.01
LONGEST FLOWPATH FROM NODE 6080.00 TO NODE 6105.00 = 3332.00 FEET.

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FLOW PROCESS FROM NODE 6105.00 TO NODE 6105.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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MAINLINE Tc(MIN.) = 25.01
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.029
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL      AREA      Fp      Ap      SCS
LAND USE      GROUP      (ACRES)      (INCH/HR)      (DECIMAL)      CN
COMMERCIAL      C      1.55      0.57      0.100      69
PUBLIC PARK      C      0.75      0.57      0.850      69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.345
SUBAREA AREA(ACRES) = 2.30      SUBAREA RUNOFF(CFS) = 3.80

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EFFECTIVE AREA(ACRES) = 42.76 AREA-AVERAGED Fm(INCH/HR) = 0.22
 AREA-AVERAGED Fp(INCH/HR) = 0.60 AREA-AVERAGED Ap = 0.36
 TOTAL AREA(ACRES) = 42.8 PEAK FLOW RATE(CFS) = 69.78

 FLOW PROCESS FROM NODE 6105.00 TO NODE 6105.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 25.01
 RAINFALL INTENSITY(INCH/HR) = 2.03
 AREA-AVERAGED Fm(INCH/HR) = 0.22
 AREA-AVERAGED Fp(INCH/HR) = 0.60
 AREA-AVERAGED Ap = 0.36
 EFFECTIVE STREAM AREA(ACRES) = 42.76
 TOTAL STREAM AREA(ACRES) = 42.76
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 69.78

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	74.41	34.06	1.685	0.91(0.12)	0.14	53.0	6000.00
2	69.78	25.01	2.029	0.60(0.22)	0.36	42.8	6080.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	136.43	25.01	2.029	0.68(0.17)	0.25	81.6	6080.00
2	130.98	34.06	1.685	0.70(0.17)	0.24	95.7	6000.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 136.43 Tc(MIN.) = 25.01
 EFFECTIVE AREA(ACRES) = 81.65 AREA-AVERAGED Fm(INCH/HR) = 0.17
 AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.25
 TOTAL AREA(ACRES) = 95.7
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6105.00 = 10899.00 FEET.

 FLOW PROCESS FROM NODE 6105.00 TO NODE 6130.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 690.85 DOWNSTREAM(FEET) = 679.50
 FLOW LENGTH(FEET) = 1791.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 54.0 INCH PIPE IS 40.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 10.59
 ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 136.43

PIPE TRAVEL TIME(MIN.) = 2.82 Tc(MIN.) = 27.82
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 27.82
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.903
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
COMMERCIAL	C	3.02	0.57	0.100	69
PUBLIC PARK	C	0.98	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.284
 SUBAREA AREA(ACRES) = 4.00 SUBAREA RUNOFF(CFS) = 6.27
 EFFECTIVE AREA(ACRES) = 85.65 AREA-AVERAGED Fm(INCH/HR) = 0.17
 AREA-AVERAGED Fp(INCH/HR) = 0.68 AREA-AVERAGED Ap = 0.25
 TOTAL AREA(ACRES) = 99.7 PEAK FLOW RATE(CFS) = 136.43
 NOTE: PEAK FLOW RATE DEFAULTED TO UPSTREAM VALUE

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
 TIME OF CONCENTRATION(MIN.) = 27.82
 RAINFALL INTENSITY(INCH/HR) = 1.90
 AREA-AVERAGED Fm(INCH/HR) = 0.17
 AREA-AVERAGED Fp(INCH/HR) = 0.68
 AREA-AVERAGED Ap = 0.25
 EFFECTIVE STREAM AREA(ACRES) = 85.65
 TOTAL STREAM AREA(ACRES) = 99.73
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 136.43

 FLOW PROCESS FROM NODE 6110.00 TO NODE 6115.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 595.00
 ELEVATION DATA: UPSTREAM(FEET) = 704.20 DOWNSTREAM(FEET) = 703.80

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
 SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 21.589
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 2.216
 SUBAREA Tc AND LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						

"5-7 DWELLINGS/ACRE" C 3.15 0.57 0.500 69 21.59
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA RUNOFF(CFS) = 5.48
 TOTAL AREA(ACRES) = 3.15 PEAK FLOW RATE(CFS) = 5.48

 FLOW PROCESS FROM NODE 6115.00 TO NODE 6120.00 IS CODE = 62

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STREET TABLE SECTION # 1 USED)<<<<<

UPSTREAM ELEVATION(FEET) = 703.80 DOWNSTREAM ELEVATION(FEET) = 702.90
 STREET LENGTH(FEET) = 386.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 20.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.018
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.018

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2
 STREET PARKWAY CROSSFALL(DECIMAL) = 0.020
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150
 Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 12.11
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.53
 HALFSTREET FLOOD WIDTH(FEET) = 20.35
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 1.56
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.82
 STREET FLOW TRAVEL TIME(MIN.) = 4.13 Tc(MIN.) = 25.72
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.995

SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	C	7.71	0.57	0.500	69
PUBLIC PARK	C	1.00	0.57	0.850	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.540
 SUBAREA AREA(ACRES) = 8.71 SUBAREA RUNOFF(CFS) = 13.24
 EFFECTIVE AREA(ACRES) = 11.86 AREA-AVERAGED Fm(INCH/HR) = 0.30
 AREA-AVERAGED Fp(INCH/HR) = 0.57 AREA-AVERAGED Ap = 0.53
 TOTAL AREA(ACRES) = 11.9 PEAK FLOW RATE(CFS) = 18.09

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.59 HALFSTREET FLOOD WIDTH(FEET) = 23.87
 FLOW VELOCITY(FEET/SEC.) = 1.71 DEPTH*VELOCITY(FT*FT/SEC.) = 1.01
 LONGEST FLOWPATH FROM NODE 6110.00 TO NODE 6120.00 = 981.00 FEET.

 FLOW PROCESS FROM NODE 6120.00 TO NODE 6125.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 702.90 DOWNSTREAM(FEET) = 689.90
 FLOW LENGTH(FEET) = 676.00 MANNING'S N = 0.013
 ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 24.000
 DEPTH OF FLOW IN 24.0 INCH PIPE IS 13.5 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 9.93
 ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 18.09
 PIPE TRAVEL TIME(MIN.) = 1.13 Tc(MIN.) = 26.86
 LONGEST FLOWPATH FROM NODE 6110.00 TO NODE 6125.00 = 1657.00 FEET.

 FLOW PROCESS FROM NODE 6125.00 TO NODE 6125.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 26.86
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.944
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	A	2.10	0.98	0.500	32
RESIDENTIAL					
"5-7 DWELLINGS/ACRE"	C	4.13	0.57	0.500	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.70
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 6.23 SUBAREA RUNOFF(CFS) = 8.92
 EFFECTIVE AREA(ACRES) = 18.09 AREA-AVERAGED Fm(INCH/HR) = 0.32
 AREA-AVERAGED Fp(INCH/HR) = 0.61 AREA-AVERAGED Ap = 0.52
 TOTAL AREA(ACRES) = 18.1 PEAK FLOW RATE(CFS) = 26.47

 FLOW PROCESS FROM NODE 6125.00 TO NODE 6130.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 689.90 DOWNSTREAM(FEET) = 679.50
 FLOW LENGTH(FEET) = 1196.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 21.4 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 7.85
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 26.47
 PIPE TRAVEL TIME(MIN.) = 2.54 Tc(MIN.) = 29.40
 LONGEST FLOWPATH FROM NODE 6110.00 TO NODE 6130.00 = 2853.00 FEET.

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 29.40
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.841

SUBAREA LOSS RATE DATA(AMC II):
 DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
 LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
 RESIDENTIAL
 "5-7 DWELLINGS/ACRE" C 8.18 0.57 0.500 69
 SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 8.18 SUBAREA RUNOFF(CFS) = 11.47
 EFFECTIVE AREA(ACRES) = 26.27 AREA-AVERAGED Fm(INCH/HR) = 0.31
 AREA-AVERAGED Fp(INCH/HR) = 0.60 AREA-AVERAGED Ap = 0.51
 TOTAL AREA(ACRES) = 26.3 PEAK FLOW RATE(CFS) = 36.27

EFFECTIVE AREA(ACRES) = 110.51 AREA-AVERAGED Fm(INCH/HR)= 0.20
 AREA-AVERAGED Fp(INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.313
 PEAK FLOW RATE(CFS) = 172.14

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	172.14	27.82	1.903	0.65(0.20)	0.31	110.5	6080.00
2	171.76	29.40	1.841	0.65(0.20)	0.31	114.4	6110.00
3	161.71	36.89	1.607	0.66(0.19)	0.30	126.0	6000.00

=====
 END OF RATIONAL METHOD ANALYSIS
 =====

 FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 1

 >>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

 TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 29.40
 RAINFALL INTENSITY(INCH/HR) = 1.84
 AREA-AVERAGED Fm(INCH/HR) = 0.31
 AREA-AVERAGED Fp(INCH/HR) = 0.60
 AREA-AVERAGED Ap = 0.51
 EFFECTIVE STREAM AREA(ACRES) = 26.27
 TOTAL STREAM AREA(ACRES) = 26.27
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 36.27

** CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	136.43	27.82	1.903	0.68(0.17)	0.25	85.6	6080.00
1	130.98	36.89	1.607	0.69(0.17)	0.24	99.7	6000.00
2	36.27	29.40	1.841	0.60(0.31)	0.51	26.3	6110.00

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	172.14	27.82	1.903	0.65(0.20)	0.31	110.5	6080.00
2	171.76	29.40	1.841	0.65(0.20)	0.31	114.4	6110.00
3	161.71	36.89	1.607	0.66(0.19)	0.30	126.0	6000.00

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 172.14 Tc(MIN.) = 27.82
 EFFECTIVE AREA(ACRES) = 110.51 AREA-AVERAGED Fm(INCH/HR) = 0.20
 AREA-AVERAGED Fp(INCH/HR) = 0.65 AREA-AVERAGED Ap = 0.31
 TOTAL AREA(ACRES) = 126.0
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

 END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 126.0 TC(MIN.) = 27.82

Appendix B – Hydraulics

- **County Line Channel WSPG Results**
- **County Line Channel HGL Comparison**
- **Haven Ave SD WSPG Results**
- **Haven Ave SD HGL Comparison**
- **Mill Creek WSPG Results**
- **Mill Creek HGL Comparison**

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	1 IS A SYSTEM OUTLET	*	*	*											
	U/S DATA	STATION	INVERT	SECT					W S ELEV						
		8118.22	659.16	12					669.16						
ELEMENT NO	2 IS A REACH	*	*	*											
	U/S DATA	STATION	INVERT	SECT						RADIUS	ANGLE	ANG PT	MAN H		
		8118.22	659.16	12				N		0.00	0.00	0.00	0		
								0.014							
ELEMENT NO	3 IS A REACH	*	*	*											
	U/S DATA	STATION	INVERT	SECT						RADIUS	ANGLE	ANG PT	MAN H		
		8619.32	660.53	12				N		0.00	0.00	0.00	0		
								0.014							
ELEMENT NO	4 IS A JUNCTION	*	*	*	*	*	*	*	*	*	*	*	*		
	U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4		
		8619.33	660.54	12	5	0	0.014	499.4	0.0	661.50	0.00	30.00	0.00		
ELEMENT NO	5 IS A REACH	*	*	*											
	U/S DATA	STATION	INVERT	SECT						RADIUS	ANGLE	ANG PT	MAN H		
		10110.31	664.62	12				N		0.00	0.00	0.00	0		
								0.014							
ELEMENT NO	6 IS A JUNCTION	*	*	*	*	*	*	*	*	*	*	*	*		
	U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4		
		10116.31	664.64	12	17	0	0.014	33.4	0.0	665.00	0.00	30.00	0.00		
ELEMENT NO	7 IS A REACH	*	*	*											
	U/S DATA	STATION	INVERT	SECT						RADIUS	ANGLE	ANG PT	MAN H		
		11452.00	668.30	12				N		0.00	0.00	0.00	0		
								0.014							
ELEMENT NO	8 IS A JUNCTION	*	*	*	*	*	*	*	*	*	*	*	*		
	U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4		
		11485.00	668.39	11	14	0	0.014	1118.8	0.0	669.00	0.00	30.00	0.00		
ELEMENT NO	9 IS A REACH	*	*	*											
	U/S DATA	STATION	INVERT	SECT						RADIUS	ANGLE	ANG PT	MAN H		
		14277.00	676.04	11				N		0.00	0.00	0.00	0		
								0.014							
ELEMENT NO	10 IS A SYSTEM HEADWORKS	*	*	*											
	U/S DATA	STATION	INVERT	SECT						W S ELEV					
		14277.00	676.04	11						676.04					

NO EDIT ERRORS ENCOUNTERED-COMPUTATION IS NOW BEGINNING

** WARNING NO. 2 ** - WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN HDWKDS, W.S.ELEV = INV + DC

SUBAREA 29
COUNTY LINE WITH UPDATED Q100 AND 2017 MP TW
2022-04-25

STATION	INVERT ELEV	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD	ENERGY GRD.EL.	SUPER ELEV	CRITICAL DEPTH	HGT/DIA	BASE/ID NO.	ZL	NO PIER	AVBPR
L/ELEM	SO					SF AVE	HF		NORM DEPTH			ZR		
8118.22	659.16	10.000	669.160	2388.5	14.95	3.471	672.631	0.00	8.848	10.00	16.00	0.00	0	0.00
501.10	0.00273					.004443	2.23		9.962			0.00		
8619.32	660.53	10.856	671.386	2388.5	14.95	3.471	674.857	0.00	8.848	10.00	16.00	0.00	0	0.00
JUNCT STR	1.00000					.003611	0.00					0.00		
8619.33	660.54	12.611	673.151	1889.1	11.83	2.171	675.322	0.00	7.567	10.00	16.00	0.00	0	0.00
1490.98	0.00274					.002779	4.14		8.328			0.00		
10110.31	664.62	12.675	677.295	1889.1	11.83	2.171	679.466	0.00	7.567	10.00	16.00	0.00	0	0.00
JUNCT STR	0.00333					.002730	0.02					0.00		
10116.31	664.64	12.814	677.454	1855.7	11.62	2.095	679.549	0.00	7.478	10.00	16.00	0.00	0	0.00
1335.69	0.00274					.002682	3.58		8.212			0.00		
11452.00	668.30	12.736	681.036	1855.7	11.62	2.095	683.131	0.00	7.478	10.00	16.00	0.00	0	0.00
JUNCT STR	0.00273					.002057	0.07					0.00		
11485.00	668.39	13.961	682.351	736.9	7.39	0.847	683.198	0.00	5.526	10.00	10.00	0.00	0	0.00
2792.00	0.00274					.001432	4.00		6.605			0.00		
14277.00	676.04	10.311	686.351	736.9	7.39	0.847	687.198	0.00	5.526	10.00	10.00	0.00	0	0.00

.
659.16 661.96 664.77 667.57 670.38 673.18 675.98 678.79 681.59 684.39 687.20

N O T E S

1. GLOSSARY

- I = INVERT ELEVATION
- C = CRITICAL DEPTH
- W = WATER SURFACE ELEVATION
- H = HEIGHT OF CHANNEL
- E = ENERGY GRADE LINE
- X = CURVES CROSSING OVER
- B = BRIDGE ENTRANCE OR EXIT
- Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT

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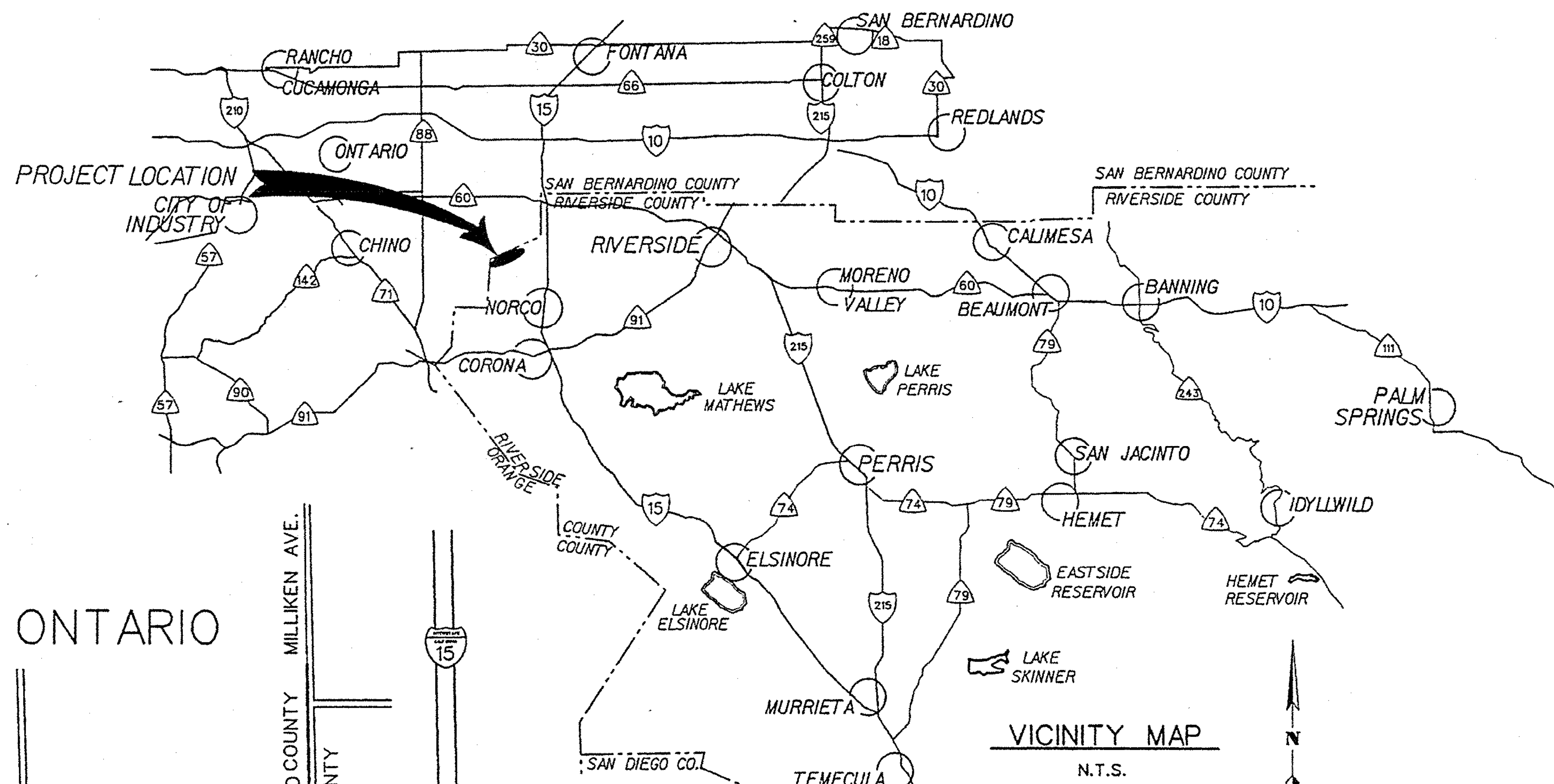
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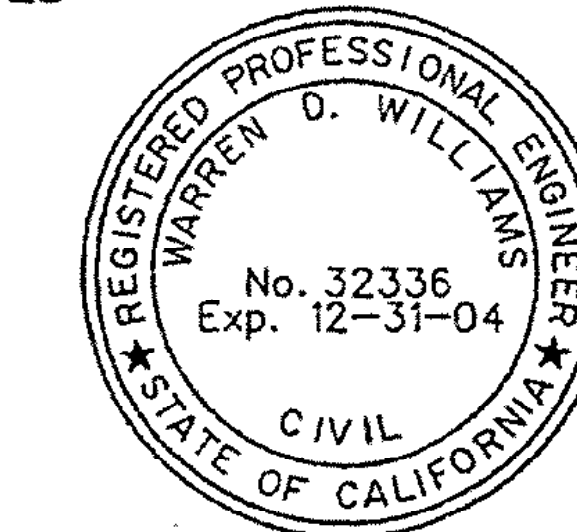
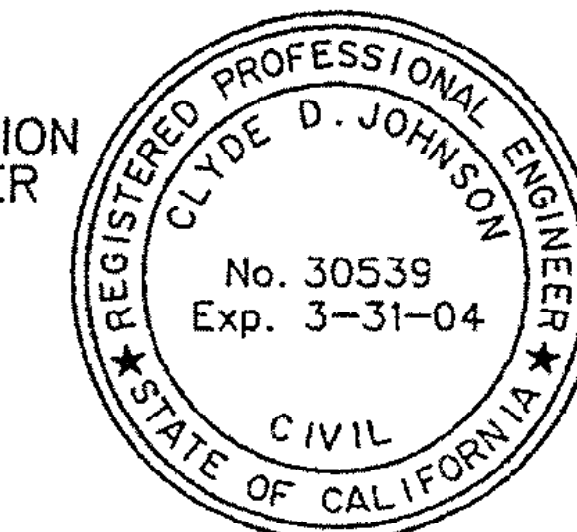
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GENERAL NOTES

- ALL STATIONING REFERS TO CENTERLINE OF CONSTRUCTION.
- ALL CHANNEL/STORM DRAIN REFERENCES AND CROSS SECTIONS ARE TAKEN LOOKING DOWNSTREAM.
- TOPOGRAPHY BY DIGITAL PHOTOGRAMMETRIC METHODS. AERIAL PHOTOGRAPHS TAKEN AT AN ALTITUDE NOT TO EXCEED A FLYING HEIGHT TO CONTOUR INTERVAL RATIO OF 1800. PHOTOGRAPHY DATED 12-19-00. THE EXISTING TOPOGRAPHY MAY BE DIFFERENT THAN AS SHOWN ON THE DRAWINGS.
- THE VERTICAL DATUM IS DERIVED FROM NAVD 88. THE HORIZONTAL DATUM IS DERIVED FROM NAD 83.
- STANDARD DRAWINGS CALLED FOR ON THE PLAN & PROFILE SHALL CONFORM TO R.C.F.C. & W.C.D. STD. DRAWINGS, UNLESS OTHERWISE NOTED.
- ELEVATIONS AND LOCATIONS OF UTILITIES WERE OBTAINED FROM AVAILABLE INFORMATION AND ARE SHOWN APPROXIMATELY ON THESE PLANS. 48 HOURS BEFORE EXCAVATION CALL UNDERGROUND SERVICE ALERT AT 1-800-227-2600. ALL UTILITIES SHALL BE PROTECTED IN PLACE EXCEPT AS NOTED ON PLANS AND SPECIFICATIONS.
- THE CONTRACTOR IS REQUIRED TO CONTACT ALL UTILITY AGENCIES REGARDING TEMPORARY SUPPORT AND SHORING REQUIREMENTS FOR THE VARIOUS UTILITY LINES SHOWN ON THESE PLANS.
- ALL OPENINGS RESULTING FROM CUTTING OR PARTIAL REMOVAL OF EXIST. CULVERTS, PIPES, OR SIMILAR STRUCTURES TO BE ABANDONED, SHALL BE SEALED AT BOTH ENDS WITH 6" MIN. CLASS "B" CONCRETE.
- STREET RECONSTRUCTION SHALL BE 0.25" TYPE "B" ASPHALT CONCRETE OVER 6" AGGREGATE BASE OR IN KIND, WHICHEVER IS GREATER. EXISTING PAVEMENT ON THE SOUTH SIDE OF BELLEGRAVE AVENUE IS .43" ASPHALT CONCRETE OVER .75" AGGREGATE BASE. RECONSTRUCTION OF ARCHIBALD AVE. PAVEMENT SHALL BE .43" ASPHALT CONCRETE OVER 0.50" AGGREGATE BASE.
- ALL RECONSTRUCTION, RESURFACING AND PAVEMENT DELINEATION, CURBS, SIDEWALKS AND OTHER IMPROVEMENTS ARE TO BE RECONSTRUCTED IN KIND AT THE SAME LOCATIONS AND ELEVATIONS AS THE EXISTING IMPROVEMENTS, UNLESS OTHERWISE NOTED.
- ⊕ INDICATES APPROX. SOIL BORING LOCATION PER SOILS REPORT DATED 10/09/02.
- FOR COMPLETE RIGHT-OF-WAY AND TEMPORARY EASEMENT REFER TO DRAWING NO. 2-322, SHT. 1 TO 14 AND DRAWING NO. 2-322T SHT. 1 TO 14 RESPECTIVELY.



COUNTY OF RIVERSIDE
TRANSPORTATION DEPARTMENT

APPROVED BY: _____

DEPUTY DIRECTOR OF TRANSPORTATION

DATE: _____

COUNTY OF SAN BERNARDINO
PUBLIC WORKS DEPARTMENT

APPROVED BY: _____

PUBLIC WORKS DIRECTOR

DATE: _____

CITY OF ONTARIO

APPROVED BY: _____

CITY ENGINEER

DATE: _____



BENCH MARK
M.L. 33 RESET JULY 1964

A BRASS DISK SET IN THE TOP OF A CONCRETE GAS COMPANY VAULT AT THE SOUTHWEST CORNER OF THE "T" INTERSECTION OF ARCHIBALD AVE. AND CLOVERDALE ROAD.

NAVD "88" ELEV=647.564

REF.	DESCRIPTION	APPR.	DATE	CHECKED BY:

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT

DESIGNED BY: G.H./A.MARTINEZ

DRAWN BY: M. HOWARD

DATE DRAWN: DEC. 2003

RECOMMENDED FOR APPROVAL BY: _____

CHIEF, DESIGN & CONSTRUCTION

DATE: 12/30/03 R.E. No. 30539

APPROVED BY: _____

CHIEF ENGINEER

DATE: 12/30/03 R.E. No. 32336

COUNTY LINE CHANNEL

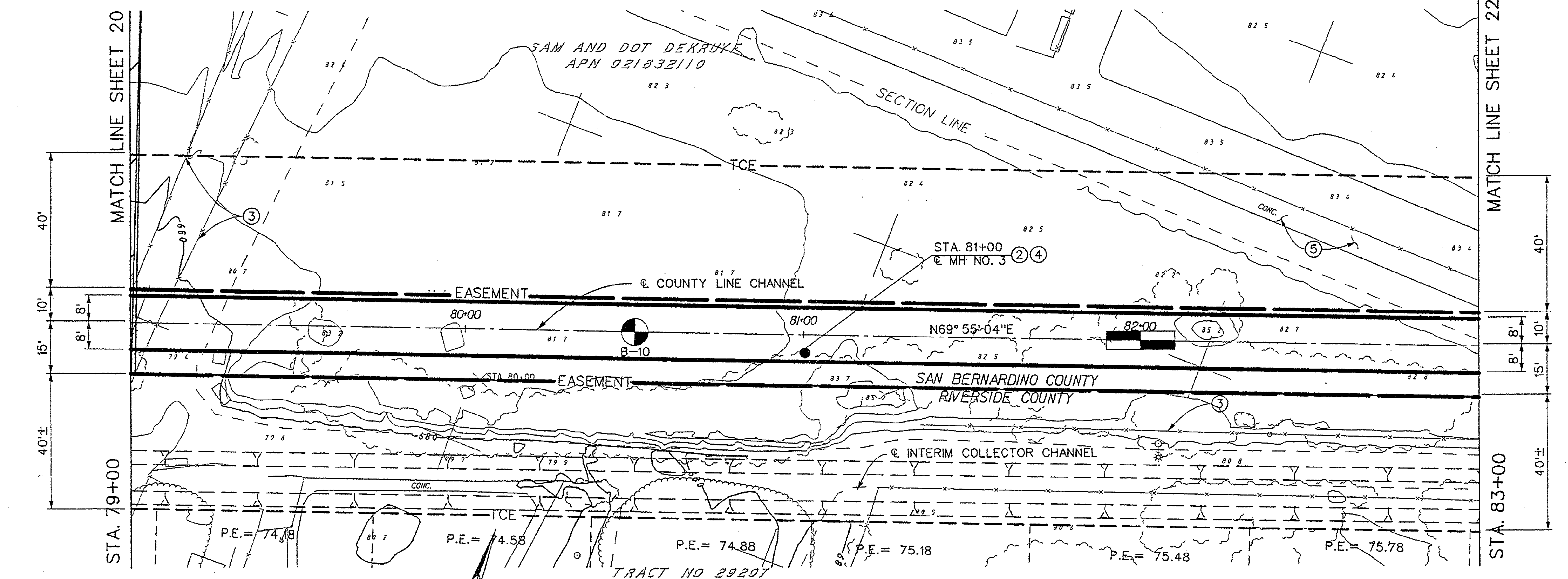
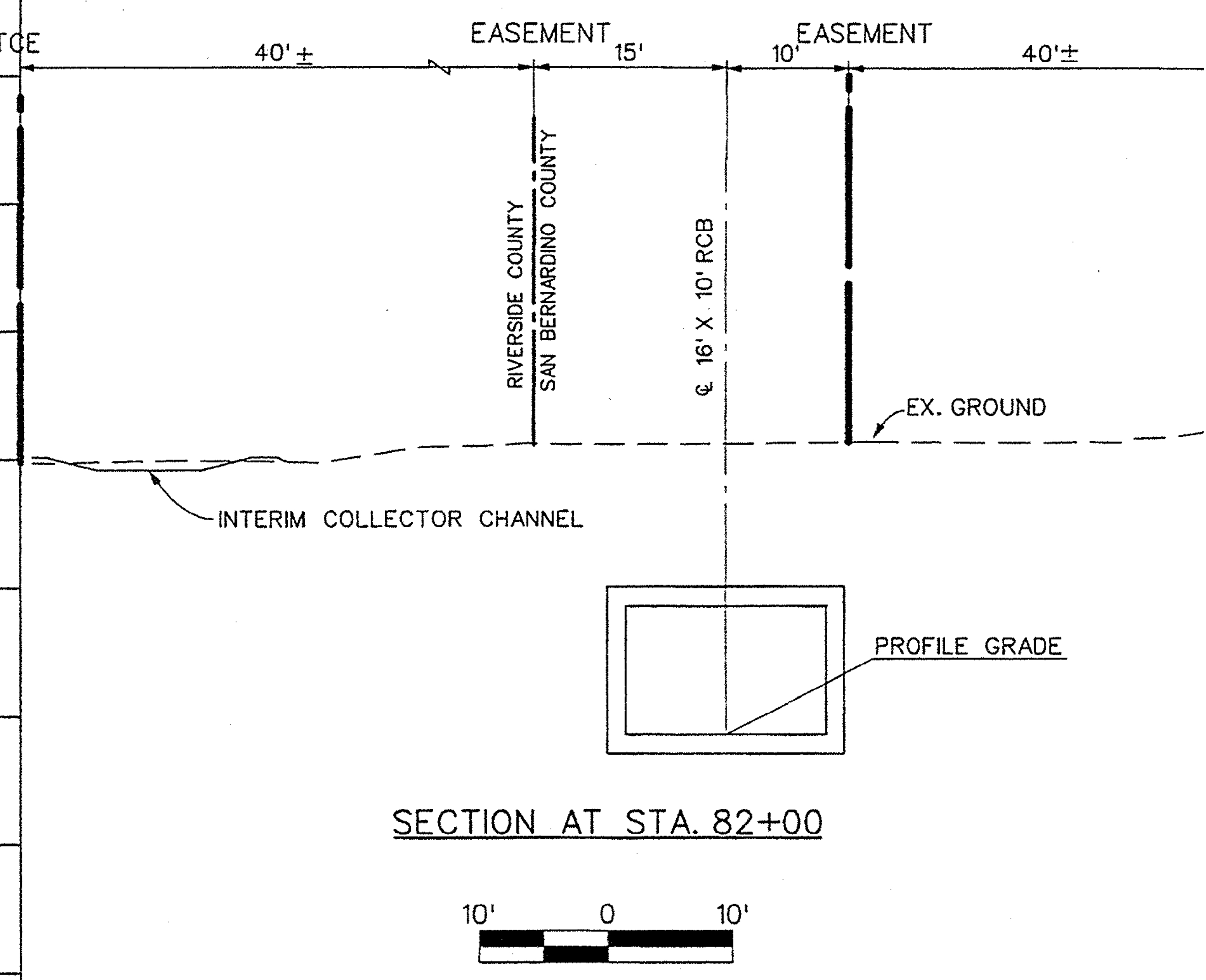
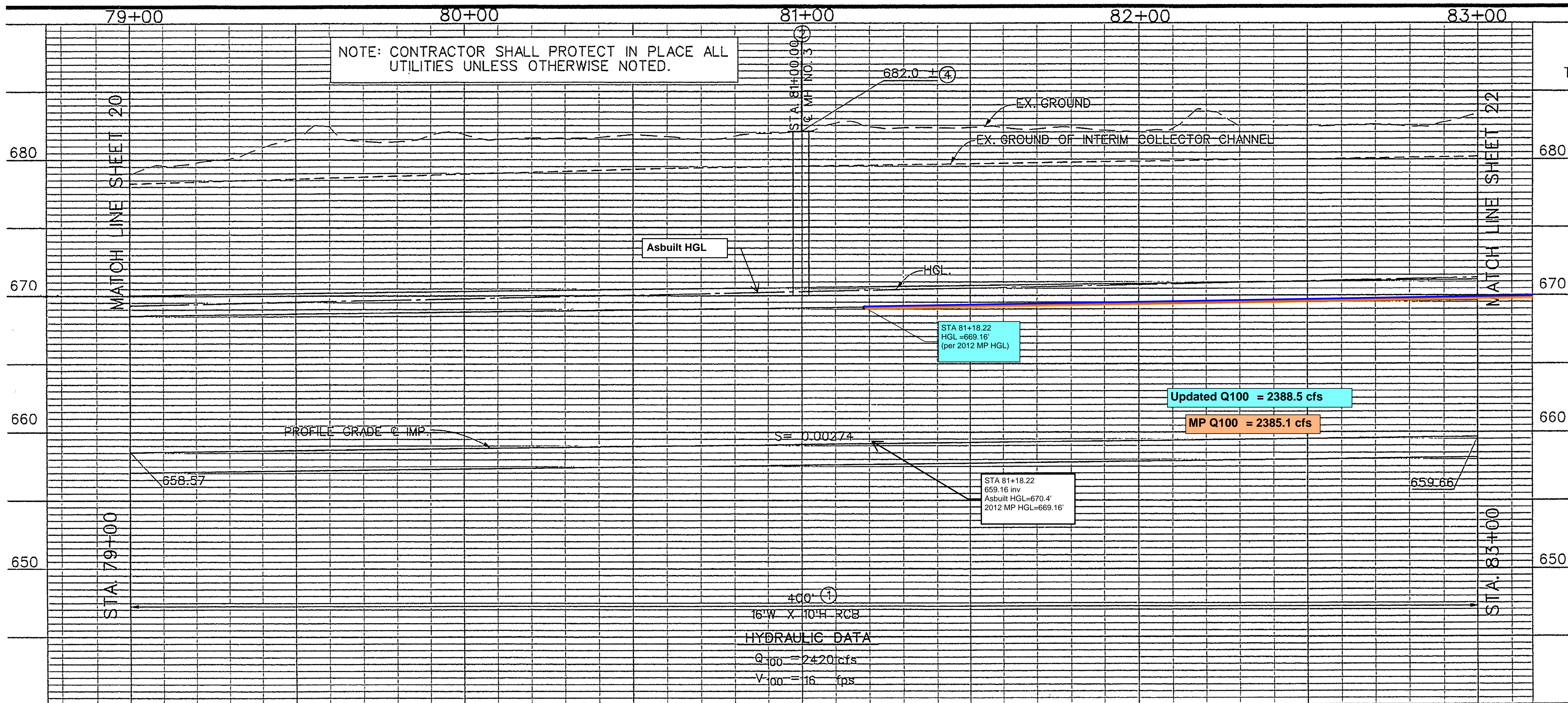
TITLE SHEET

PROJECT NO. 2-0-0.

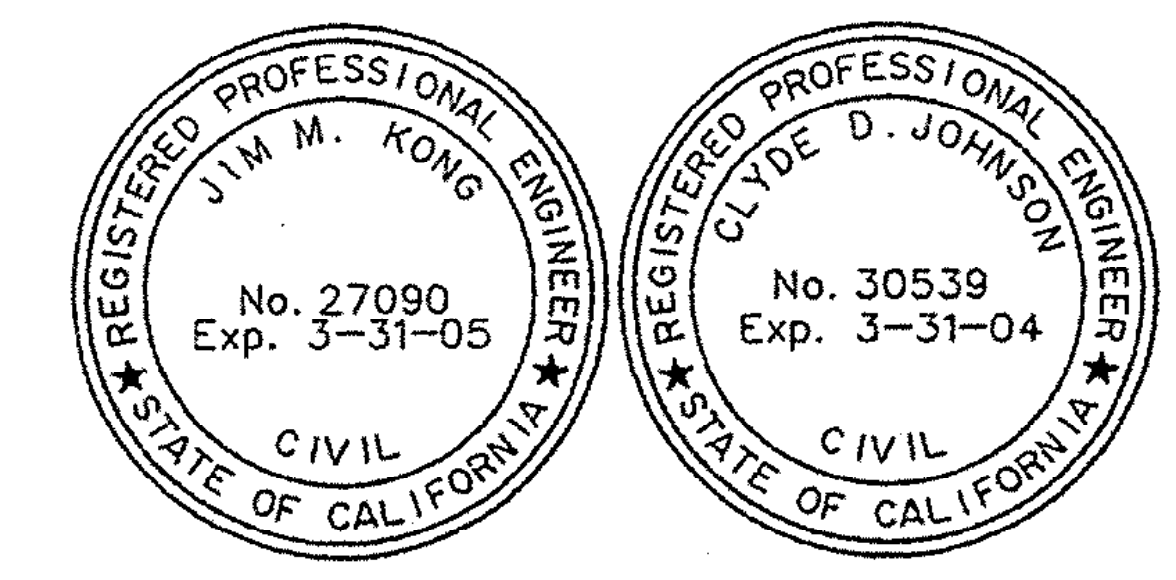
DRAWING NO. 2-31

SHEET NO. 1 OF 1

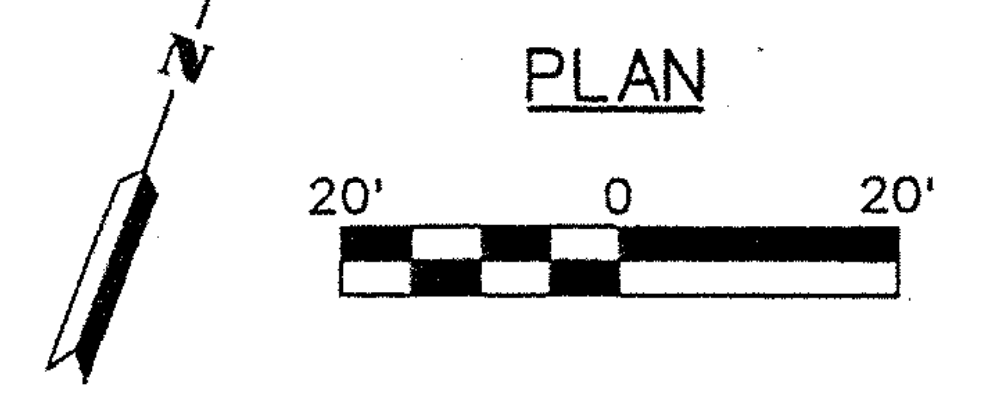
File #1-317-1



- NOTES**
- ① CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - ② CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - ③ REMOVE EXISTING FENCE AND COW FEEDER.
 - ④ CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
 - ⑤ REMOVE AND REPLACE COW FEEDER AND METAL FENCE AS REQUIRED.



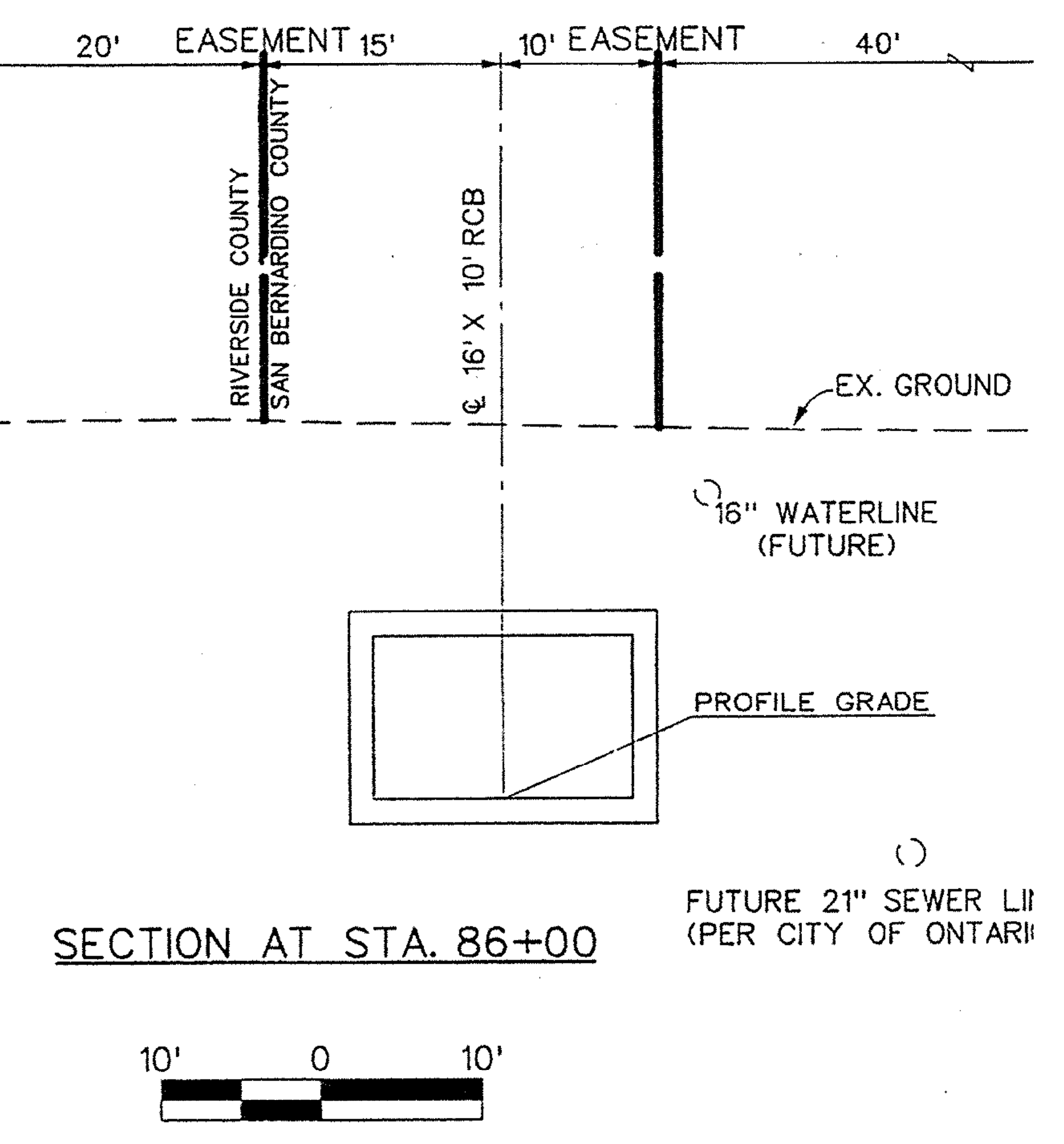
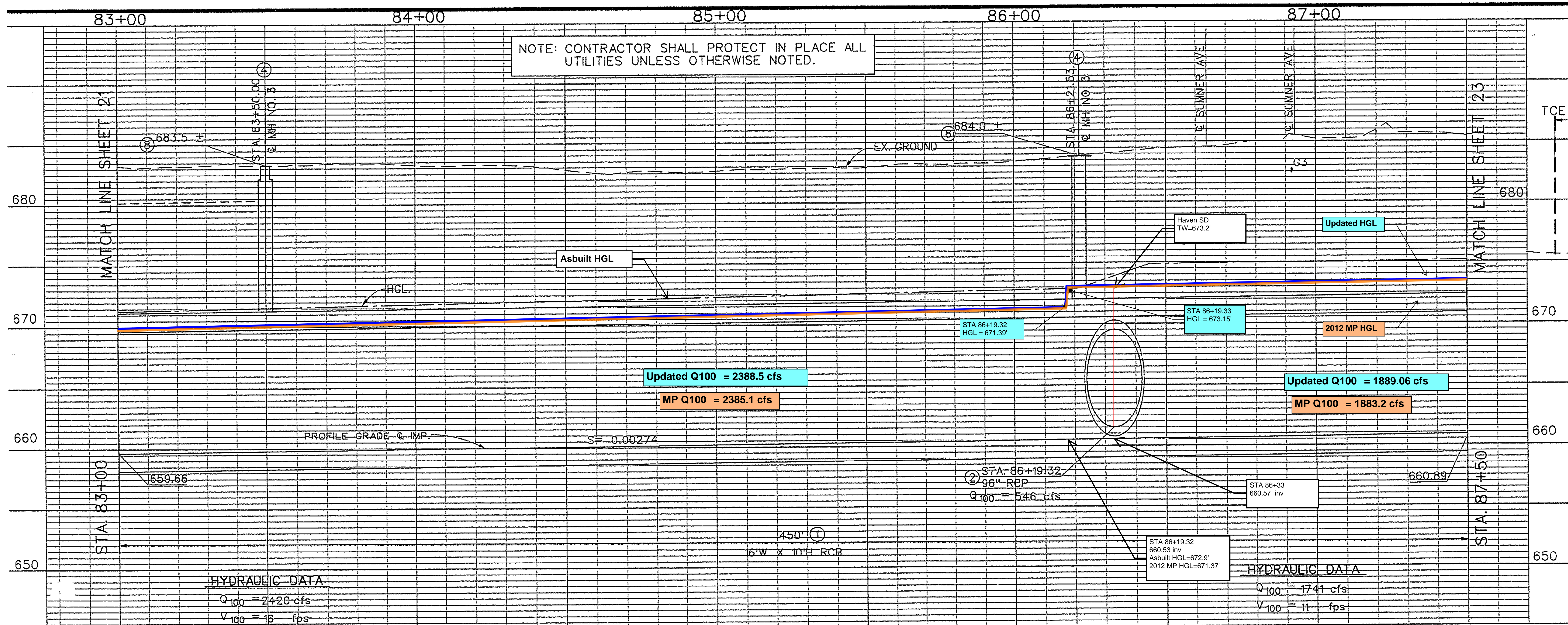
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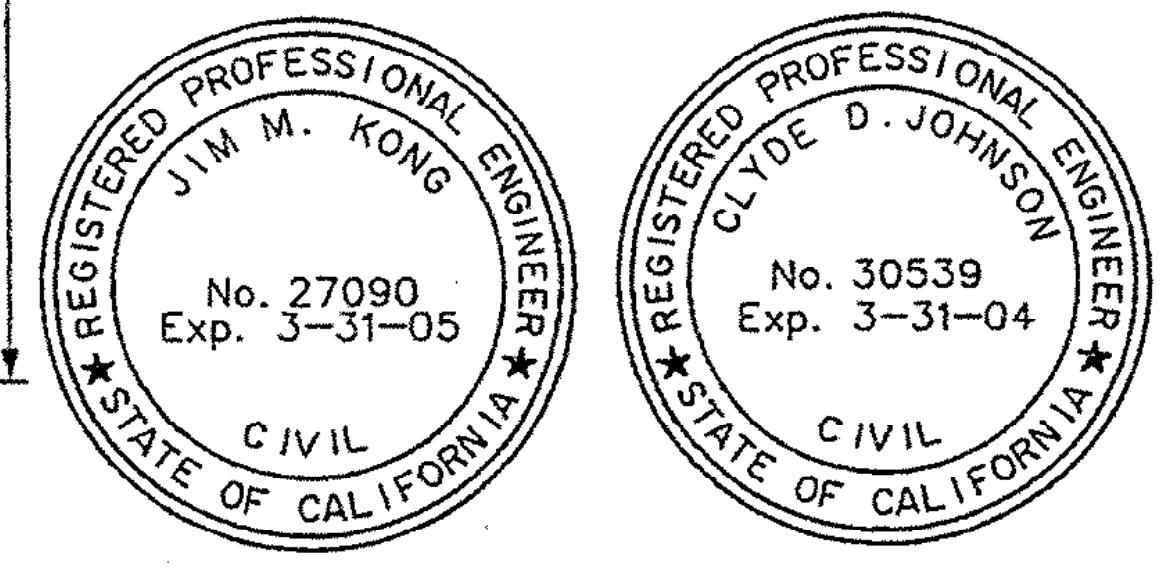
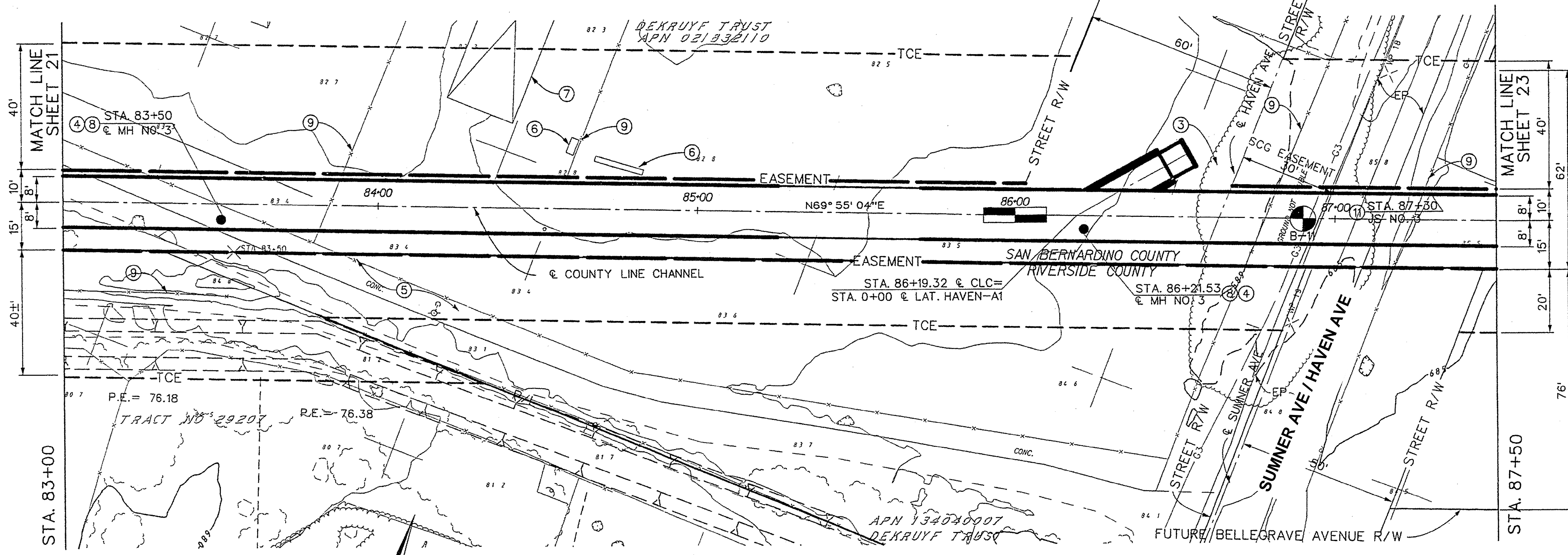
NO.	DESCRIPTION	DATE

DESIGNED BY: G.H./A.MARTINEZ		
DRAWN BY: M. HOWARD		
DATE DRAWN: DEC. 2003		
CHECKED BY:		
RECOMMENDED FOR APPROVAL BY:		
APPROVED BY:		
DATE: 12/30/03	R.E. No. 27090	DATE: 12/30/03 R.E. No. 30539

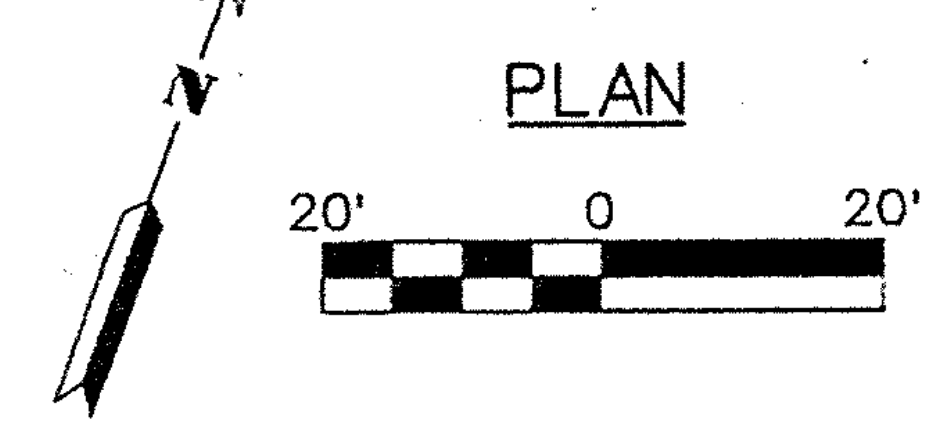
RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT	PROJECT NO.	2-0-0
	DRAWING NO.	2-31
	SHEET NO.	21 OF
COUNTY LINE CHANNEL		
STA. 79+00 TO STA. 83+00		



- NOTES**
- CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - CONSTRUCT JS NO. 1 PER STD. JS226 A= 30°, B= 96", C= 31. ELEV. R= 660.94, ELEV. S= 660.87. SEE PROFILE ON SHEET 62.
 - REMOVE EXISTING TREES AS REQUIRED FOR CONSTRUCTION.
 - CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPI THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FR AND COVER PER SBCFC STD. 205.
 - REMOVE AND REPLACE COW FEEDER AND METAL FENCE AS REC
 - REMOVE AND REINSTALL EXISTING WATER FEED.
 - PROTECT EXISTING COW SHED.
 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260
 - REMOVE AND REINSTALL EXISTING FENCE AS NECESSARY.



#1-317-

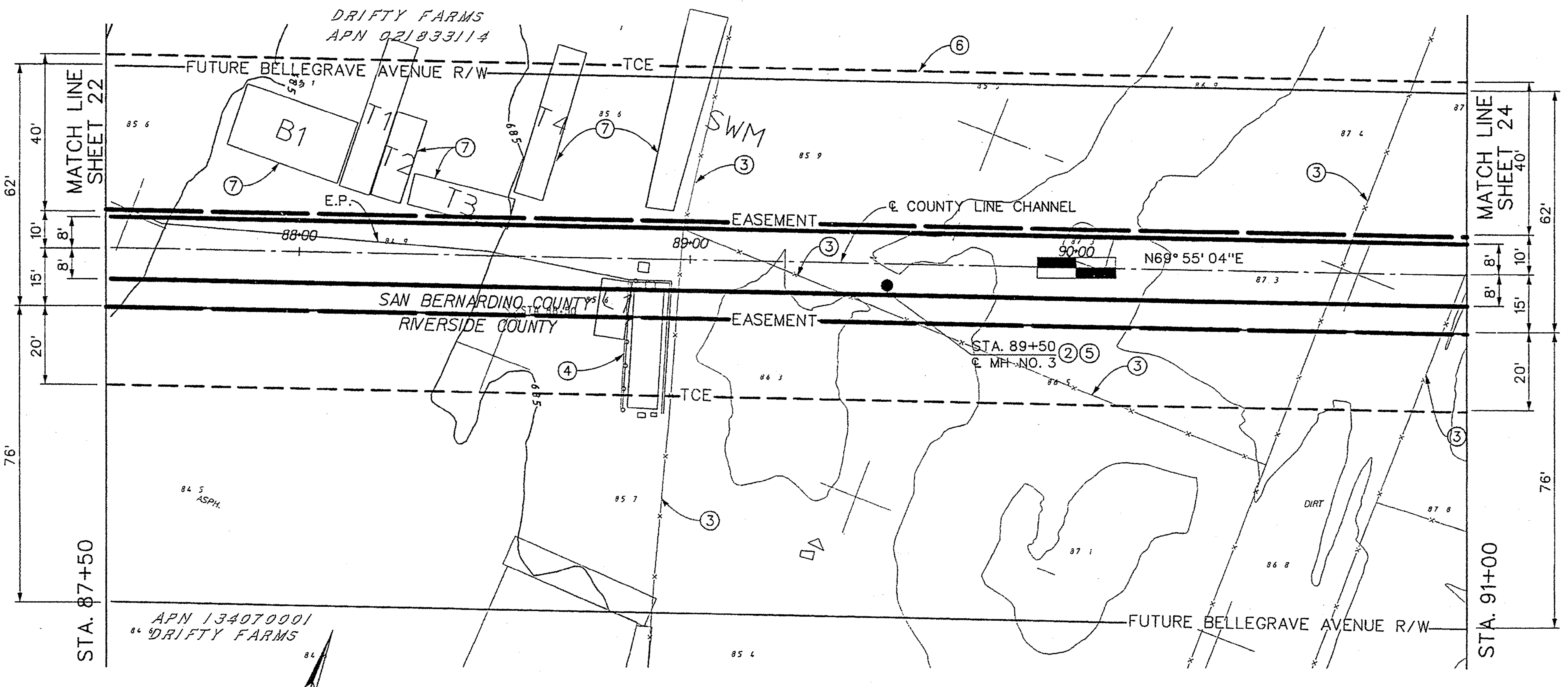
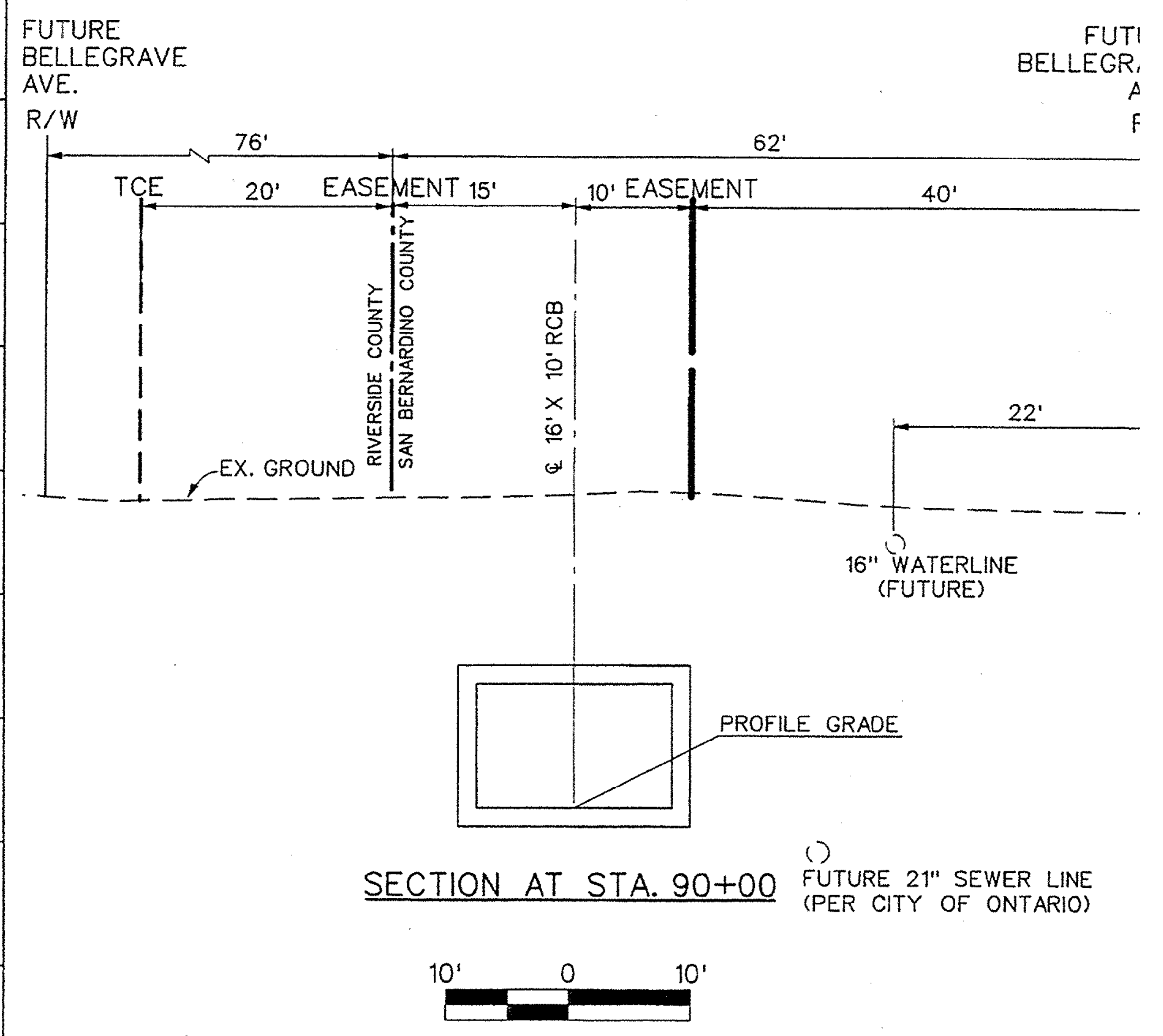
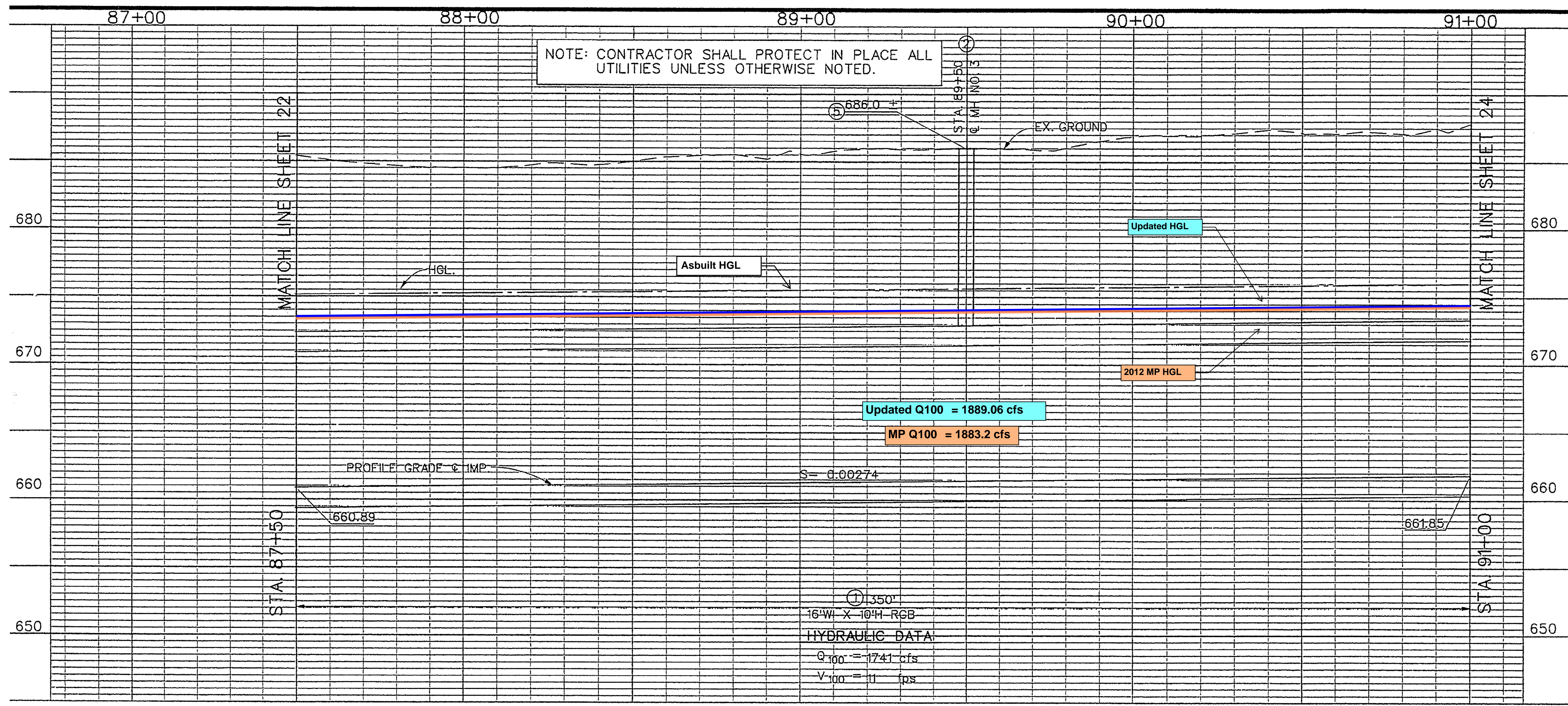


BENCH MARK
M.L. 34-1-64.
A BRASS DISK SET IN THE TOP OF A CONCRETE POST AT THE NORTHEAST CORNER OF THE "T" INTERSECTION OF ORANGE ST. AND SUMNER AVE 35.0' EAST OF SUMNER AVE, 21.0' NORTH OF ORANGE ST.
NAVD "88" ELEV. = 609.290

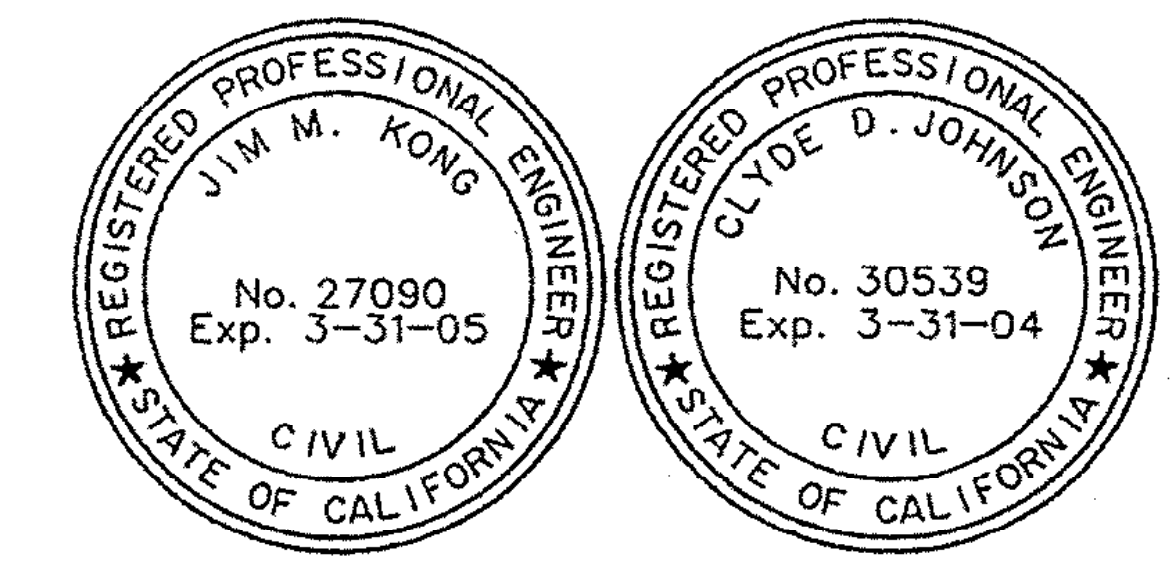
REF.	DESCRIPTION	APPR.	DATE	CHECKED BY:

DESIGNED BY: G.H. / A. MARTINEZ			RECOMMENDED FOR APPROVAL BY:			APPROVED BY:		
DRAWN BY: M. HOWARD			[Signature]			[Signature]		
DATE DRAWN: DEC. 2003			DATE: 12/20/03 R.E. No. 27090			DATE: 12/20/03 R.E. No. 30539		

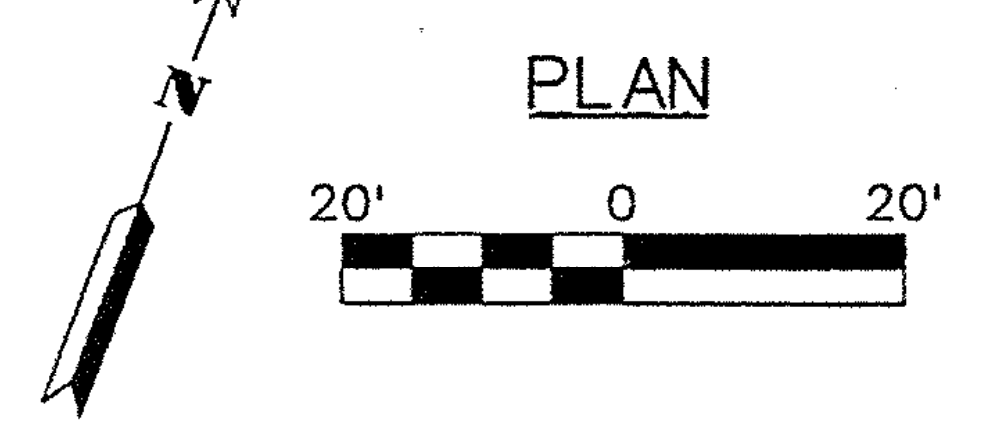
RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		PROJECT NO. 2-0-(
COUNTY LINE CHANNEL		DRAWING NO. 2-3	
STA. 83+00 TO STA. 87+50		SHEET NO. 22 OF 27	



- NOTES**
- CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - REMOVE EXISTING FENCE AS REQUIRED.
 - EXISTING ABOVE GROUND DIESEL STORAGE TANK TO BE RELOCATED BY OTHERS.
 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
 - INSTALL 6' TEMPORARY CHAIN LINK FENCE.
 - EXISTING BUILDING, TRAILERS AND RELATED STRUCTURES TO BE RELOCATED BY OTHERS.



#1-317-1

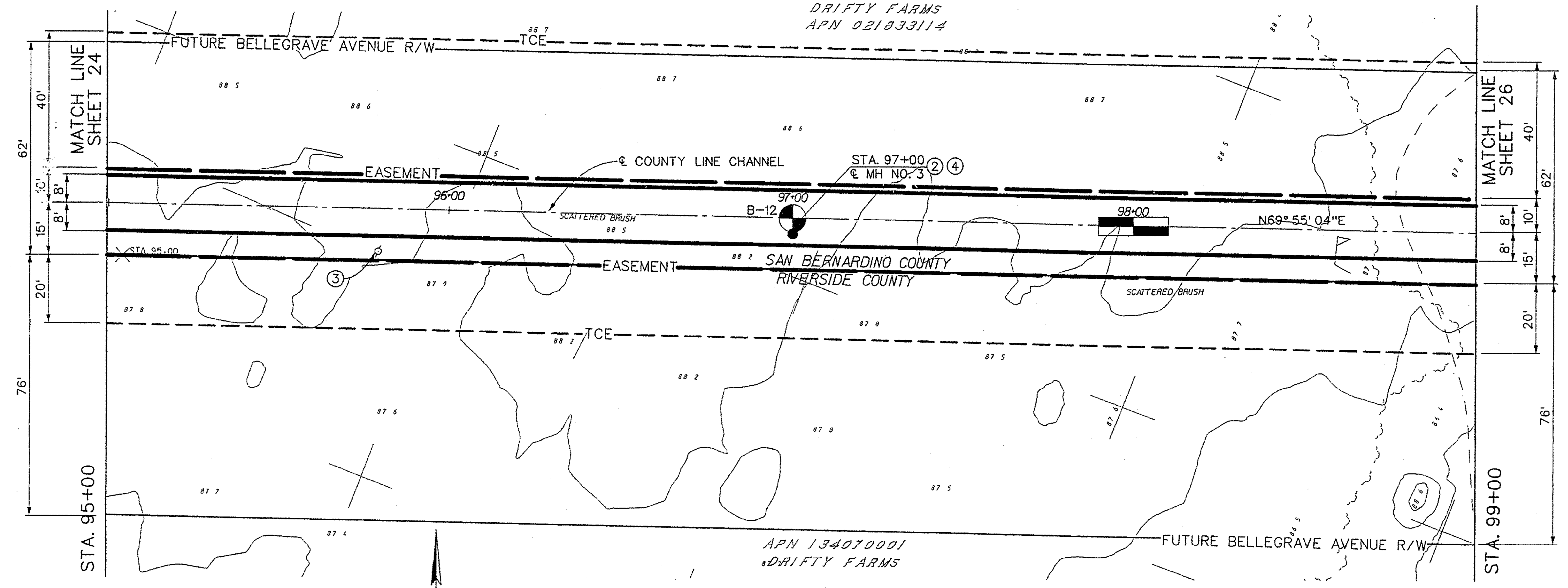
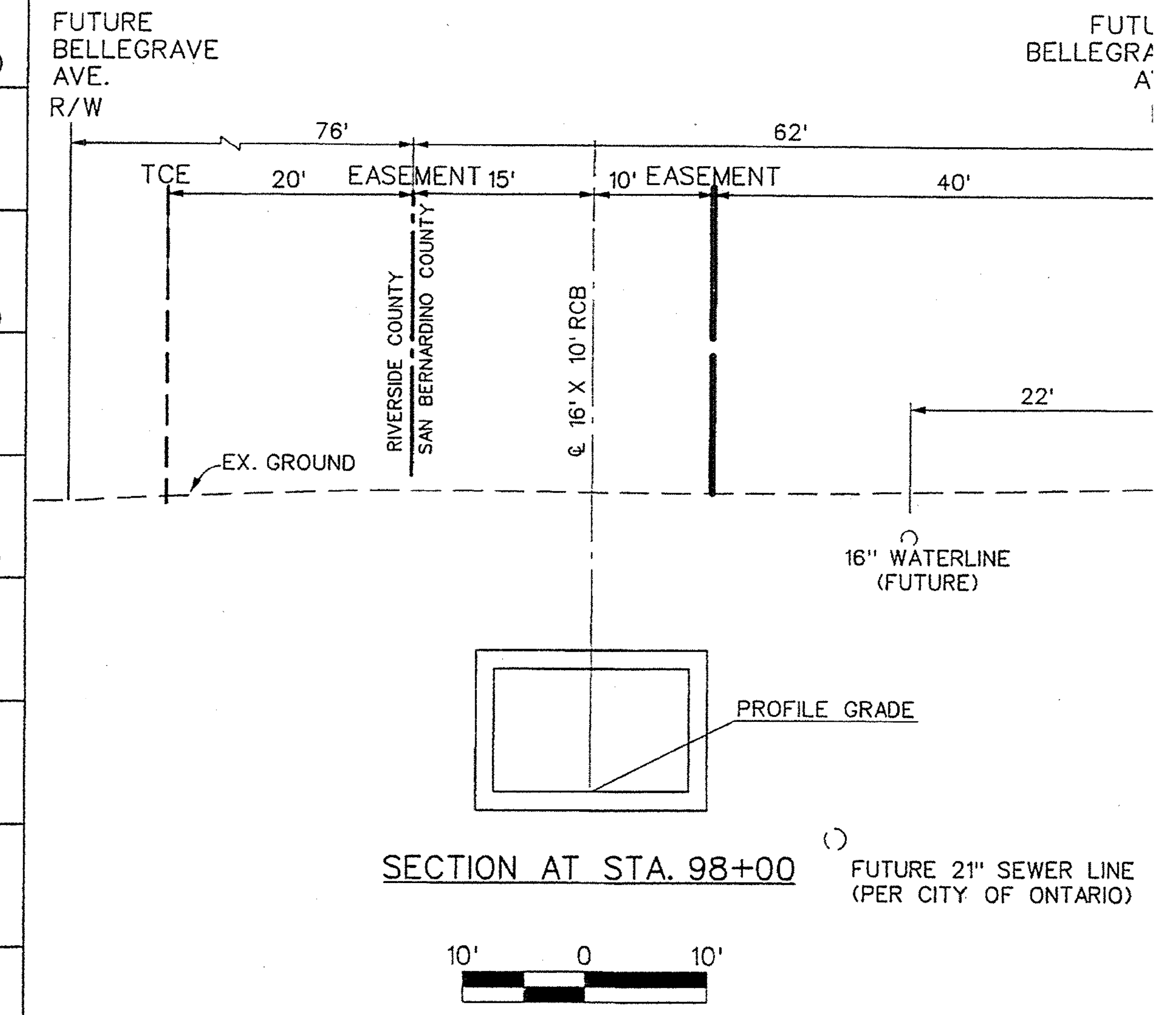
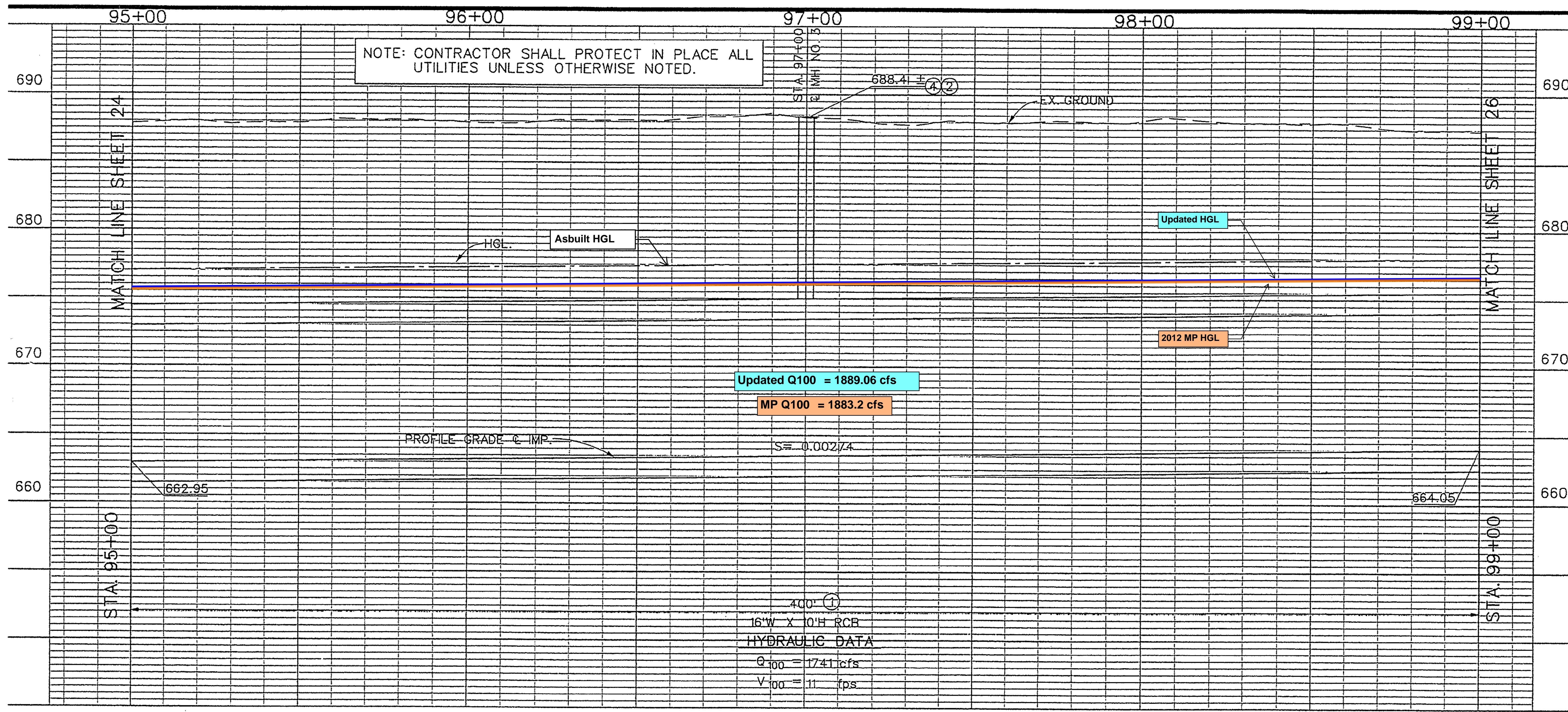


BENCH MARK AT THE NORTHEAST CORNER OF THE 1ST INTERSECTION OF ORANGE ST. AND SUMNER AVE. 35.0' EAST OF SUMMER AVE, 21.0' NORTH OF ORANGE ST., 2.5' WEST OF POWER POLE #375282 1.0' EAST OF A MARKER POST. A BRASS DISK SET IN THE TOP OF A CONCRETE POST AND MARKED M.L. 34-1. APRIL 1964 NAVD 88 ELEV=609.29

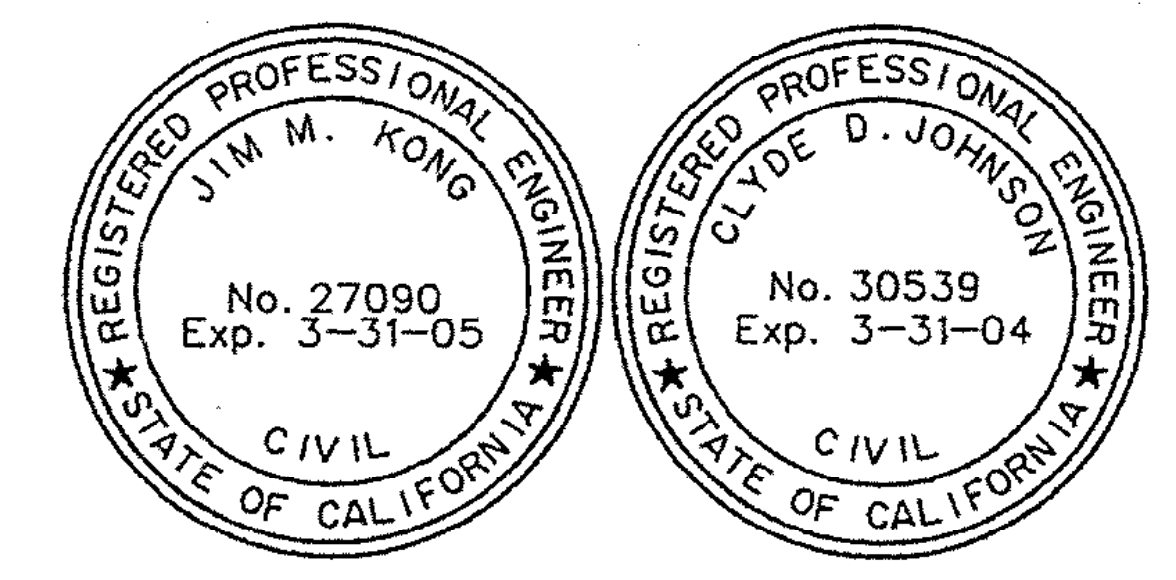
REF.	DESCRIPTION	APPR.	DATE	CHECKED BY:

DESIGNED BY: G.H./A.MARTINEZ	RECOMMENDED FOR APPROVAL BY: <i>[Signature]</i>	APPROVED BY: <i>[Signature]</i>
DRAWN BY: M. HOWARD	DATE DRAWN: DEC. 2003	DATE: 12/20/03 R.E. No. 27090

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		PROJECT NO. 2-0-0
COUNTY LINE CHANNEL		DRAWING NO. 2-31
STA. 87+50 TO STA. 91+00		SHEET NO. 23 OF



- NOTES**
- ① CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - ② CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - ③ EXISTING POWER POLE TO BE RELOCATED BY OTHERS.
 - ④ CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.



#1-317-1



REF.	DESCRIPTION	APPR.	DATE	CHECKED BY:

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT

DESIGNED BY: G.H./A.MARTINEZ
 DRAWN BY: M. HOWARD
 DATE DRAWN: DEC. 2003

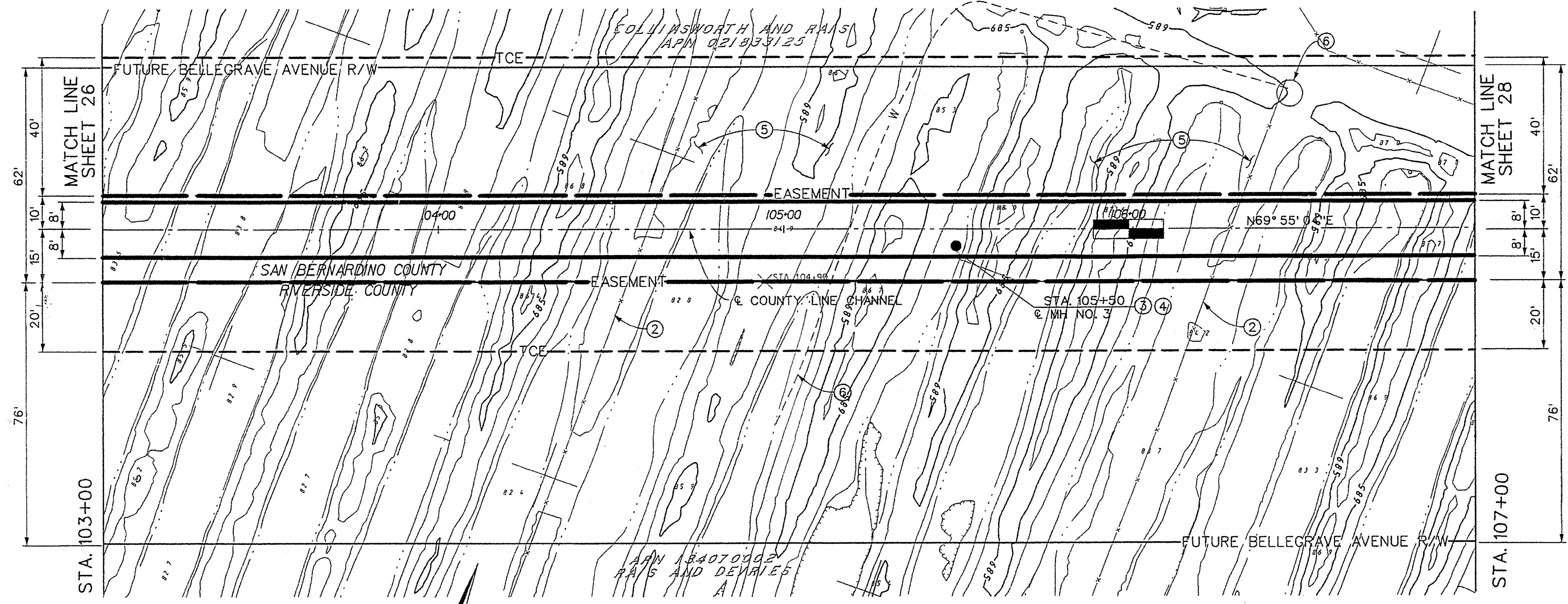
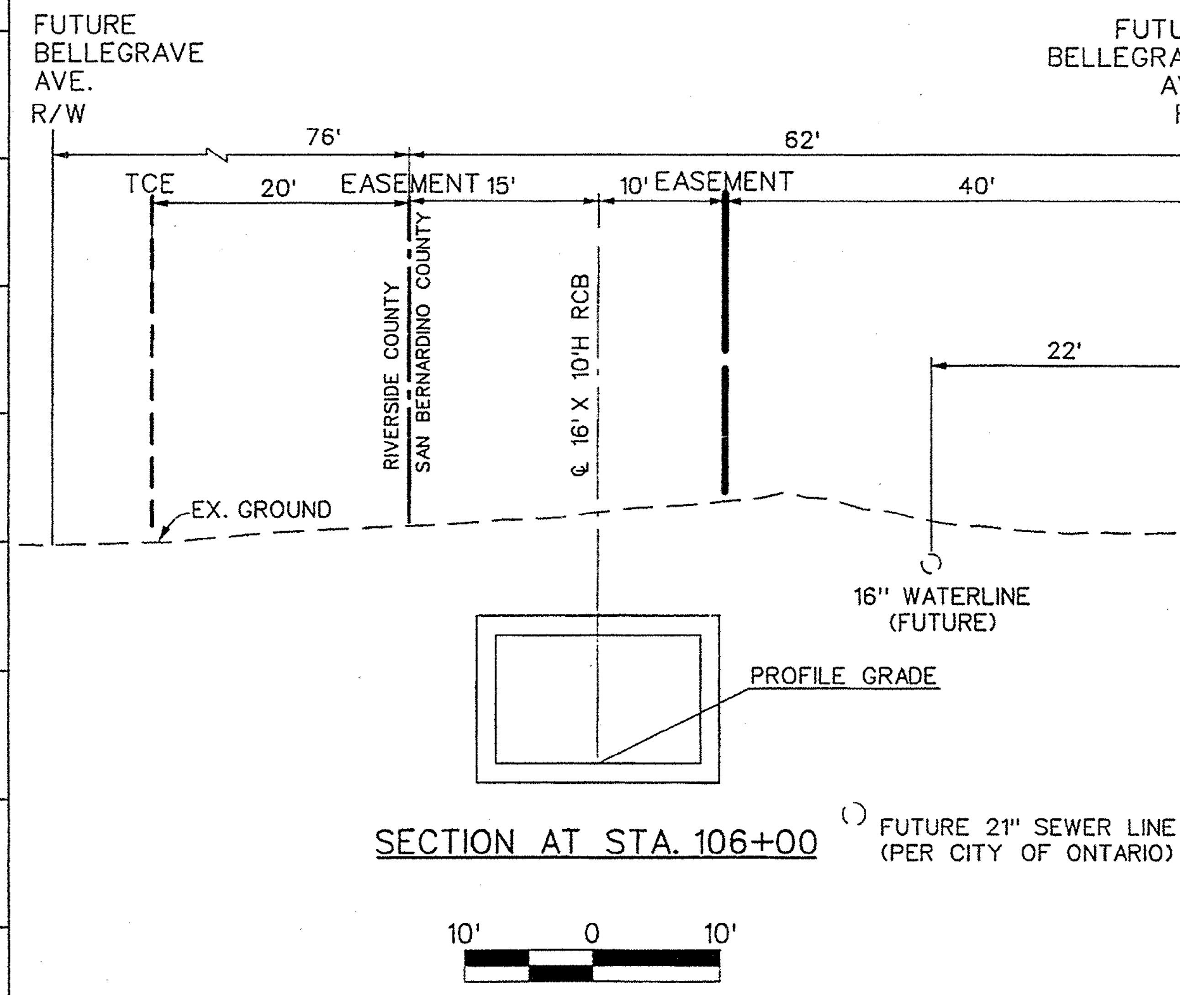
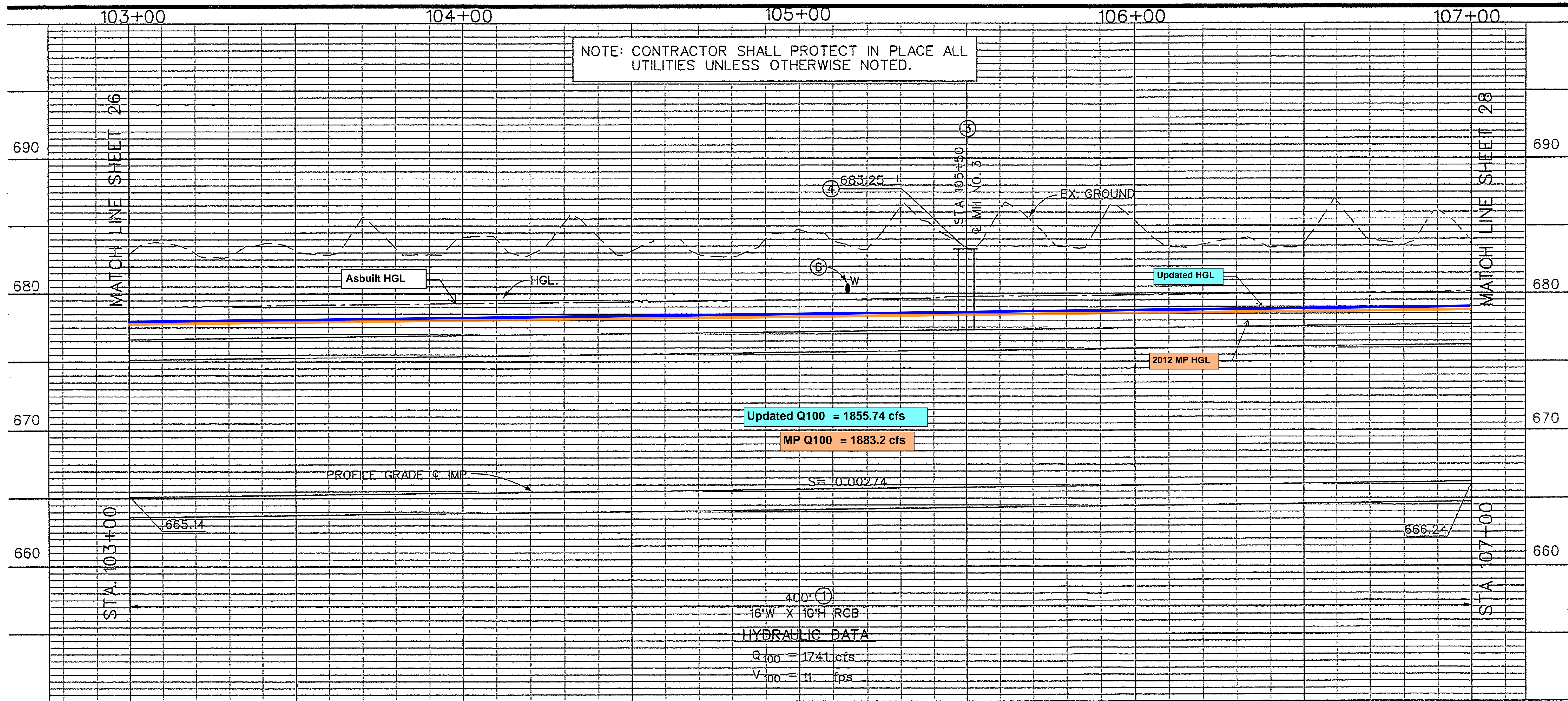
RECOMMENDED FOR APPROVAL BY: [Signature]
 SENIOR CIVIL ENGINEER
 DATE: 12/10/03 R.E. No. 27090

APPROVED BY: [Signature]
 CHIEF DESIGN & CONSTRUCTION
 DATE: 12/10/03 R.E. No. 30539

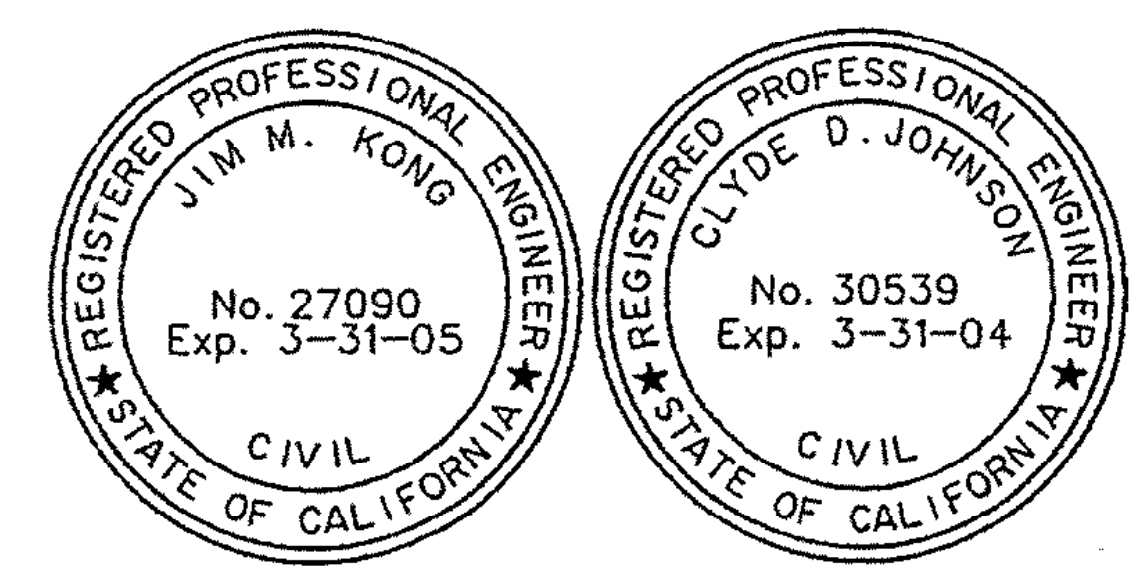
COUNTY LINE CHANNEL

STA. 95+00 TO STA. 99+00

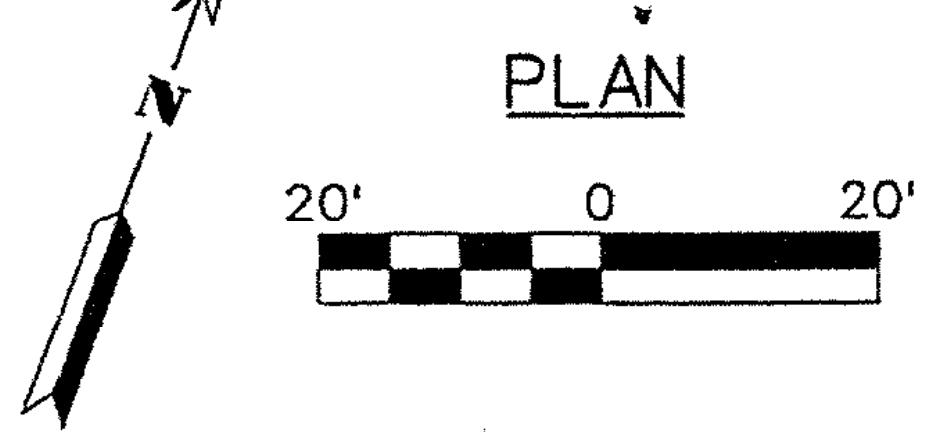
PROJECT NO. 2-0-0
 DRAWING NO. 2-31
 SHEET NO. 25 OF 1



- NOTES**
- CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - REMOVE AND REINSTALL EXISTING FENCE AS REQUIRED.
 - CONSTRUCT MANHOLE NO. 3 PER STD. MH253, MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
 - RESTORE EXISTING SWALE TO ORIGINAL CONDITION AFTER CONSTRUCTION.
 - MAINTAIN THE OPERATION OF THE EXISTING DAIRY WASTE WATER DISTRIBUTION SYSTEM DURING CONSTRUCTION. RELOCATE AND REPLACE THE WATER LINE AS REQUIRED.



#1-317-1



NO.	REVISIONS	DATE	BY

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT

DESIGNED BY: G.H./A.MARTINEZ
 DRAWN BY: M. HOWARD
 DATE DRAWN: DEC. 2003

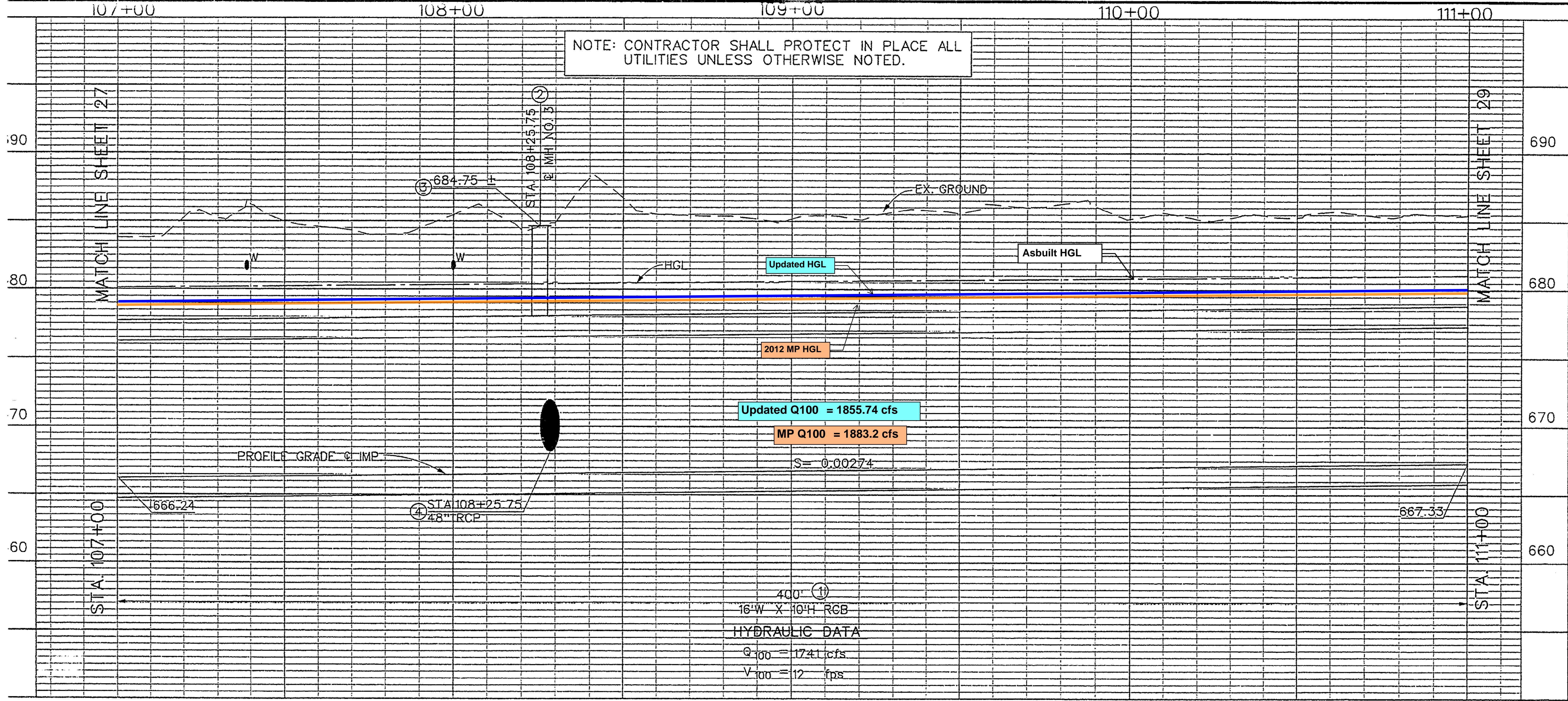
RECOMMENDED FOR APPROVAL BY: [Signature]
 SENIOR CIVIL ENGINEER
 DATE: 12/20/02 R.E. No. 27090

APPROVED BY: [Signature]
 CHIEF, DESIGN & CONSTRUCTION
 DATE: 12/20/02 R.E. No. 30539

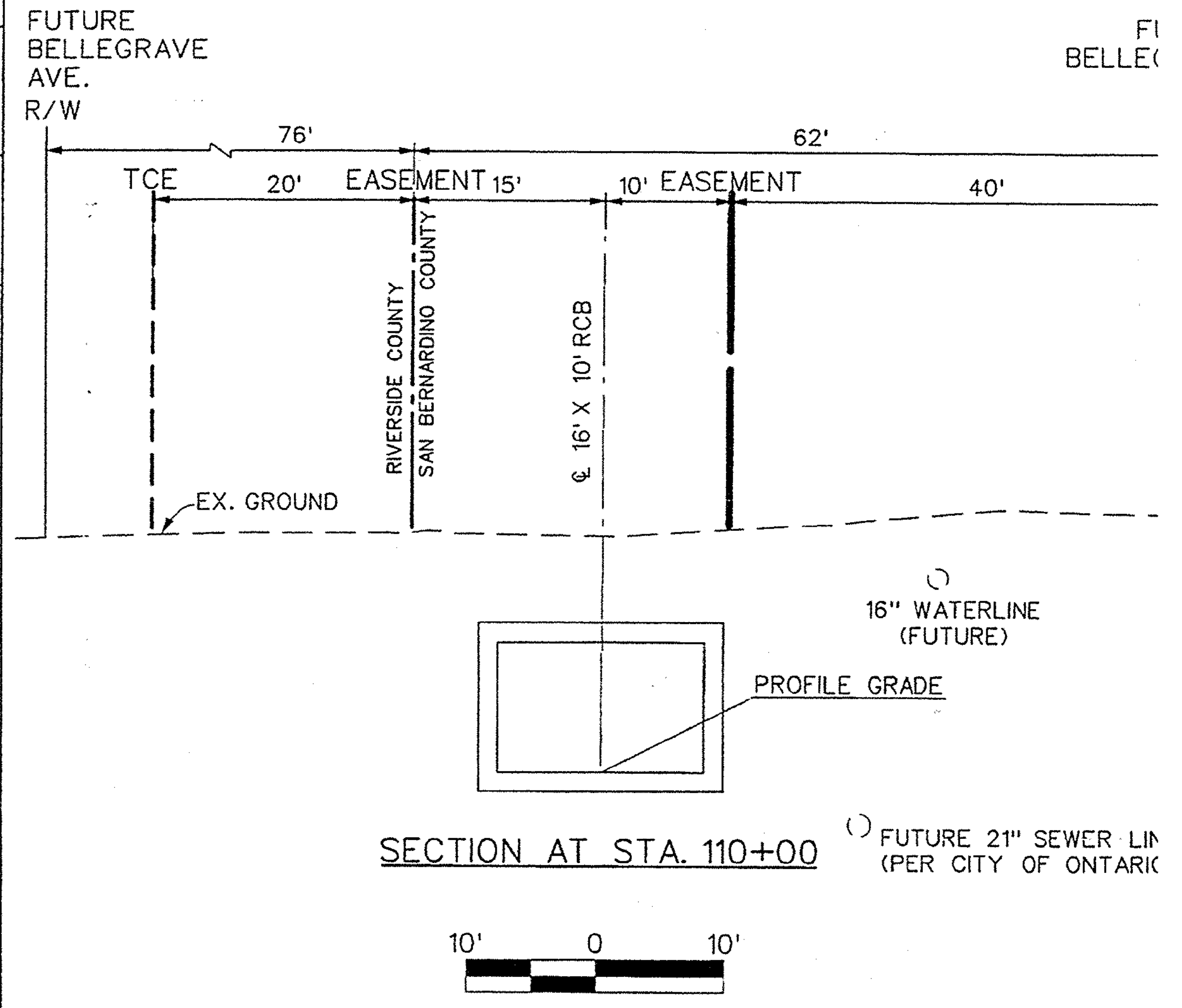
COUNTY LINE CHANNEL

PROJECT NO. 2-0-C
 DRAWING NO. 2-3
 SHEET NO. 27 OF

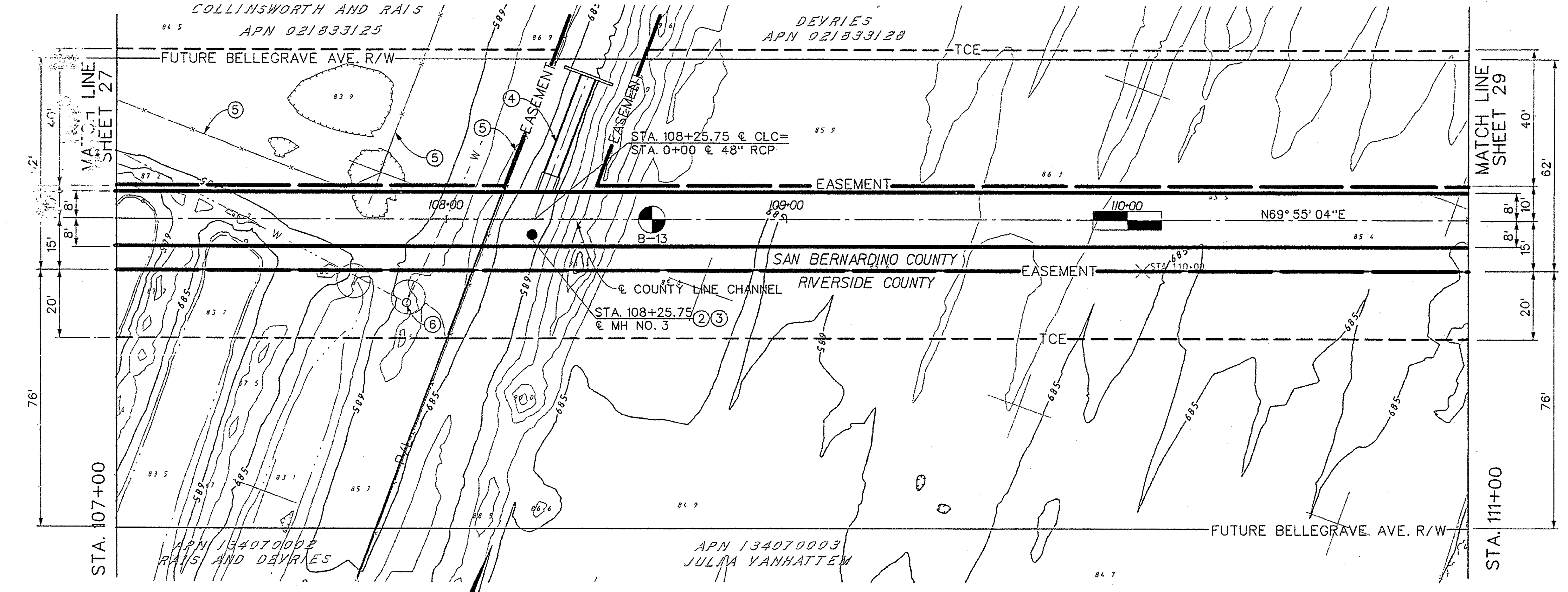
STA. 103+00 TO STA. 107+00



NOTE: CONTRACTOR SHALL PROTECT IN PLACE ALL UTILITIES UNLESS OTHERWISE NOTED.

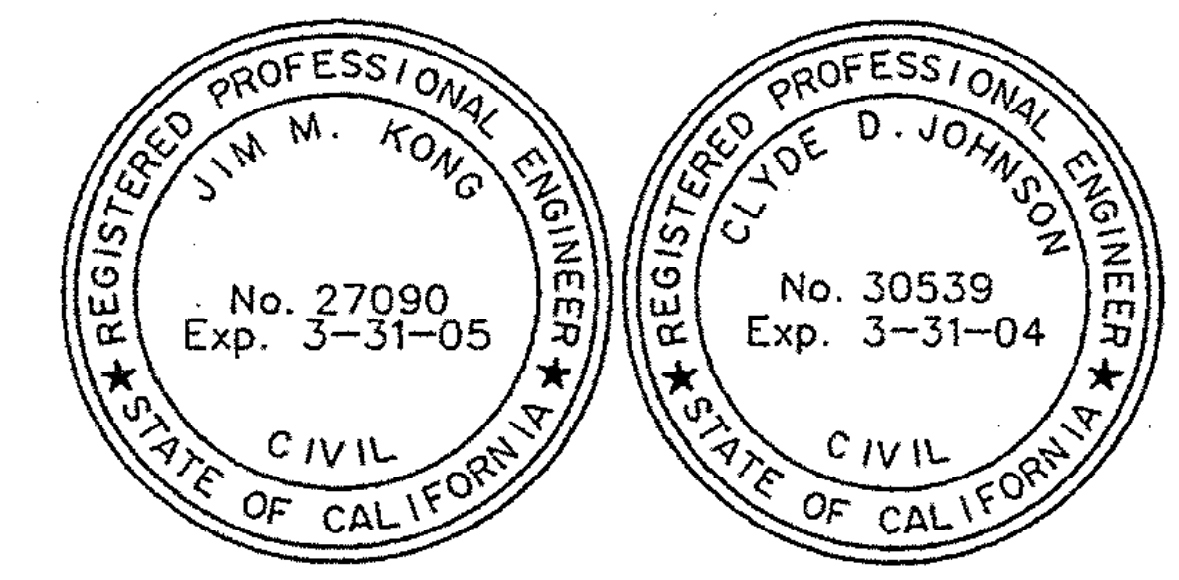


SECTION AT STA. 110+00 (1) FUTURE 21" SEWER LIN (PER CITY OF ONTARIO)



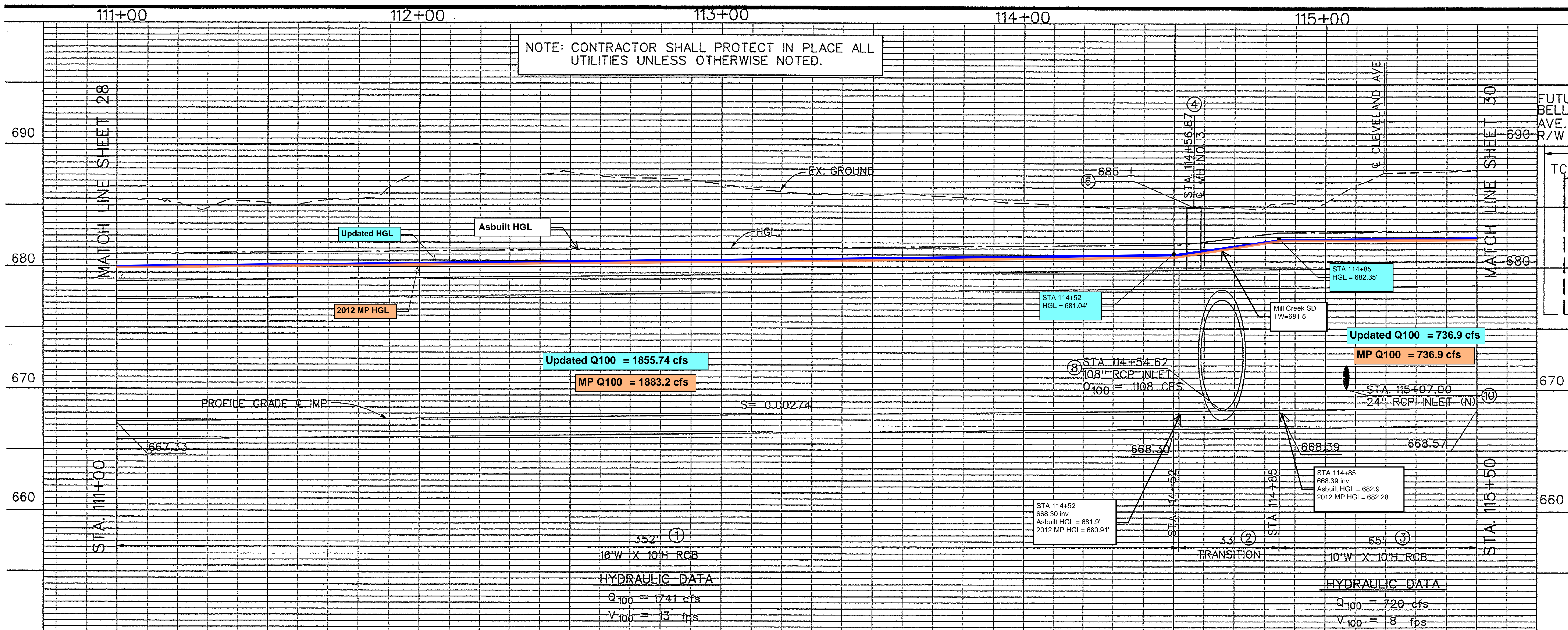
NOTES

- ① CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
- ② CONSTRUCT MANHOLE NO. 3 PER STD. MH253, MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
- ③ CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
- ④ INSTALL 48" RCP CONNECTOR PER DETAILS ON SHEET 70.
- ⑤ REMOVE AND REINSTALL EXISTING FENCE AS REQUIRED.
- ⑥ MAINTAIN THE OPERATION OF THE EXISTING DAIRY WASTE WATER DISTRIBUTION SYSTEM DURING CONSTRUCTION. RELOCATE AND REPLACE THE WATER LINE AS REQUIRED.

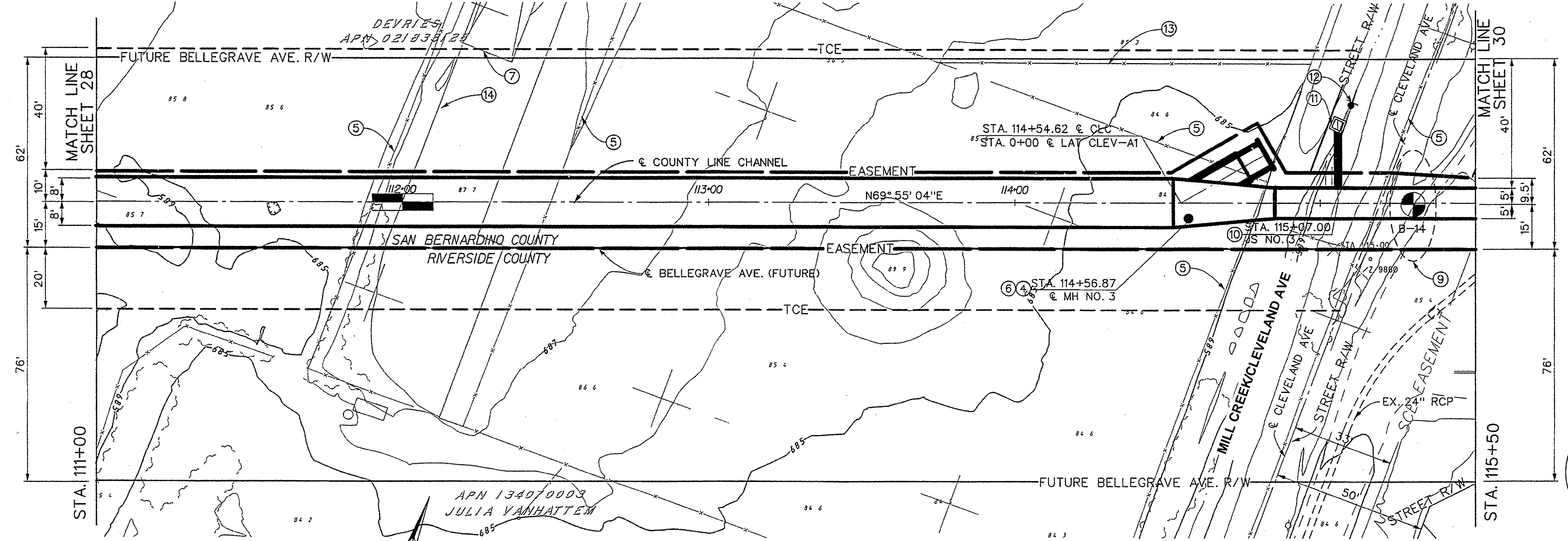
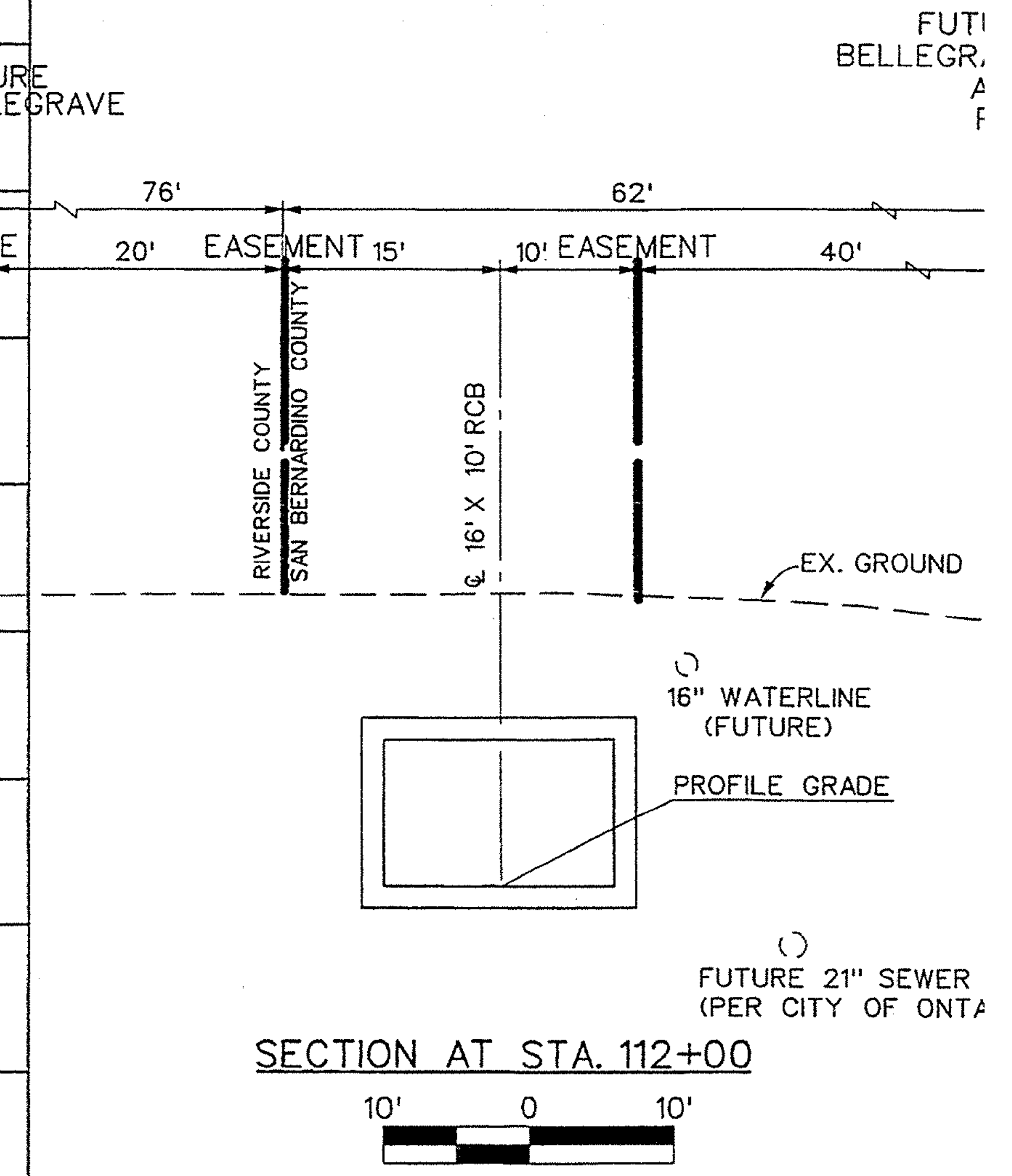


#1-317-1

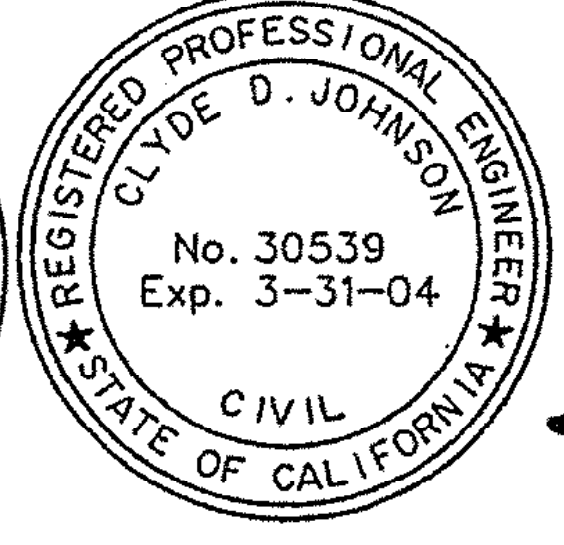
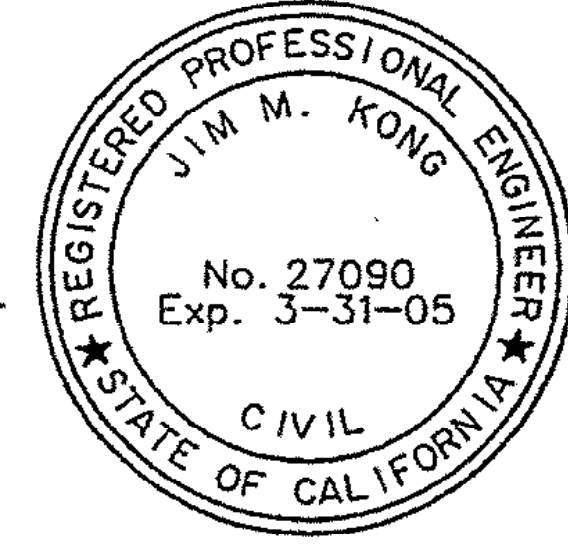
BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-	
				DESIGNED BY: G.H. / A. MARTINEZ		APPROVED BY: [Signature]		DRAWING NO. 2-	
				DRAWN BY: M. HOWARD		RECOMMENDED FOR APPROVAL BY: [Signature]		SHEET NO. 28 OF	
				DATE DRAWN: DEC. 2003		DATE: 12/30/03 R.E. No. 27090		STA. 107+00 TO STA. 111+00	
				CHECKED BY:		DATE: 12/30/03 R.E. No. 30539			



NOTE: CONTRACTOR SHALL PROTECT IN PLACE ALL UTILITIES UNLESS OTHERWISE NOTED.



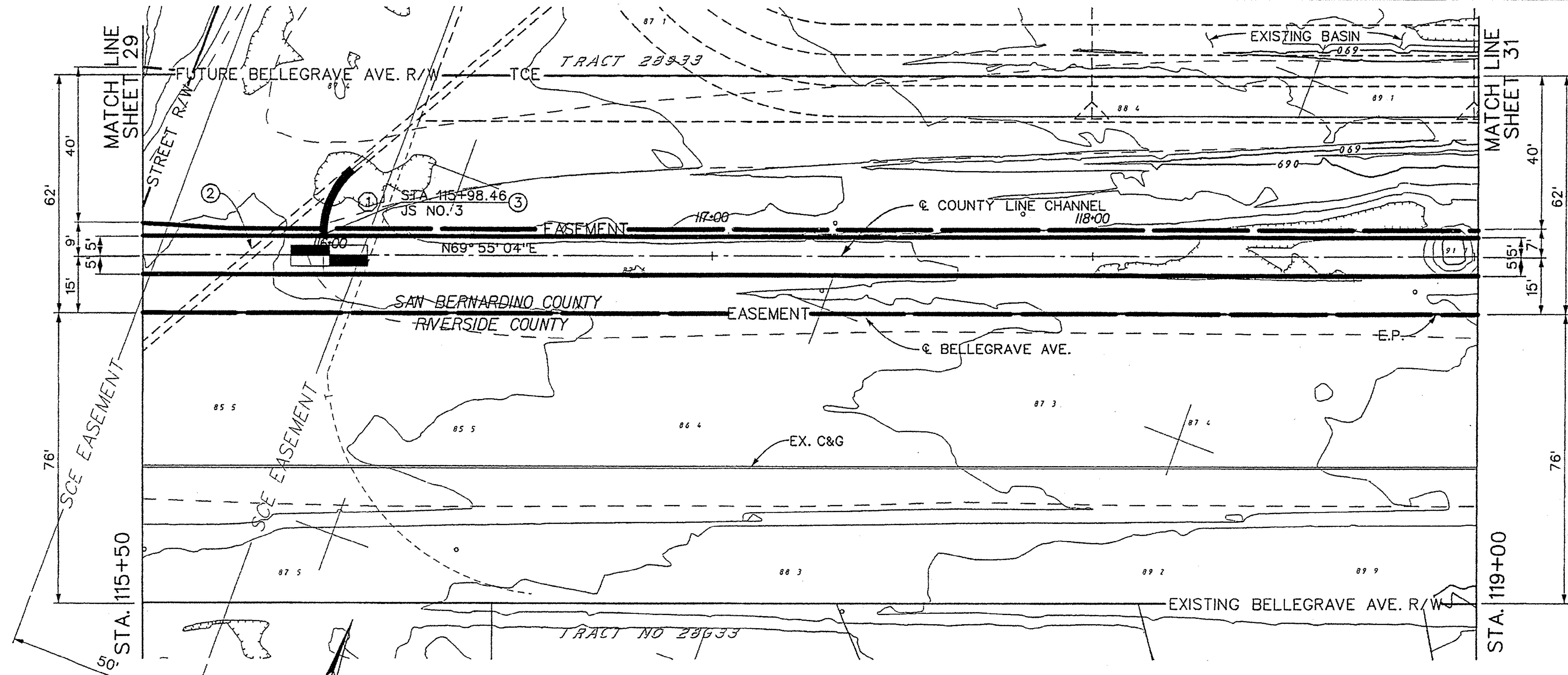
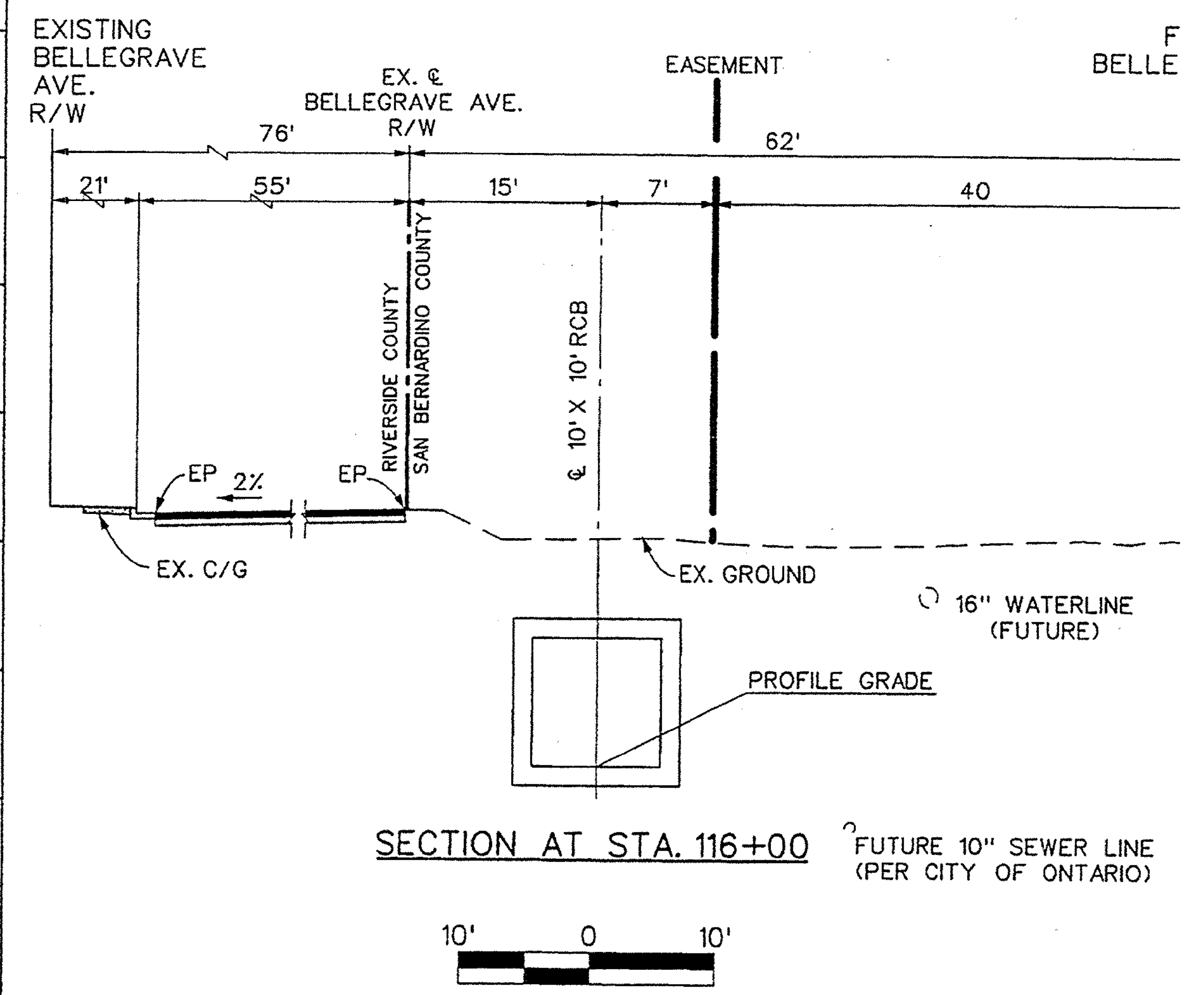
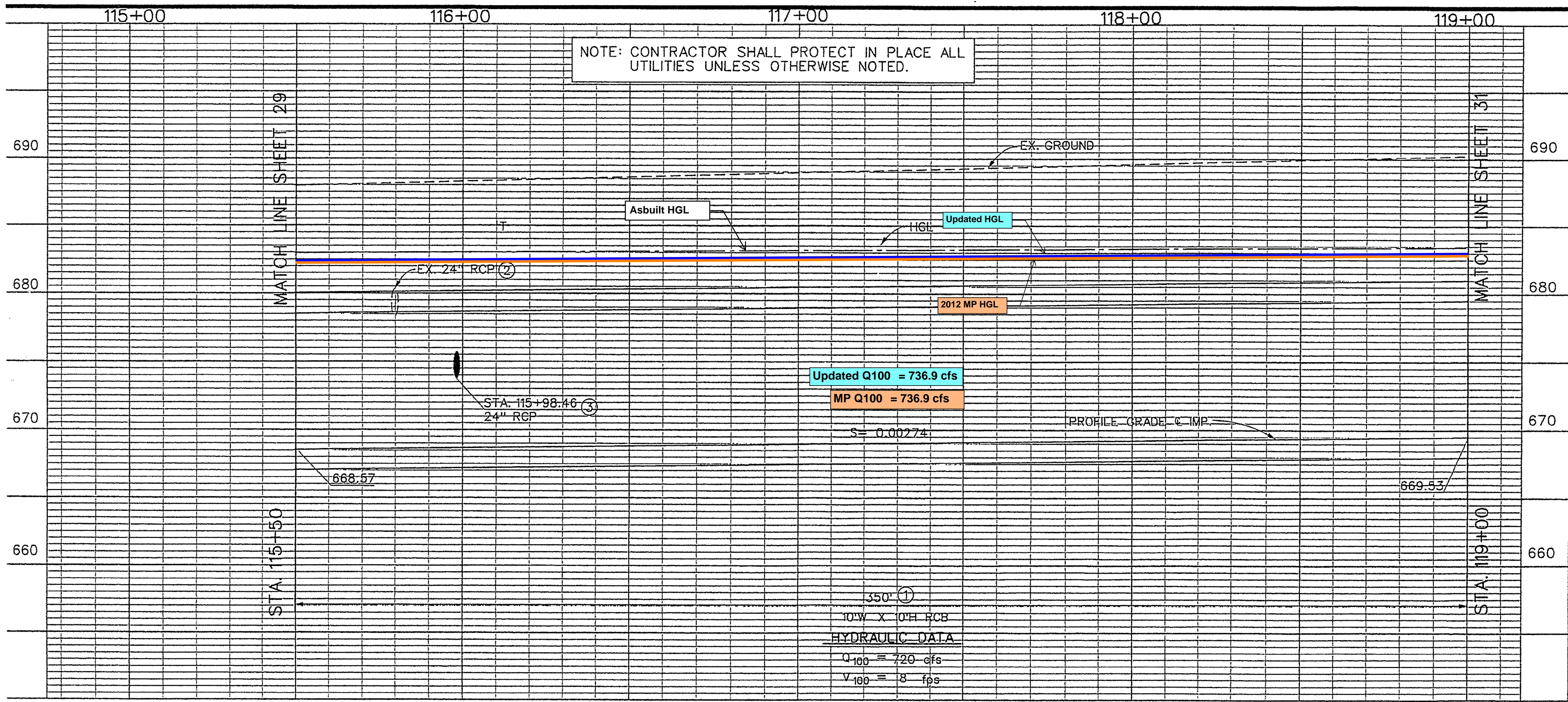
- NOTES**
- ① CONSTRUCT 16' X 10' RCB PER DETAILS ON SHEET 40.
 - ② CONSTRUCT TRANSITION STRUCTURE NO. 2 PER STD. TS302.
 - ③ CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
 - ④ CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO PLAC REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AI COVER PER SBCFC STD. 205.
 - ⑤ REMOVE AND REINSTALL EXISTING FENCE AS REQUIRED.
 - ⑥ CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
 - ⑦ PROTECT EXISTING COW SHED.
 - ⑧ CONSTRUCT JS NO. 1 PER STD. JS226. A = 30°, B = 108", C = 23. ELEV. R = 668.85, ELEV. S = 668.7. SEE PROFILE ON SHEET 63.
 - ⑨ CAUTION, LOW HANGING WIRES.
 - ⑩ CONSTRUCT JS NO. 3 PER STD. JS228.
 - ⑪ CONSTRUCT CONCRETE DROP INLET WITH NORTH AND WEST SIDES OPEN PER STD. CB110, V = 4.0'. SEE PROFILE ON SHEET 71.
 - ⑫ INSTALL TYPE L-1 MARKER PER CALTRANS STD. A73A.
 - ⑬ INSTALL 5' TEMPORARY METAL FENCE PER DETAIL ON SHEET 72.
 - ⑭ REMOVE EXISTING COW FEEDER AS REQUIRED.



#1-317-1

BENCH MARK	REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		PROJECT NO. 2-0-0
	DESIGNED BY: G.H./A.MARTINEZ	RECOMMENDED FOR APPROVAL BY:	APPROVED BY:	DRAWING NO. 2-31	
	DRAWN BY: M. HOWARD	DATE: 12/30/03	DATE: 12/30/03	R.E. No. 27090	SHEET NO. 29

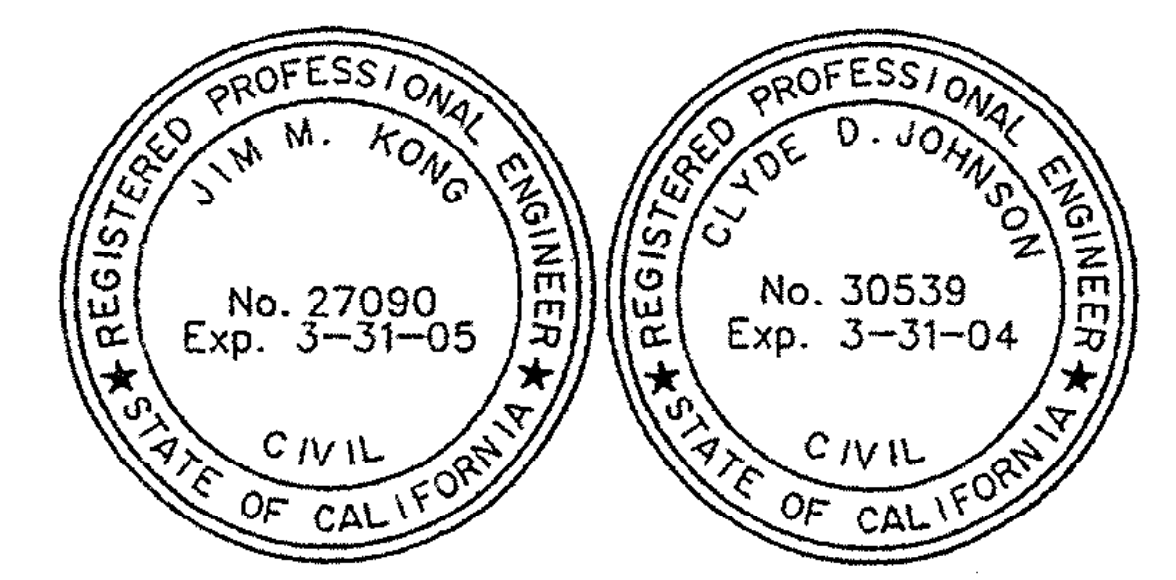
COUNTY LINE CHANNEL
STA. 111+00 TO STA. 115+50



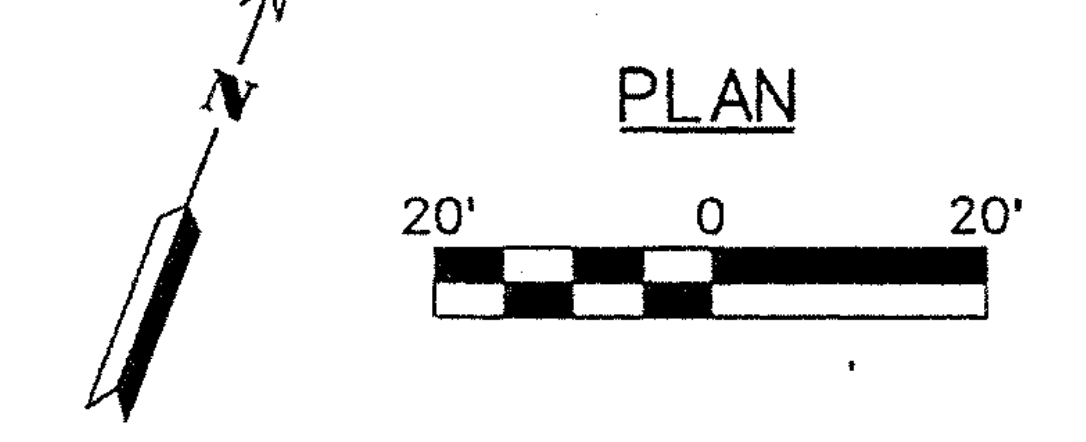
NOTES

- ① CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
- ② REMOVE INTERFERING PORTION OF EXISTING 24" RCP AND PLUG.
- ③ CONSTRUCT JS NO. 3 PER STD. JS228. SEE DETAILS ON SHEET 63.

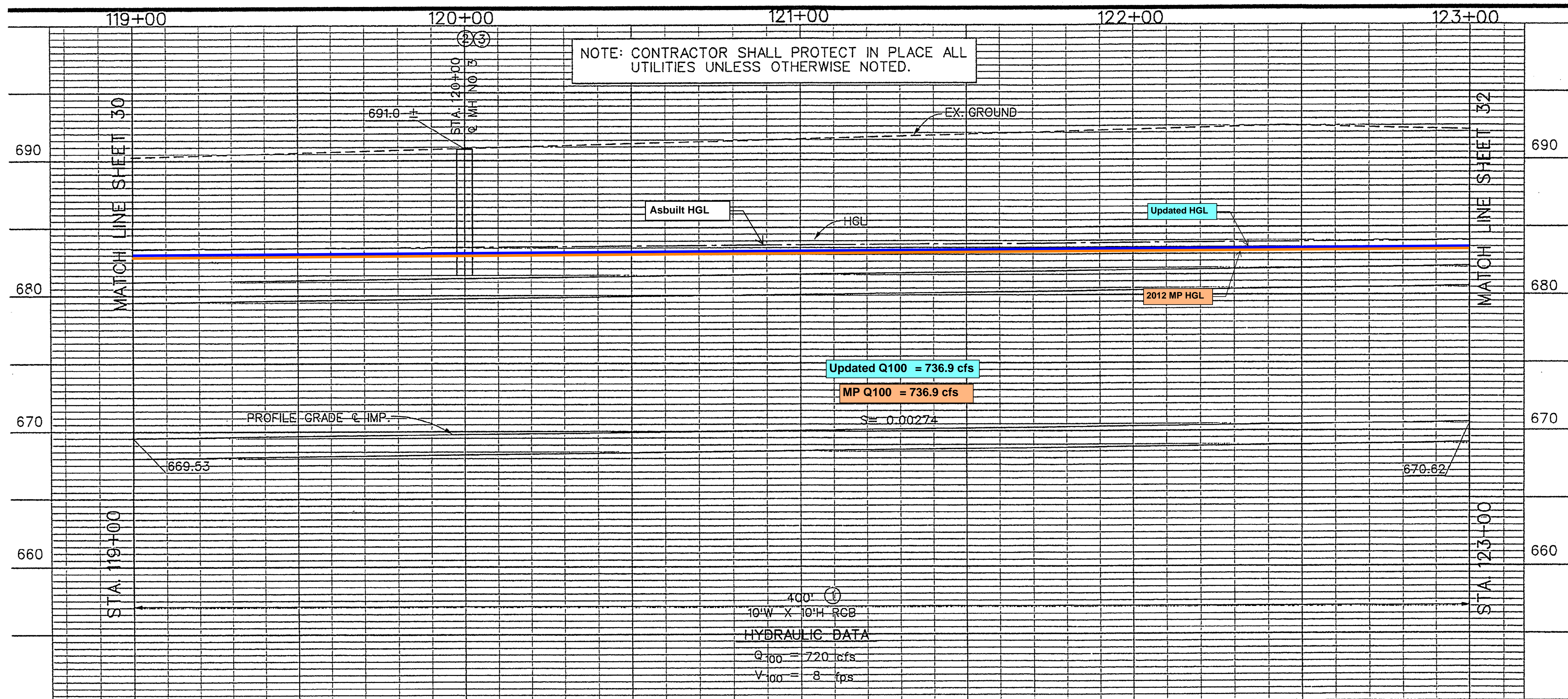
① **24" RCP CURVE DATA**
 $\Delta = 49^\circ 48' 23.29''$
 $R = 22.5'$
 $L = 19.56'$
 $T = 10.45'$
 $BC = STA. 0+10.00$
 $EC = STA. 0+29.56$
 $PI = N 2304882.260$
 $E 6162467.118$



#1-317-1

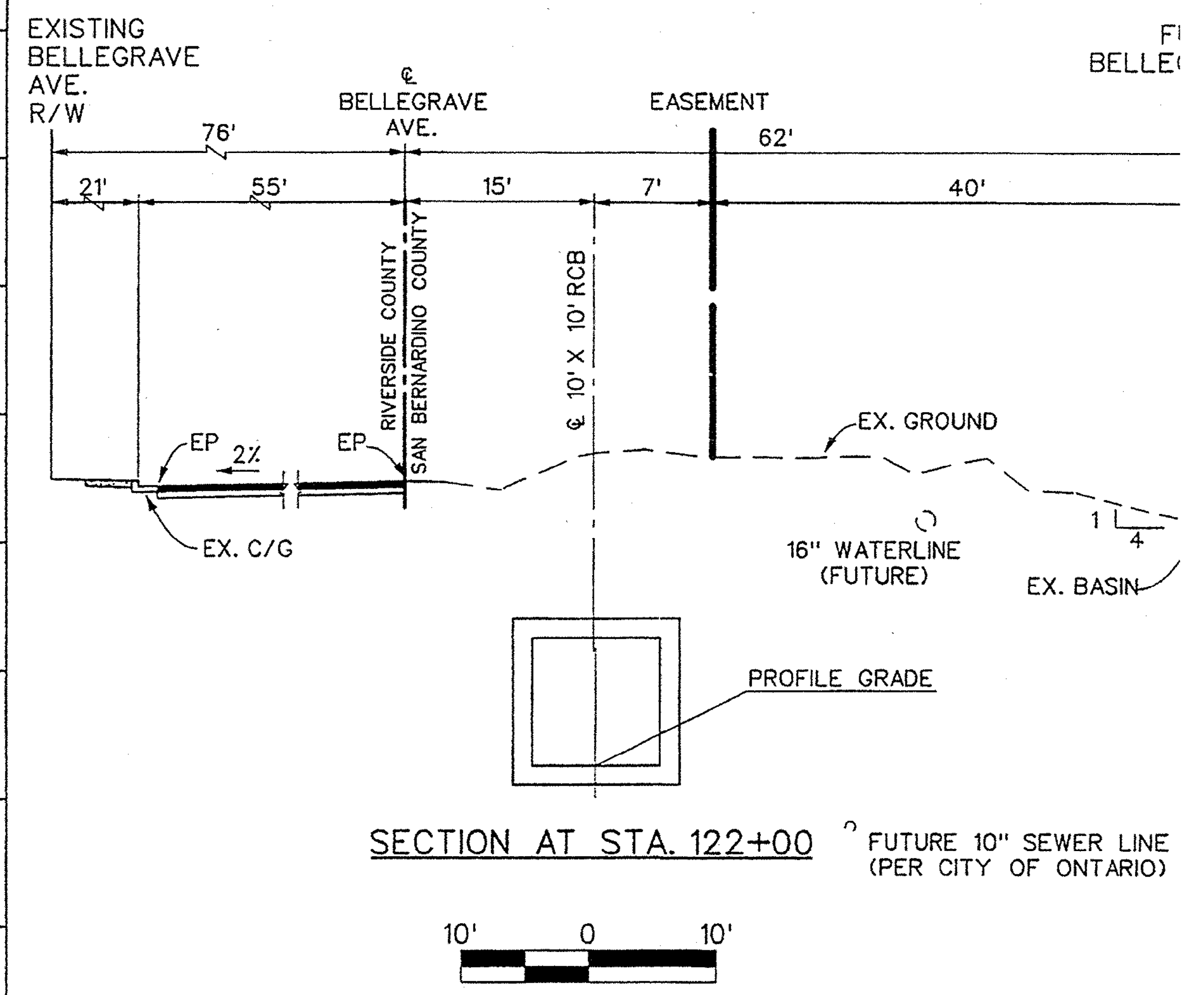


BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-03
				DESIGNED BY: G.H./A.MARTINEZ		APPROVED BY:		DRAWING NO. 2-31E
				DRAWN BY: M. HOWARD		DATE: 12/2/03 R.E. No. 27090		SHEET NO. 30 OF 1
				DATE DRAWN: DEC. 2003		DATE: 12/2/03 R.E. No. 30539		
REF.	DESCRIPTION	APPR	DATE	CHECKED BY:				

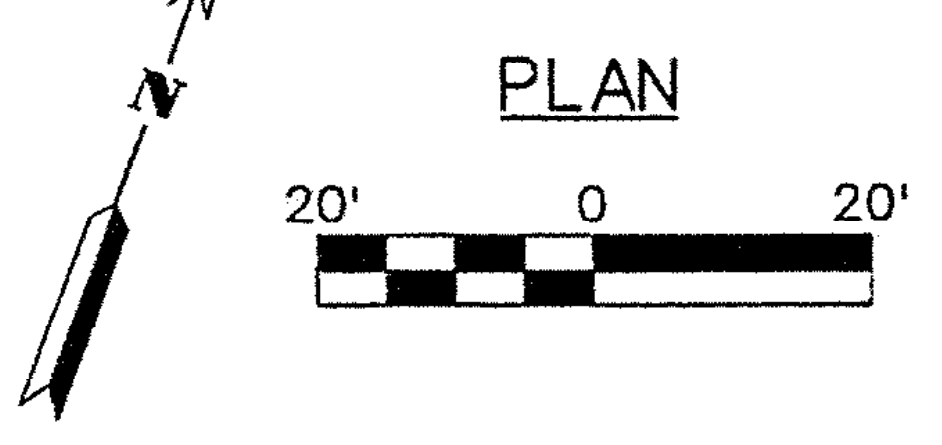
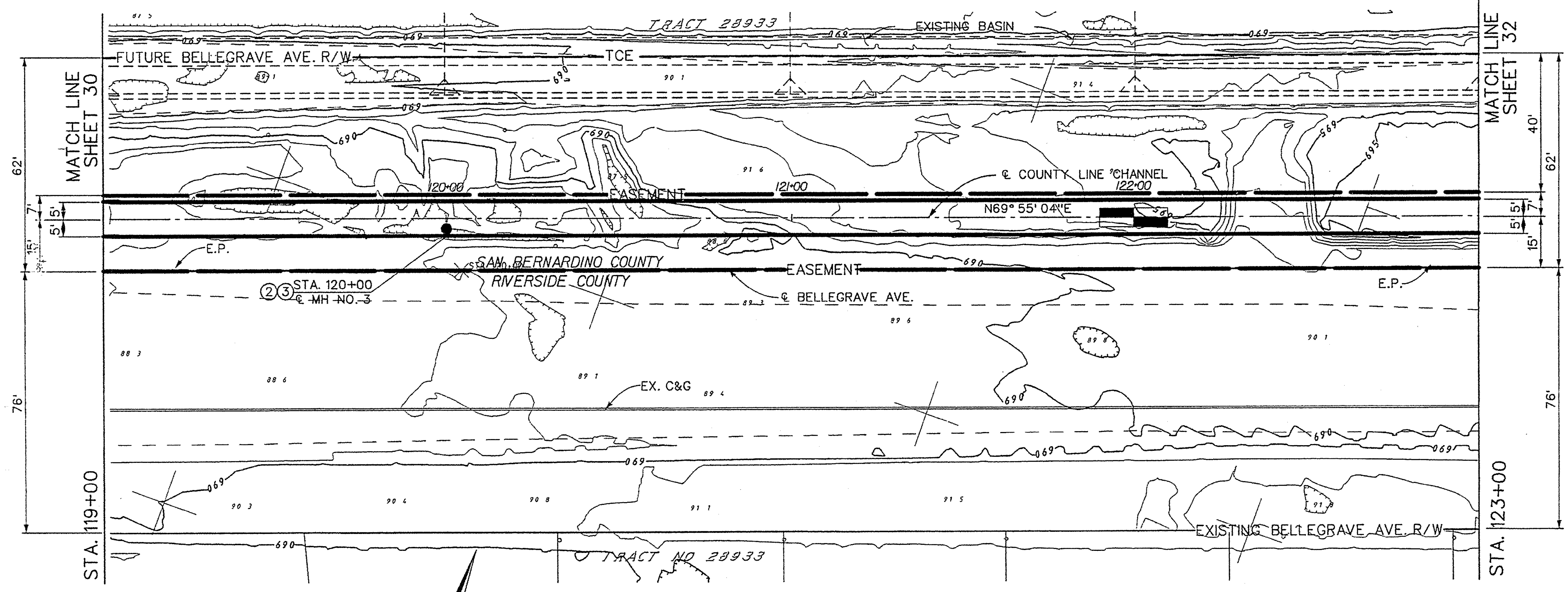


400' ϕ
 10'W X 10'H RCB
 HYDRAULIC DATA
 Q₁₀₀ = 720 cfs
 V₁₀₀ = 8 fps

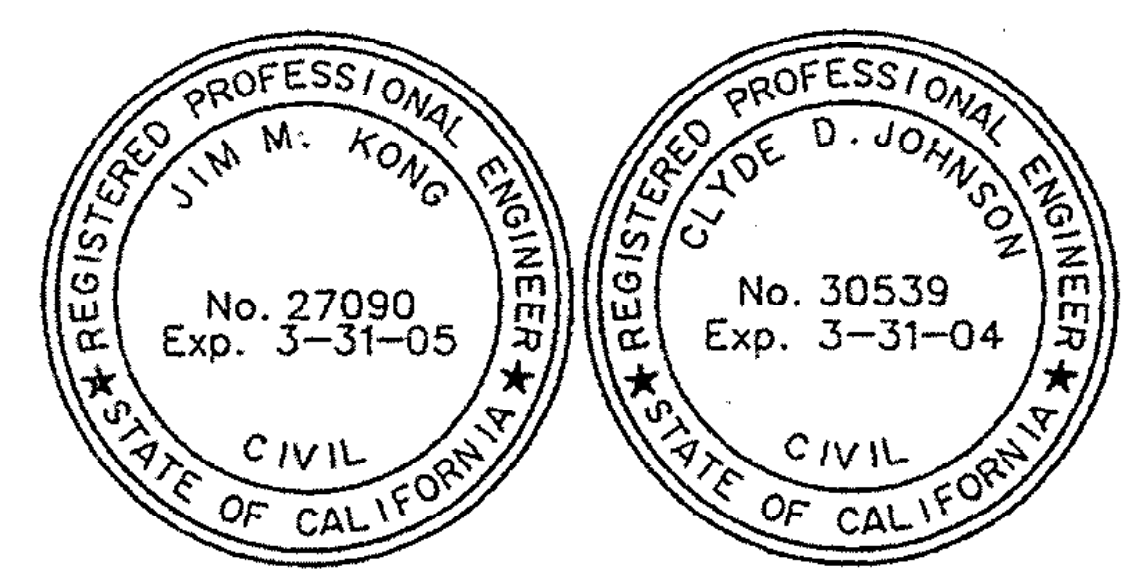
Updated Q₁₀₀ = 736.9 cfs
 MP Q₁₀₀ = 736.9 cfs
 S = 0.00274



SECTION AT STA. 122+00 FUTURE 10" SEWER LINE (PER CITY OF ONTARIO)

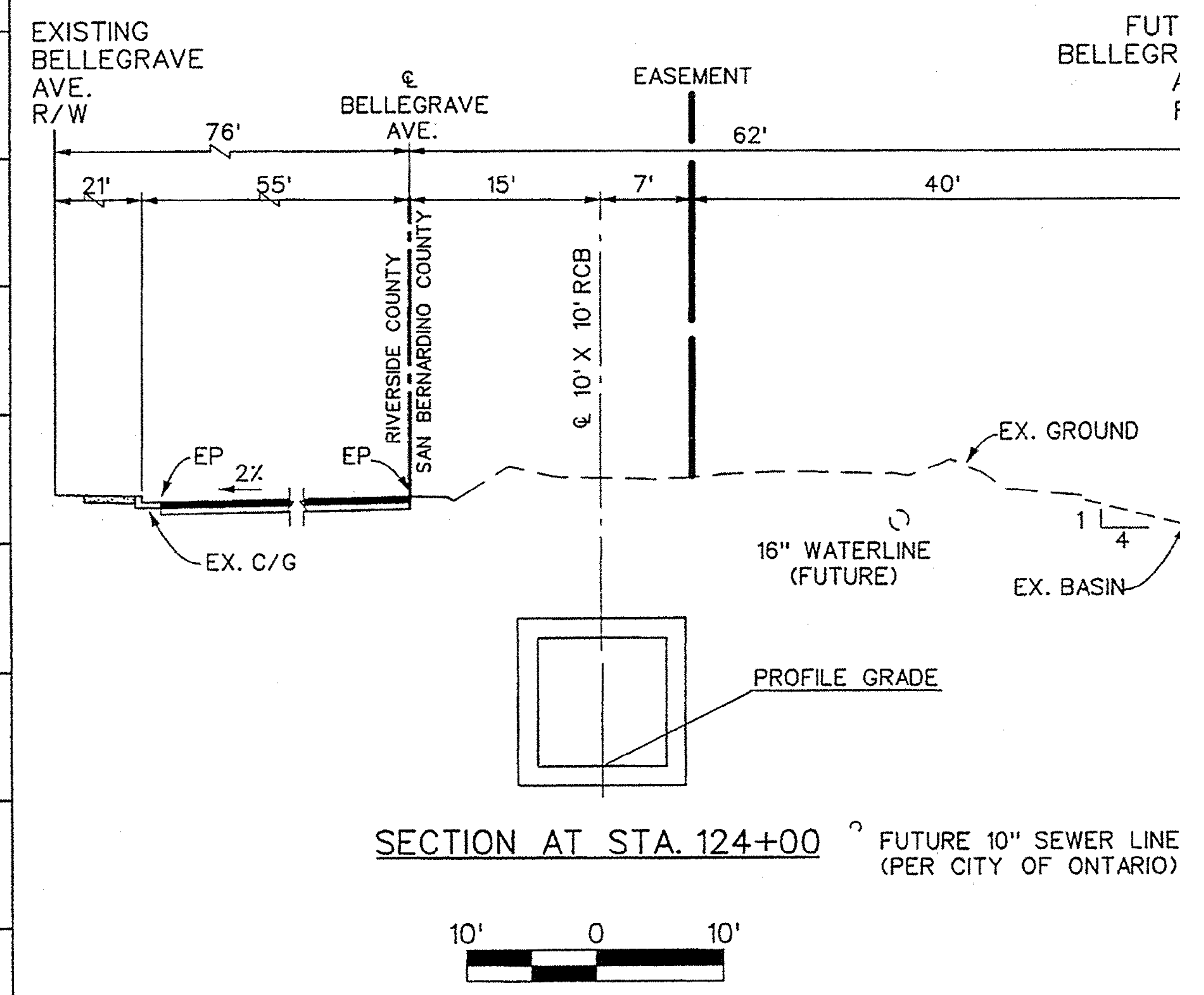
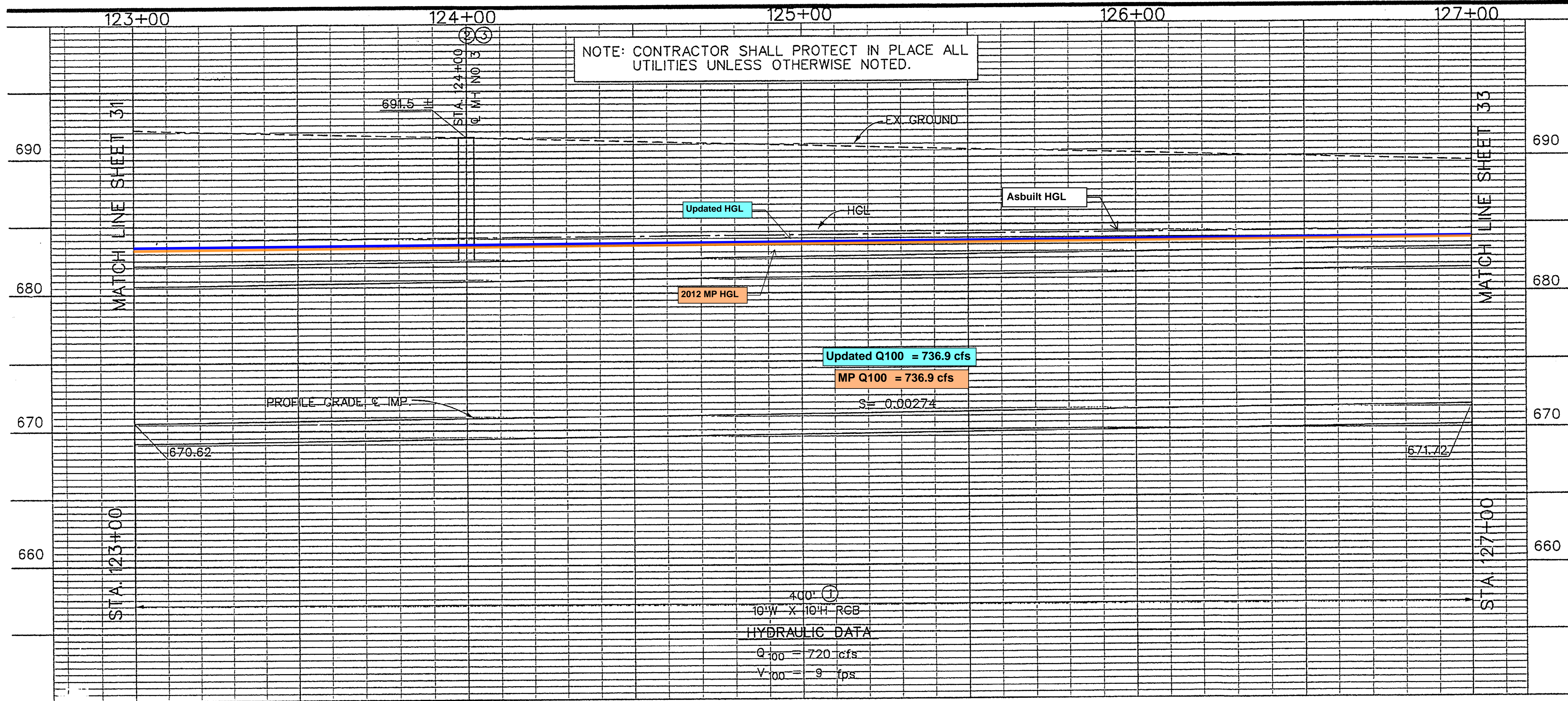


- NOTES**
- CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
 - CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.



#1-317-1

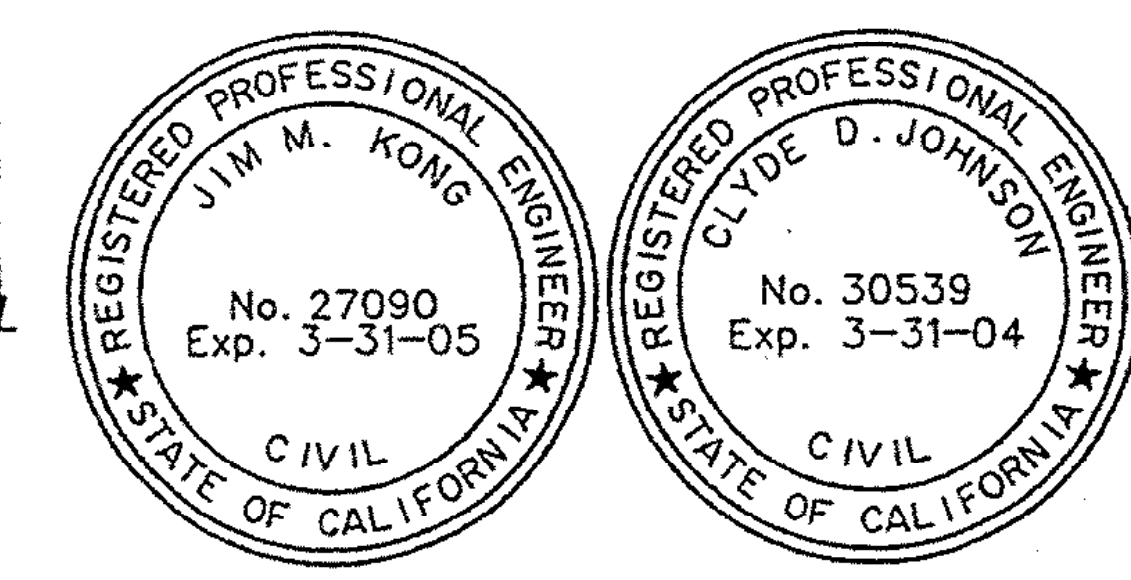
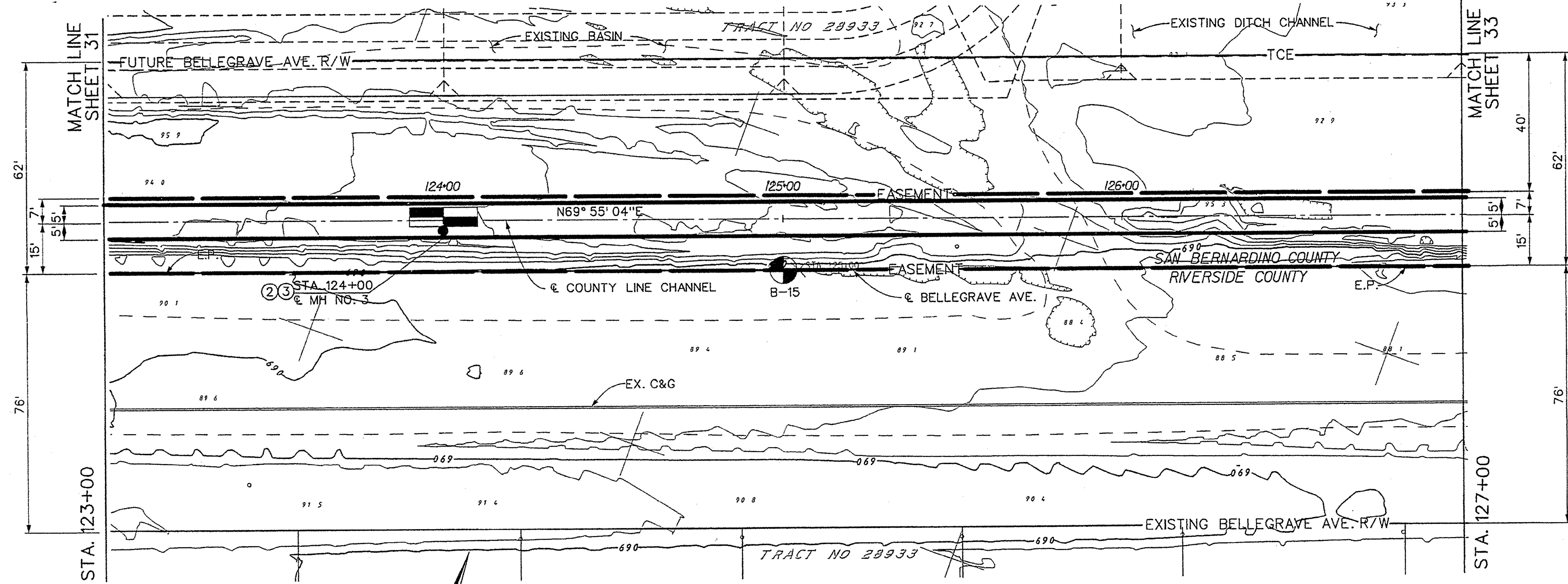
BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-0
				DESIGNED BY: G.H./A.MARTINEZ		APPROVED BY:		DRAWING NO. 2-31
				DRAWN BY: M. HOWARD		RECOMMENDED FOR APPROVAL BY:		SHEET NO. 31 OF
				DATE DRAWN: DEC. 2003		DATE: 12/20/03 R.E. No. 27090		
				DATE: 12/20/03 R.E. No. 30539		DATE: 12/20/03 R.E. No. 30539		
				CHECKED BY:		STA. 119+00 TO STA. 123+00		



400' ϕ
10' X 10' RCB
HYDRAULIC DATA
Q₁₀₀ = 720 cfs
V₁₀₀ = 9 fps

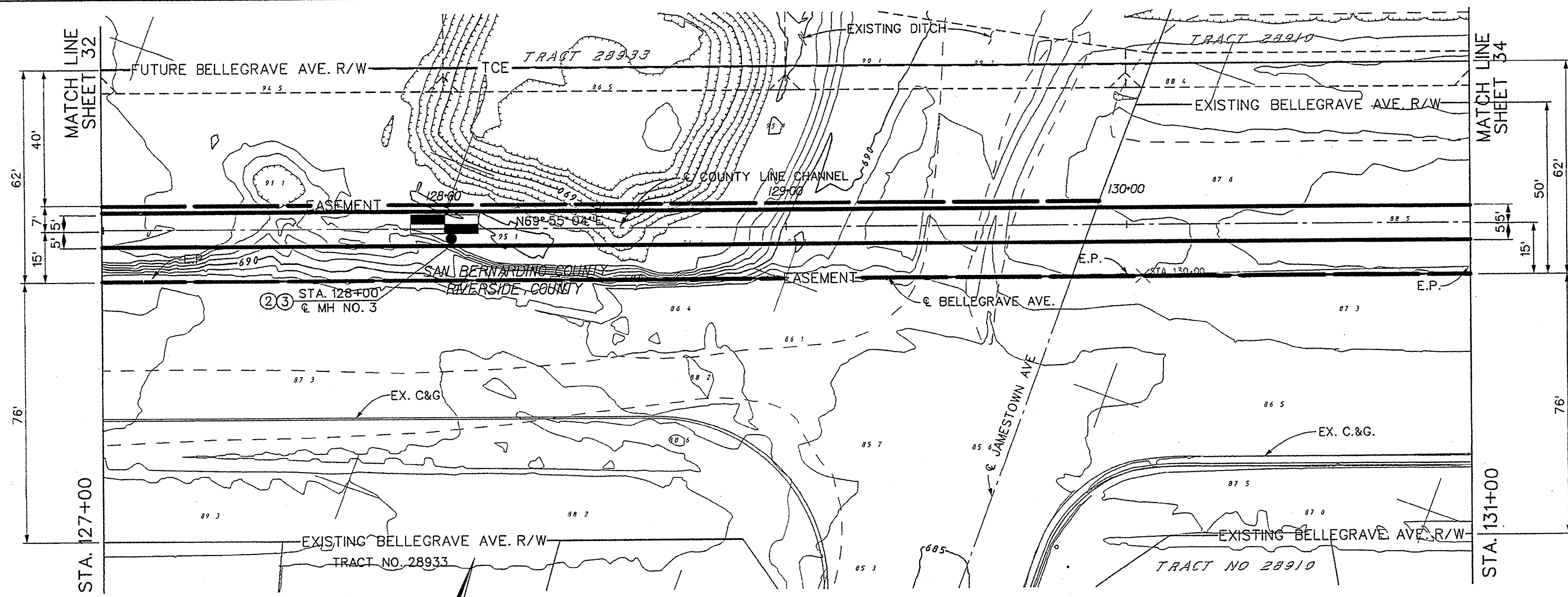
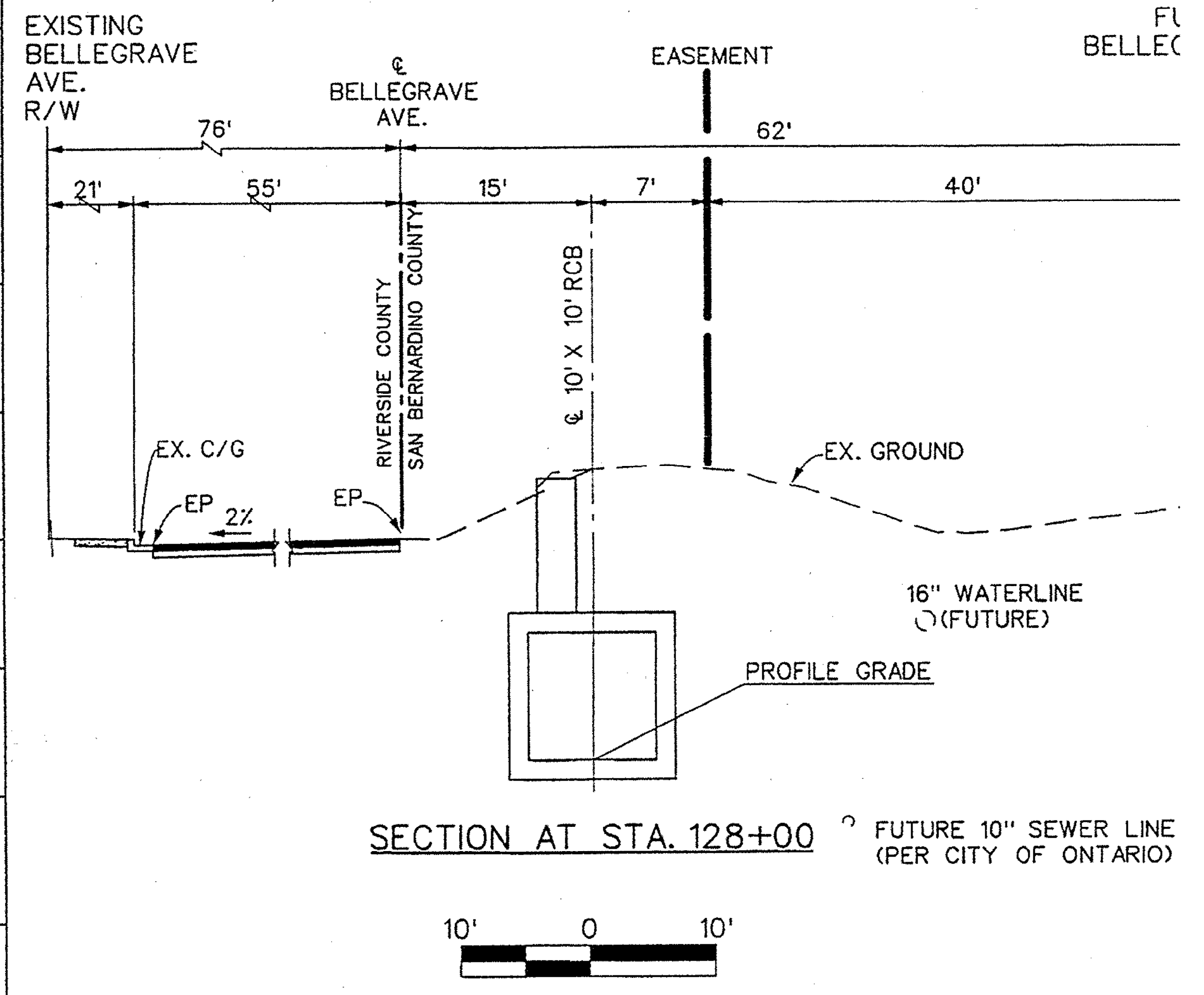
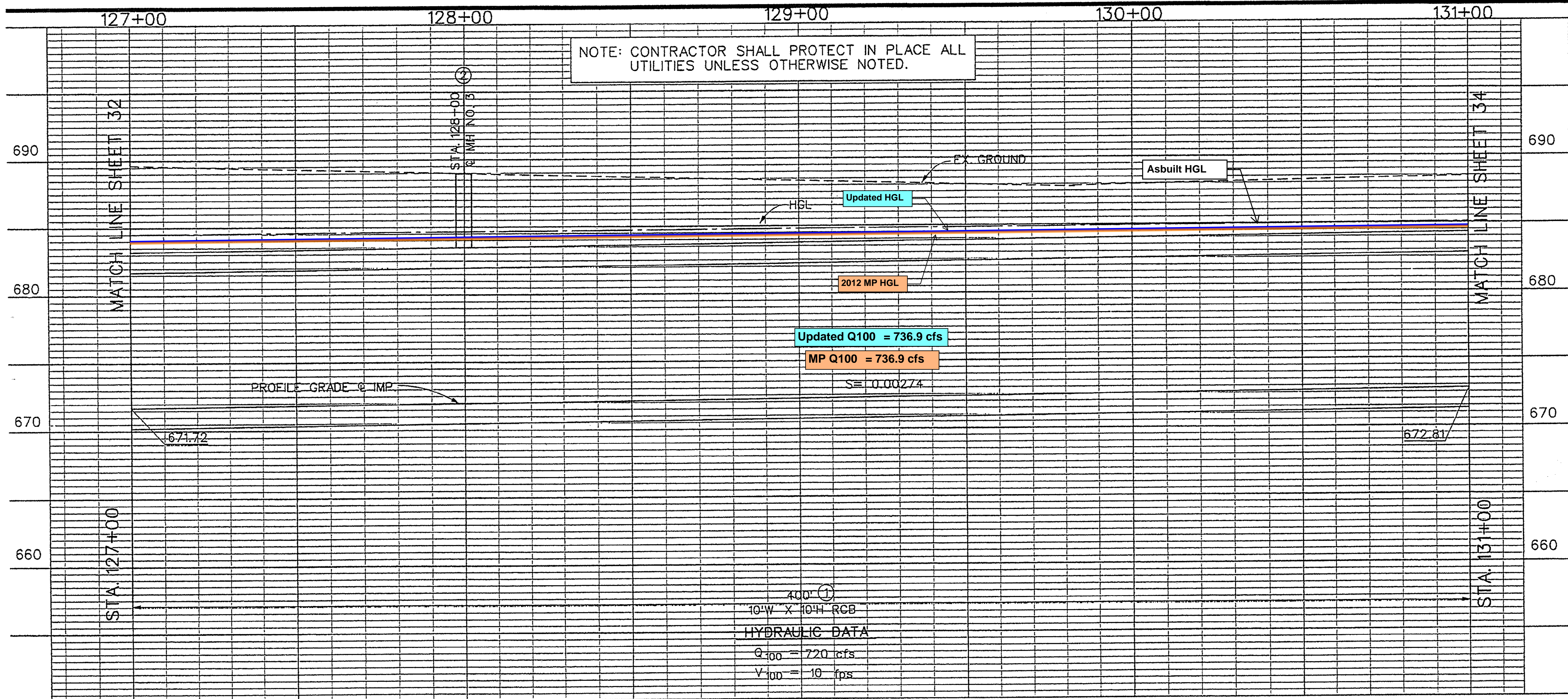
NOTES

- ① CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
- ② CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
- ③ CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.

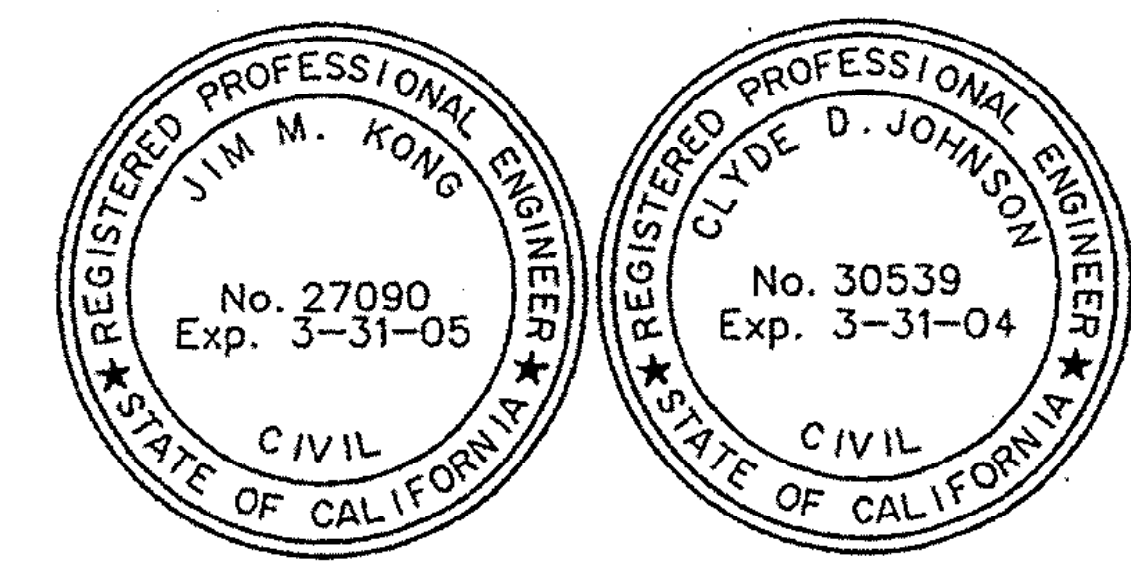


#1-317-1

BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-
REF.	DESCRIPTION	APPR.	DATE	DESIGNED BY: G.H./A.MARTINEZ	RECOMMENDED FOR APPROVAL BY: [Signature]	APPROVED BY: [Signature]	STA. 123+00 TO STA. 127+00	DRAWING NO. 2-3
				DRAWN BY: M. HOWARD	DATE: 12/20/03 R.E. No. 27090	DATE: 12/16/03 R.E. No. 30539		SHEET NO. 32 OF

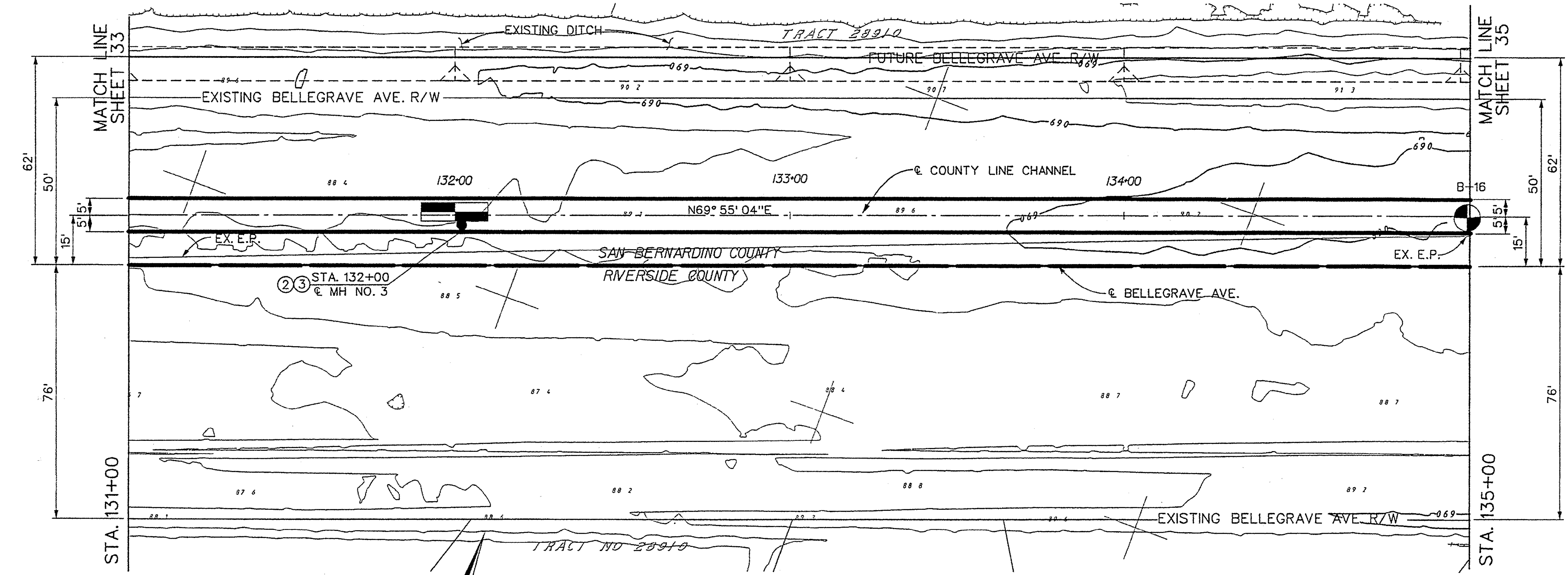
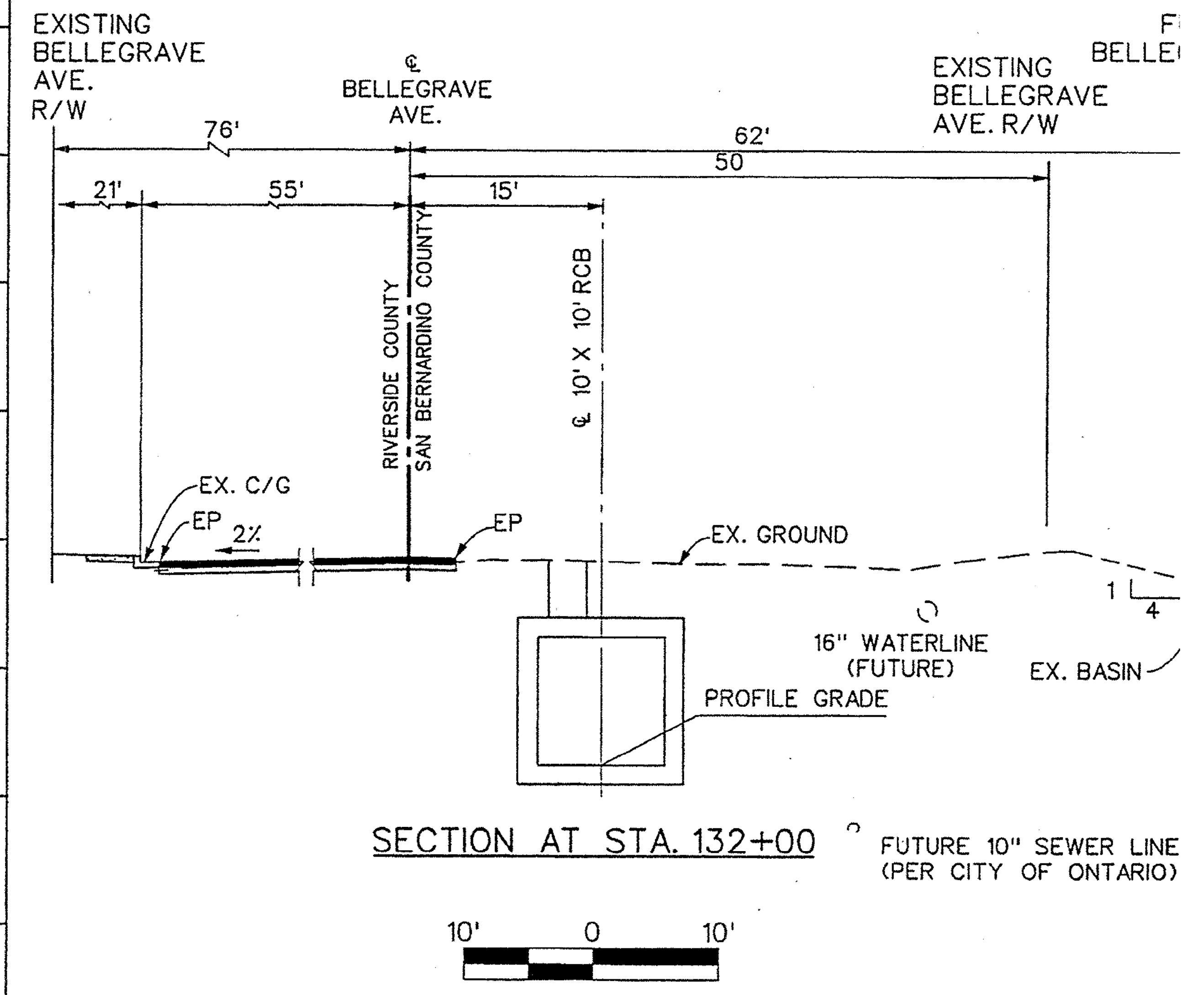
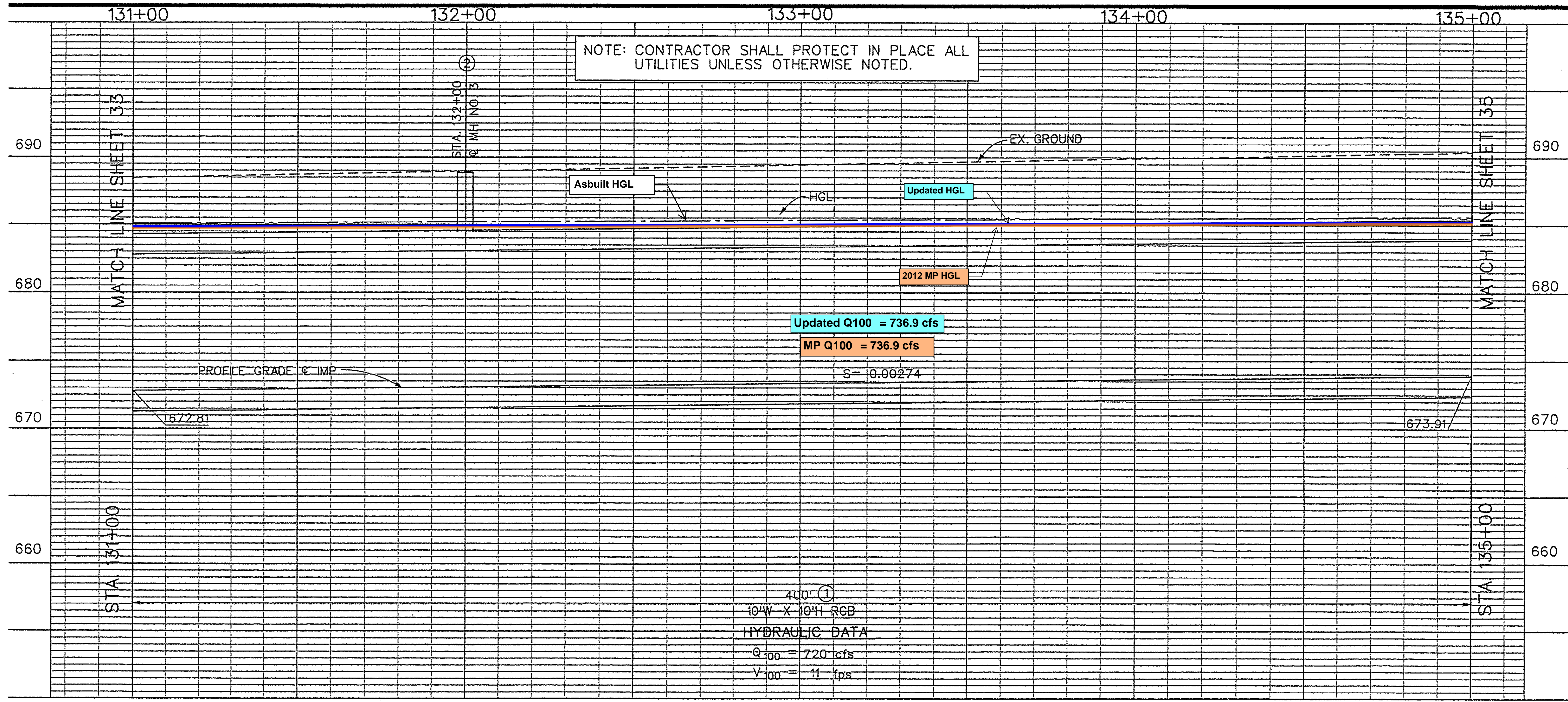


- NOTES**
- CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
 - CONSTRUCT MANHOLE NO. 3 PER STD. MH253. MODIFIED TO REPLACE THE REDUCER WITH 3' DIAMETER SHAFT AND WITH MANHOLE FRAME AND COVER PER SBCFC STD. 205.
 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.



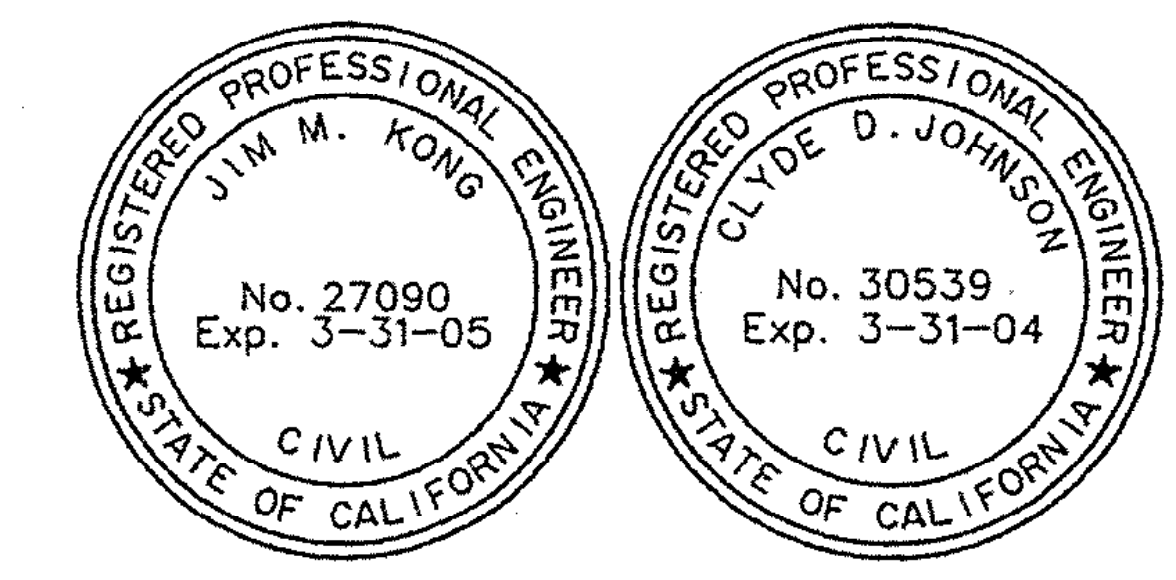
#1-317-1

BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-0
REF.	DESCRIPTION	APPR.	DATE	DESIGNED BY: G.H./A.MARTINEZ	RECOMMENDED FOR APPROVAL BY:	APPROVED BY:	STA. 127+00 TO STA. 131+00	DRAWING NO. 2-3
				DRAWN BY: M. HOWARD	DATE: 12/20/03 R.E. No. 27090	DATE: 12/24/03 R.E. No. 30539		SHEET NO. 33 OF

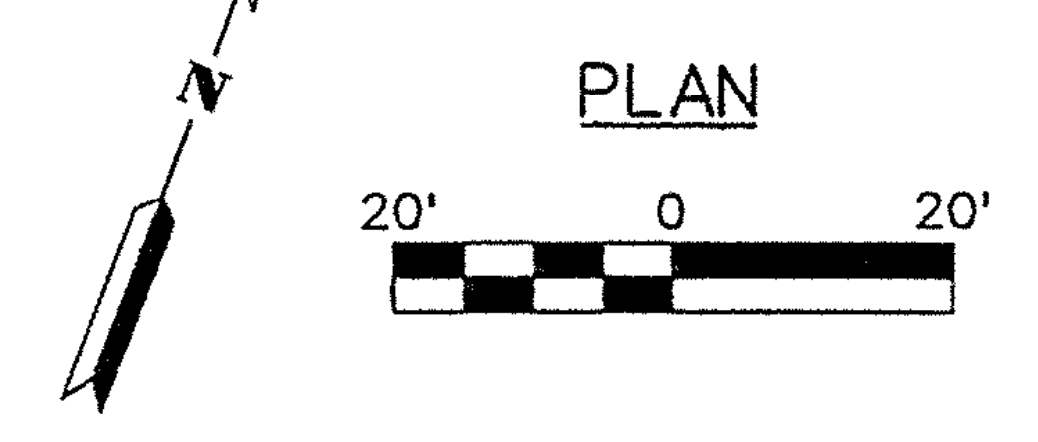


NOTES

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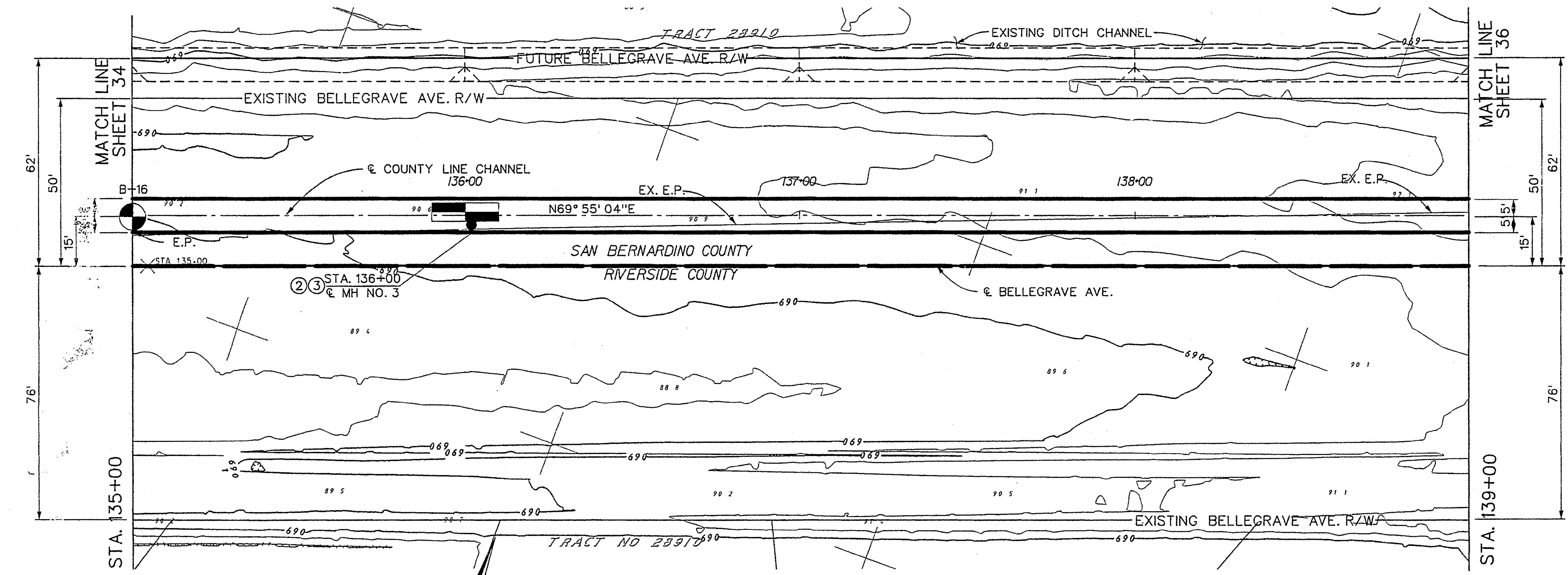
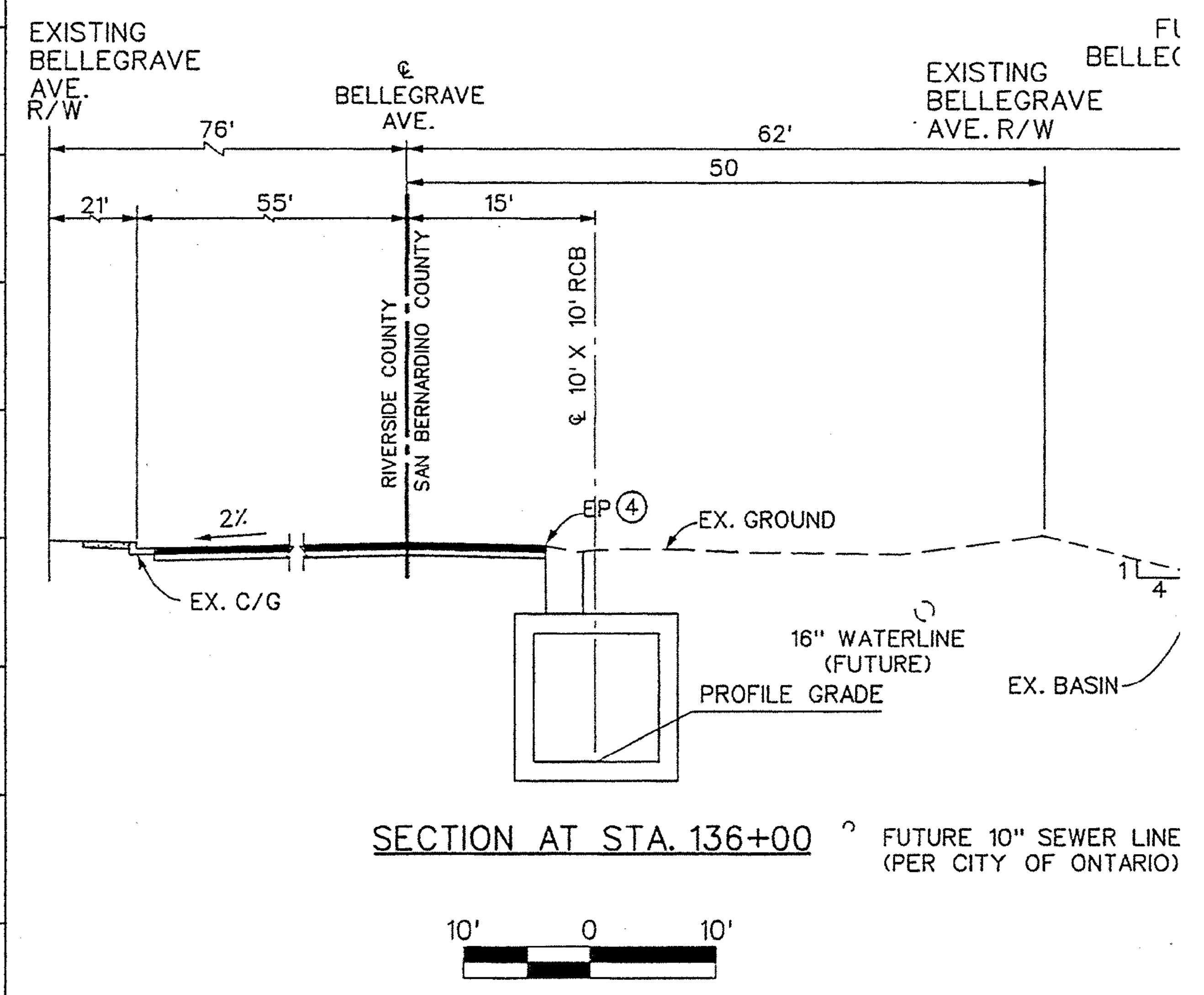
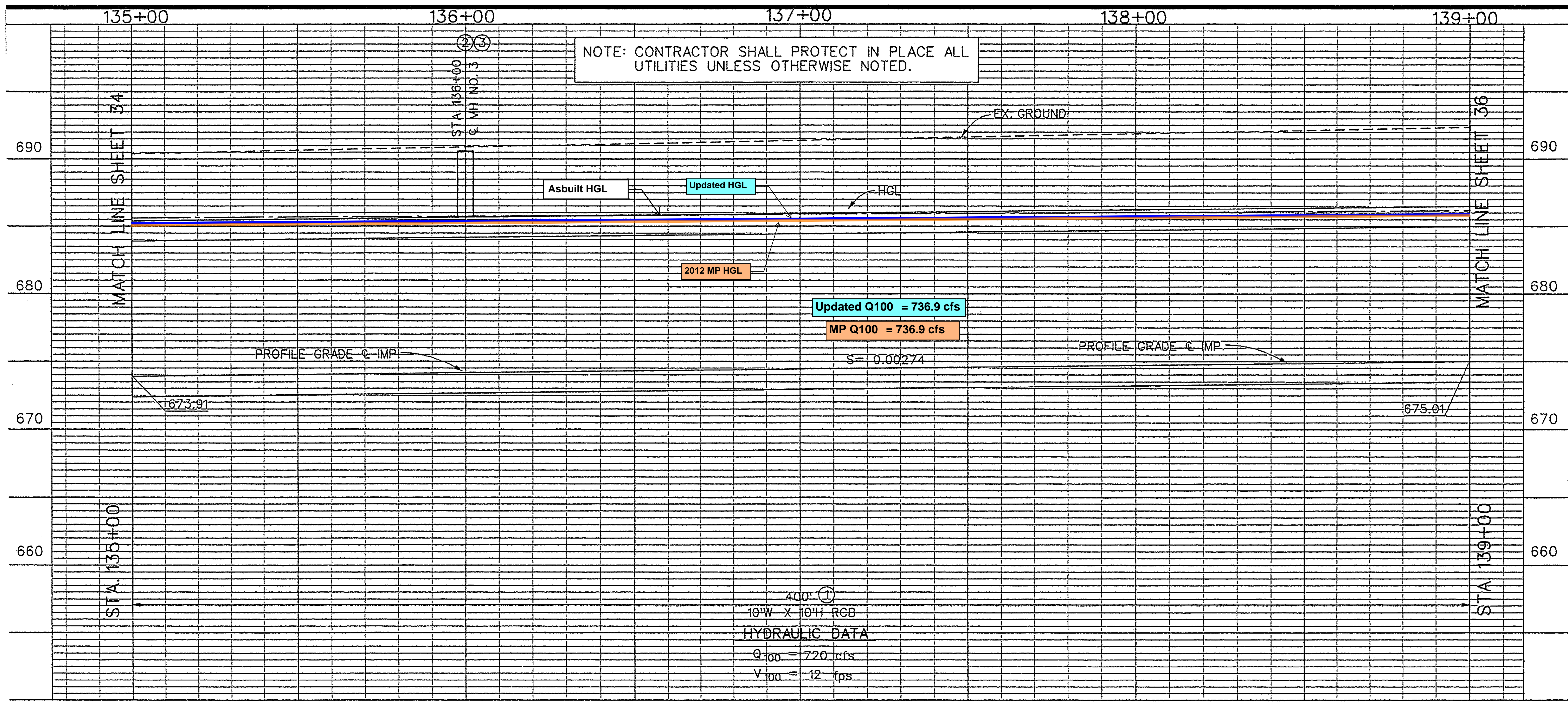
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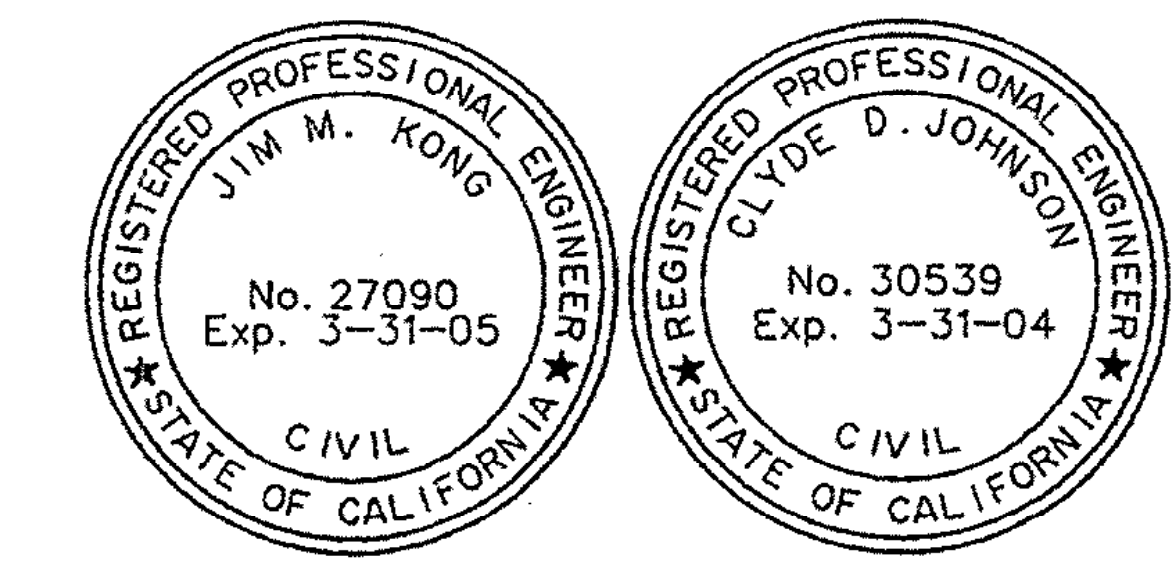
REF.	DESCRIPTION	APPR.	DATE	CHECKED BY:

DESIGNED BY: G.H./A.MARTINEZ		RECOMMENDED FOR APPROVAL BY:		APPROVED BY:	
DRAWN BY: M. HOWARD		DATE: 12/20/03 R.E. No. 27090		DATE: 12/20/03 R.E. No. 30539	

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-0	
STA. 131+00 TO STA. 135+00		DRAWING NO. 2-31		SHEET NO. 34 OF	

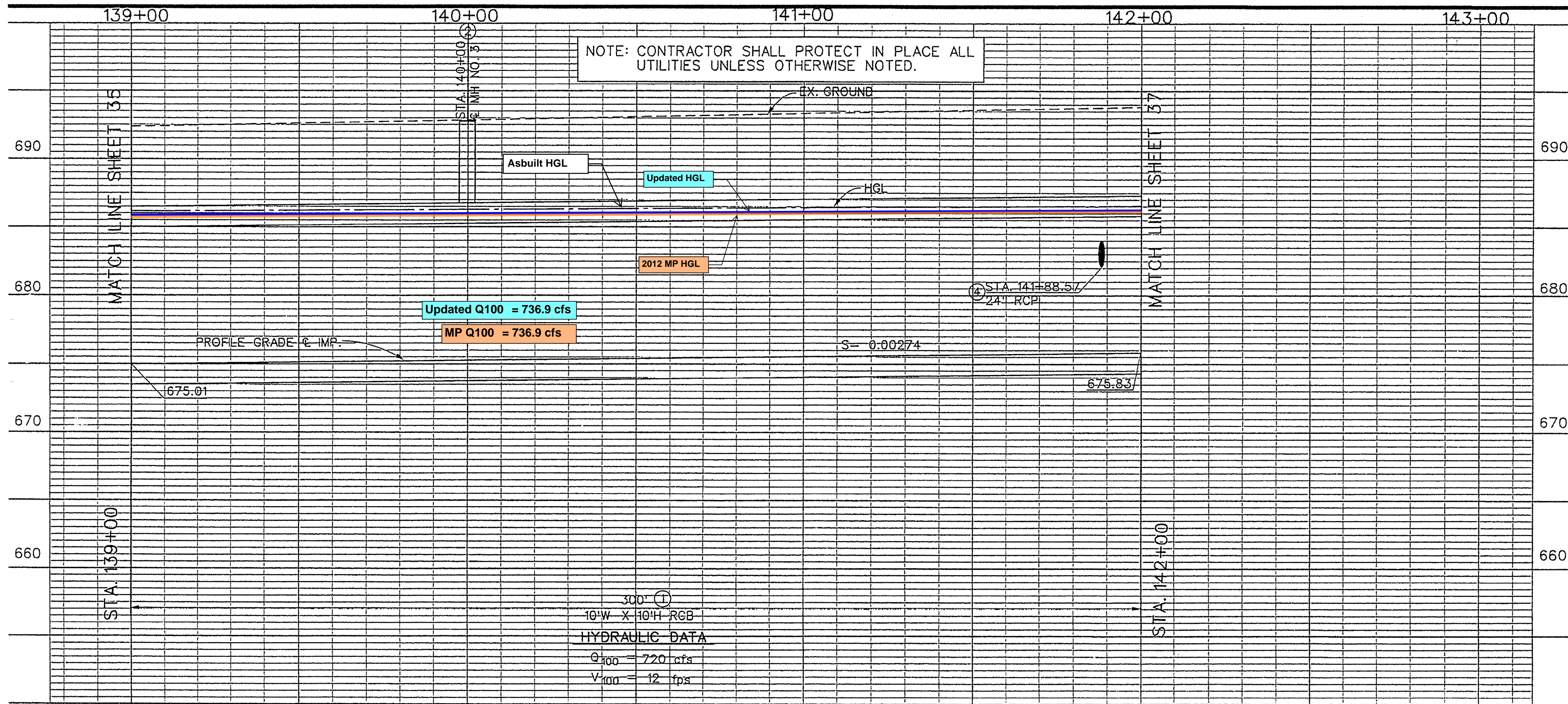


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 - CONSTRUCT MANHOLE APRON PER SBCFC STD. D260.
 - EXISTING EDGE OF PAVEMENT NORTH OF THE COUNTY LINE BETWEEN JAMESTOWN AVE. AND HAMNER AVE. VARIES FROM 0' TO 22'. PAVEMENT SHALL BE REPLACED WITH 0.43' TYPE 'B' AC AND 0.75' CLASS 2 AG. BASE WITHIN THE DISTRICT'S PAY LINES. ANY PAVEMENT REMOVED OUTSIDE THE PAY LIMITS SHALL BE REPLACED IN KIND BY THE CONTRACTOR.

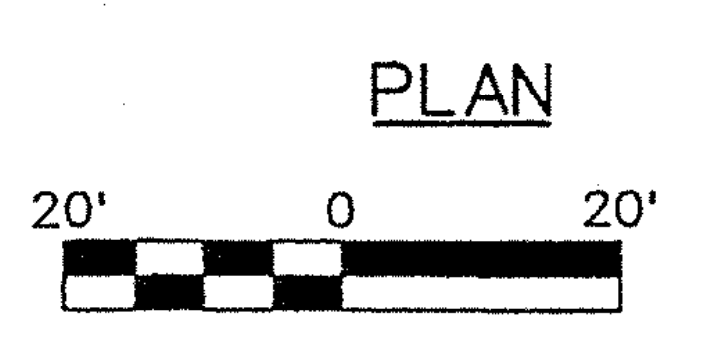
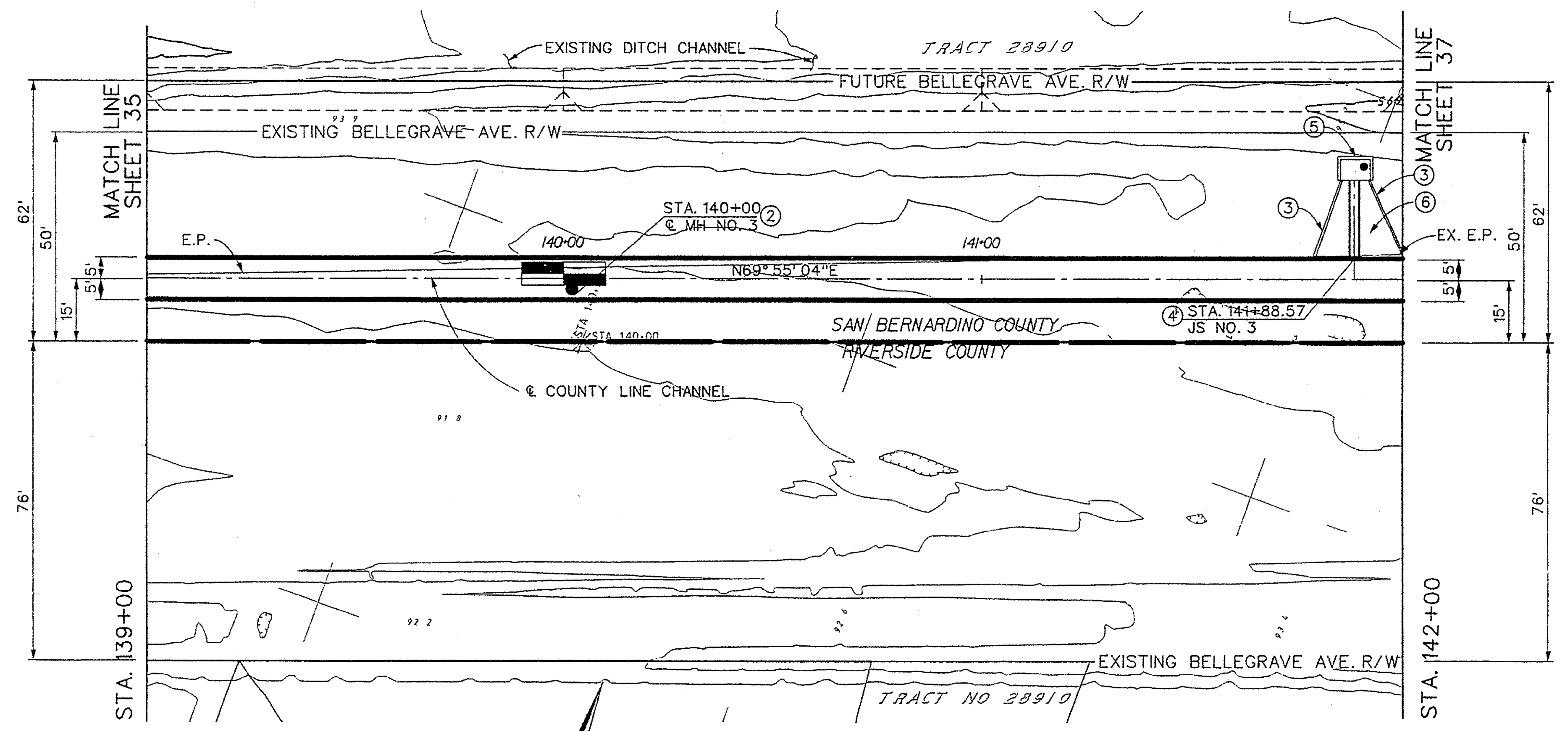
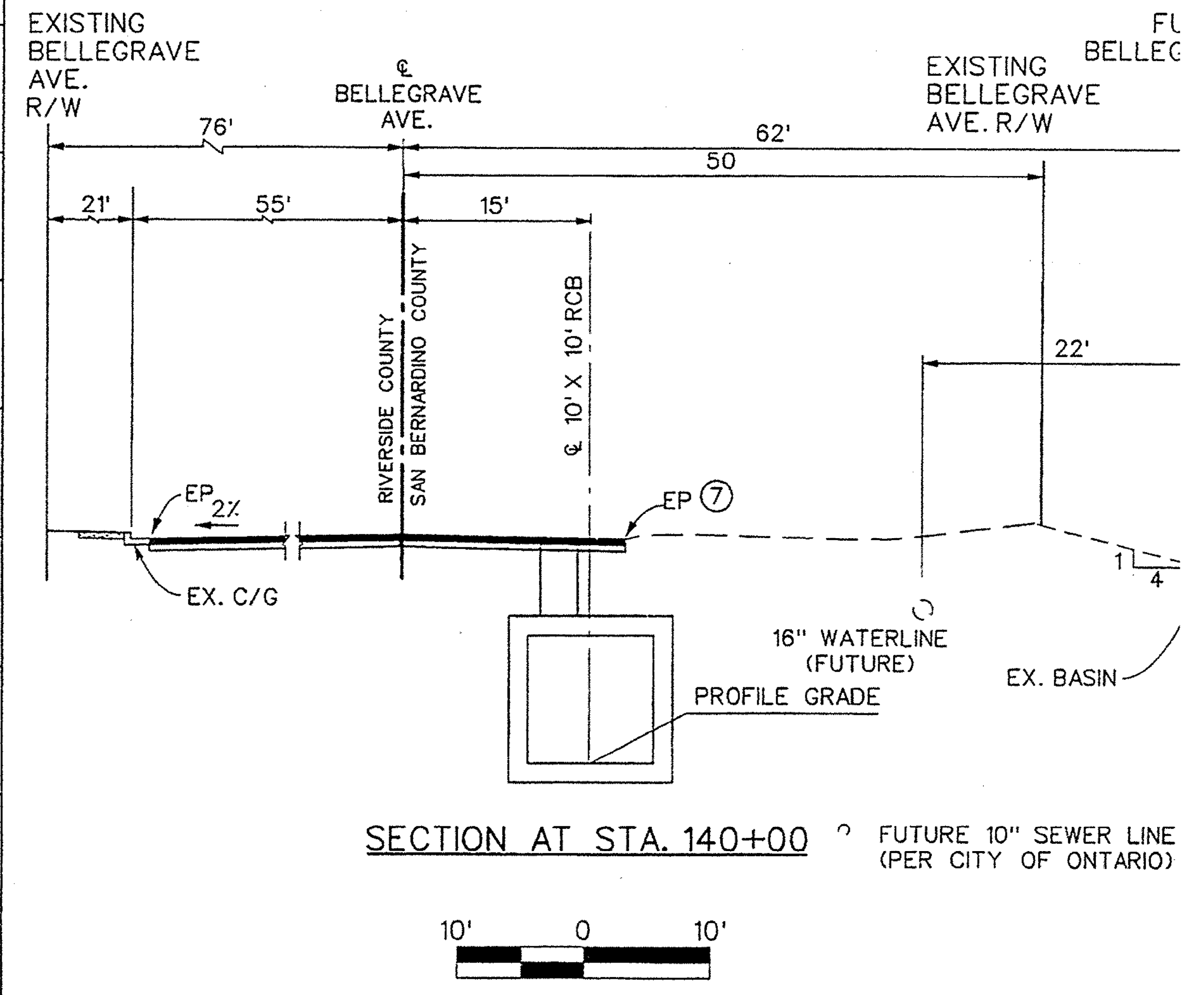


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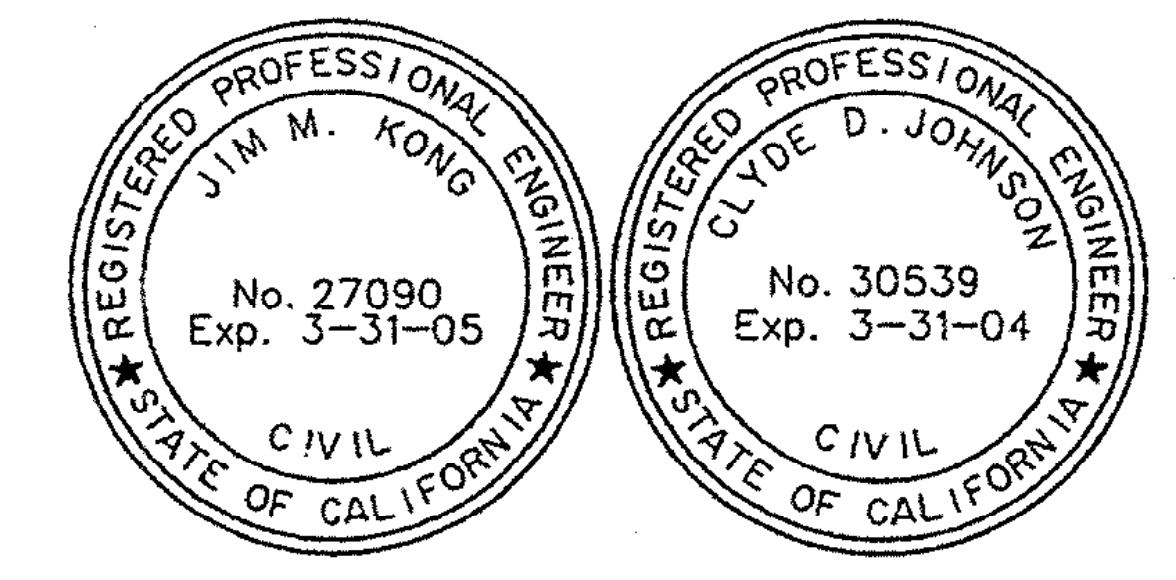
BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-0	
				DESIGNED BY: G.H./A.MARTINEZ		APPROVED BY:		DRAWING NO. 2-31	
				DRAWN BY: M. HOWARD		RECOMMENDED FOR APPROVAL BY:		SHEET NO. 35 OF	
				DATE DRAWN: DEC. 2003		DATE: 12/30/03 R.E. No. 27090		STA. 135+00 TO STA. 139+00	
				CHECKED BY:		DATE: 12/29/03 R.E. No. 30539			



300' (1)
 10' W X 10' H RCB
 HYDRAULIC DATA
 $Q_{100} = 720$ cfs
 $V_{100} = 12$ fps

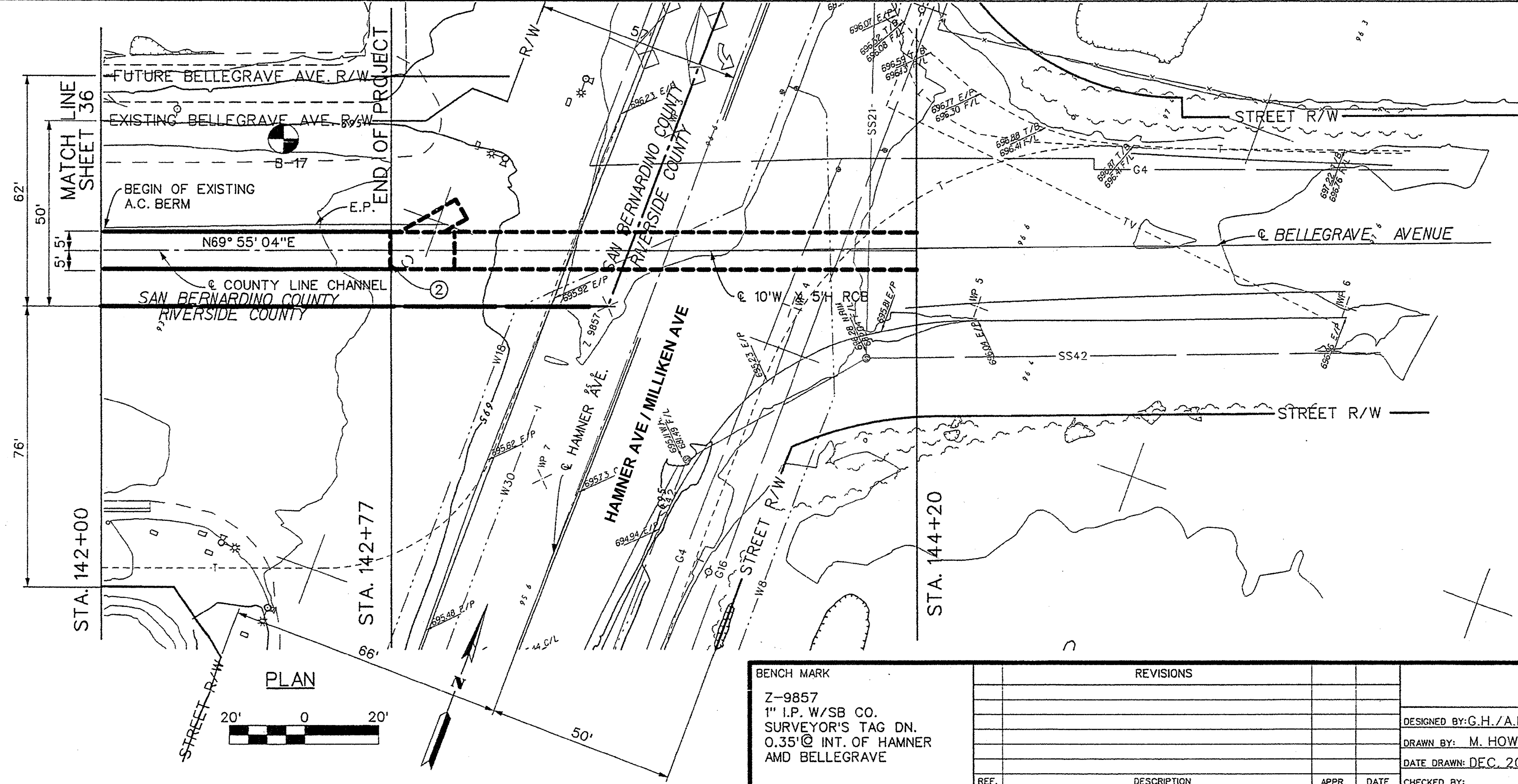
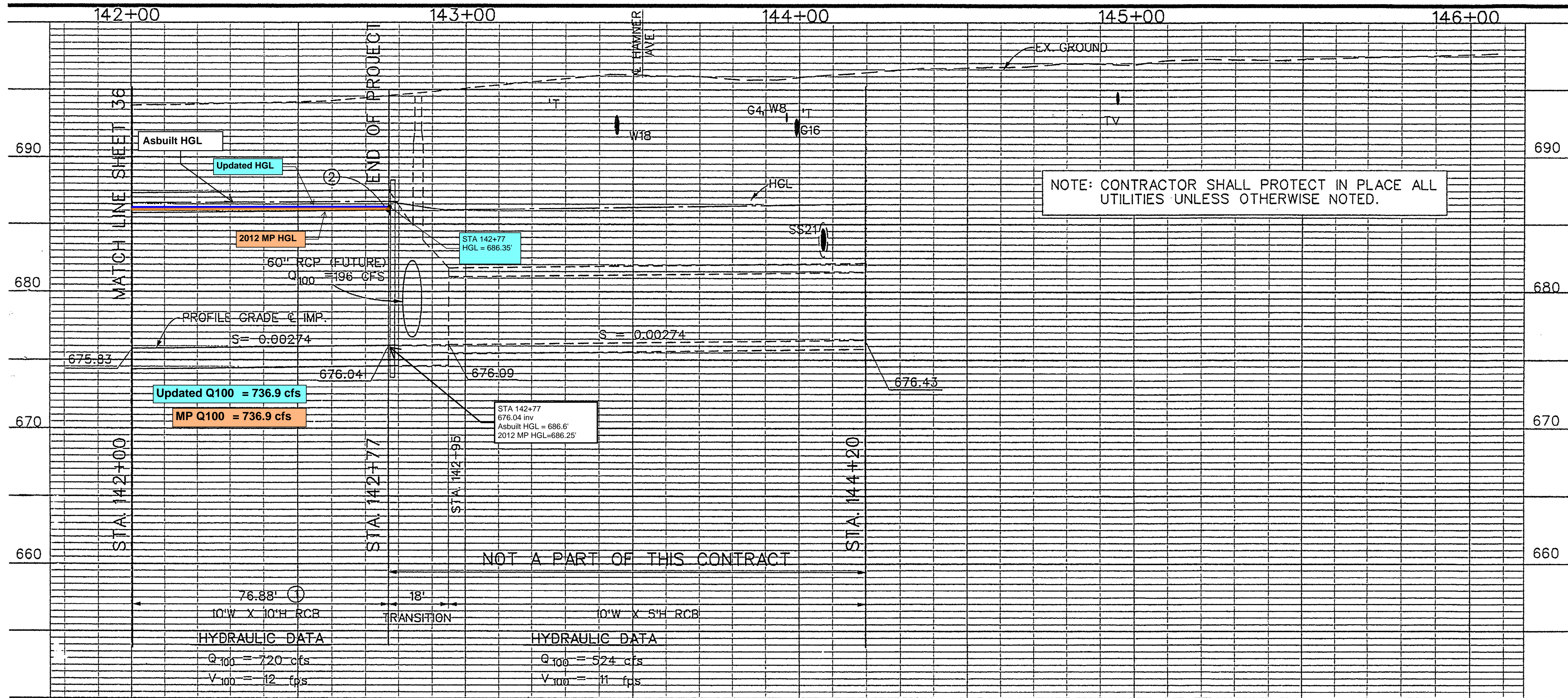


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 - CONSTRUCT AC DIKE PER DETAILS ON SHEET 72.
 - CONSTRUCT JS NO. 3 PER STD. JS228.
 - CONSTRUCT CB NO. 1 PER STD. CB100 W= 7.0', V= 5.0', 11.3' - 24" RCP CL IV. SEE PROFILE ON SHEET 71.
 - CONSTRUCT 3" AC APRON AROUND INLET PER DETAILS ON SHEET 72.
 - EXISTING EDGE OF PAVEMENT NORTH OF THE COUNTY LINE BETWEEN JAMESTOWN AVE. AND HAMNER AVE. VARIES FROM 0' TO 22'. PAVEMENT SHALL BE REPLACED WITH 0.43' TYPE 'B' AC AND 0.75' CLASS 2 AG. BASE WITHIN THE DISTRICT'S PAY LINES. ANY PAVEMENT REMOVED OUTSIDE THE PAY LIMITS SHALL BE REPLACED IN KIND BY THE CONTRACTOR.

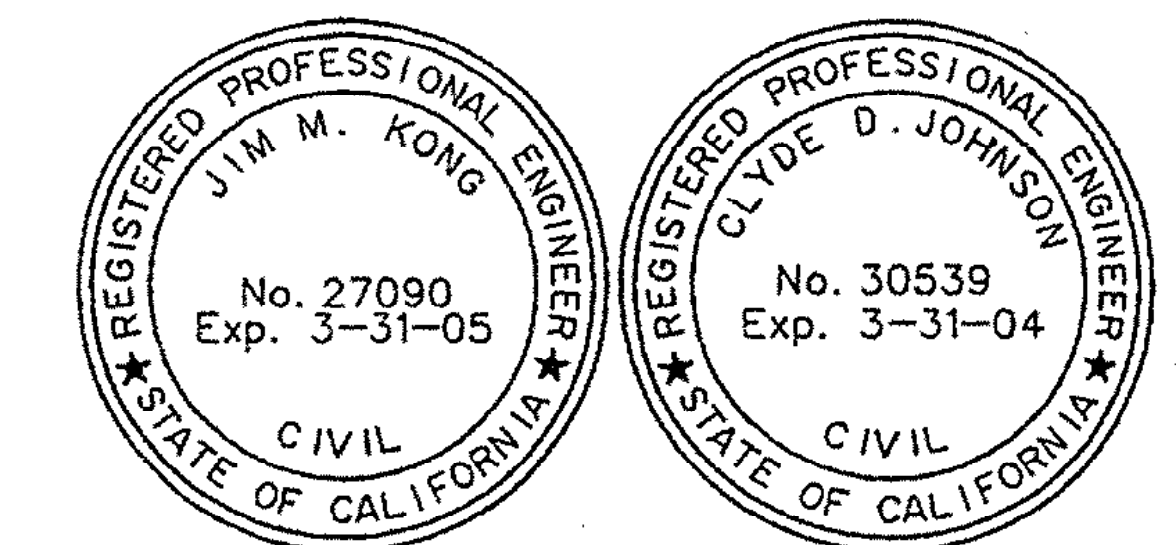


#1-317-1

BENCH MARK		REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL		PROJECT NO. 2-0-0:	
REF.	DESCRIPTION	APPR.	DATE	DESIGNED BY: G.H./A. MARTINEZ	RECOMMENDED FOR APPROVAL BY:	APPROVED BY:		DRAWING NO. 2-31:	
				DRAWN BY: M. HOWARD				SHEET NO. 36	OF 1
				DATE DRAWN: DEC. 2003	DATE: 12/30/03 R.E. No. 27090	DATE: 12/30/03 R.E. No. 30539			



- NOTES**
- CONSTRUCT 10' X 10' RCB PER CALTRANS STD. D80. TRANSVERSE CONSTRUCTION JOINT PER NOTE 6 ON SHEET 38.
 - INSTALL CONCRETE BULKHEAD PER DETAILS ON SHEET 74.



#1-317-1

BENCH MARK Z-9857 1" I.P. W/SB CO. SURVEYOR'S TAG DN. 0.35' @ INT. OF HAMNER AND BELLEGRAVE	REVISIONS		RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		COUNTY LINE CHANNEL STA. 142+00 TO STA. 142+77	PROJECT NO. 2-0-0 DRAWING NO. 2-31 SHEET NO. 37 OF
	DESIGNED BY: G.H. / A. MARTINEZ DRAWN BY: M. HOWARD DATE DRAWN: DEC. 2003	RECOMMENDED FOR APPROVAL BY: SENIOR CIVIL ENGINEER DATE: 12/30/02 R.E. No. 27090	APPROVED BY: CHIEF DESIGN & CONSTRUCTION DATE: 12/30/02 R.E. No. 30539	CHECKED BY:		

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	DESCRIPTION	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	RADIUS	ANGLE	ANG PT	MAN H
1	IS A SYSTEM OUTLET	1018.47	658.65	1													
2	IS A REACH	1018.47	658.65	1										0.00	0.00	0.00	0
3	IS A REACH	1129.56	659.22	1										0.00	0.00	0.00	0
4	IS A JUNCTION	1139.56	659.31	1	3	0	0.013	22.6	0.0	659.72	0.00	90.00	0.00				
5	IS A REACH	2100.00	667.78	1										0.00	0.00	0.00	0
6	IS A JUNCTION	2114.00	667.91	1	3	0	0.013	10.1	0.0	667.78	0.00	90.00	0.00				
7	IS A REACH	2205.40	668.71	1										0.00	0.00	0.00	0
8	IS A JUNCTION	2217.40	669.32	2	0	0	0.013	0.0	0.0	0.00	0.00	0.00	0.00				
9	IS A REACH	2928.21	675.95	2										0.00	0.00	0.00	0
10	IS A JUNCTION	2933.71	676.00	2	3	0	0.013	4.3	0.0	679.33	0.00	90.00	0.00				

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	11 IS A REACH	*	*	*					
	U/S DATA	STATION	INVERT	SECT	N	RADIUS	ANGLE	ANG PT	MAN H
		3100.00	677.55	2	0.013	0.00	0.00	0.00	0

ELEMENT NO	12 IS A SYSTEM HEADWORKS			*					
	U/S DATA	STATION	INVERT	SECT		W S ELEV			
		3100.00	677.55	2		677.55			

NO EDIT ERRORS ENCOUNTERED-COMPUTATION IS NOW BEGINNING

** WARNING NO. 2 ** - WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN HDWKDS, W.S.ELEV = INV + DC

SUBAREA 29
HAVEN LINE
2022-04-25

STATION	INVERT ELEV	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD	ENERGY GRD.EL.	SUPER ELEV	CRITICAL DEPTH	HGT/DIA	BASE/ID NO.	ZL	NO PIER	AVBPR
L/ELEM	SO					SF AVE	HF		NORM DEPTH			ZR		
1018.47	658.65	14.550	673.200	700.4	13.93	3.015	676.215	0.00	6.686	8.00	0.00	0.00	0	0.00
111.09	0.00513					.005897	0.66		7.324			0.00		
1129.56	659.22	14.635	673.855	700.4	13.93	3.015	676.870	0.00	6.686	8.00	0.00	0.00	0	0.00
JUNCT STR	0.00900					.005710	0.06					0.00		
1139.56	659.31	14.985	674.295	677.8	13.48	2.823	677.118	0.00	6.592	8.00	0.00	0.00	0	0.00
960.44	0.00882					.005523	5.30		5.370			0.00		
2100.00	667.78	11.819	679.599	677.8	13.48	2.823	682.422	0.00	6.592	8.00	0.00	0.00	0	0.00
JUNCT STR	0.00929					.005441	0.08					0.00		
2114.00	667.91	11.932	679.842	667.7	13.28	2.740	682.582	0.00	6.549	8.00	0.00	0.00	0	0.00
91.40	0.00875					.005359	0.49		5.326			0.00		
2205.40	668.71	11.622	680.332	667.7	13.28	2.740	683.072	0.00	6.549	8.00	0.00	0.00	0	0.00
JUNCT STR	0.05083					.006460	0.08					0.00		
2217.40	669.32	10.286	679.606	667.7	15.11	3.547	683.153	0.00	6.560	7.50	0.00	0.00	0	0.00
710.81	0.00933					.007561	5.37		5.564			0.00		
2928.21	675.95	9.031	684.981	667.7	15.11	3.547	688.528	0.00	6.560	7.50	0.00	0.00	0	0.00
JUNCT STR	0.00909					.007513	0.04					0.00		
2933.71	676.00	9.113	685.113	663.4	15.02	3.501	688.614	0.00	6.544	7.50	0.00	0.00	0	0.00
166.29	0.00932					.007464	1.24		5.536			0.00		
3100.00	677.55	8.804	686.354	663.4	15.02	3.501	689.855	0.00	6.544	7.50	0.00	0.00	0	0.00

658.65 661.77 664.89 668.01 671.13 674.25 677.37 680.49 683.61 686.73 689.85

N O T E S

1. GLOSSARY

- I = INVERT ELEVATION
- C = CRITICAL DEPTH
- W = WATER SURFACE ELEVATION
- H = HEIGHT OF CHANNEL
- E = ENERGY GRADE LINE
- X = CURVES CROSSING OVER
- B = BRIDGE ENTRANCE OR EXIT
- Y = WALL ENTRANCE OR EXIT

2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY

PRIVATE ENGINEER'S NOTICE TO CONTRACTOR

- ALL AVAILABLE RECORDS FROM THE CITY AND UTILITY COMPANIES INVOLVED HAVE BEEN INVESTIGATED AND ALL KNOWN UTILITY CONDUITS AND SUBSTRUCTURES ARE SHOWN HEREON. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE ALL UTILITY CONDUITS AND SUBSTRUCTURES SHOWN OR NOT SHOWN ON THESE PLANS BY "POI HOLING" PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BEAR THE TOTAL EXPENSE OF REPAIR AND/OR REPLACEMENT OF SAID UTILITY CONDUITS AND SUBSTRUCTURES DAMAGED BY HIS OPERATION IN CONNECTION WITH THE LIMITS OF THIS PROJECT. THE CONTRACTOR IS TO NOTIFY THE ENGINEER OF RECORD IMMEDIATELY WITH ANY DISCREPANCIES. COMMENCEMENT OF WORK INDICATES ACCEPTANCE OF ALL EXISTING UTILITIES SHOWN OR NOT SHOWN BY THE CONTRACTOR.
- EXISTING UTILITIES SHALL BE MAINTAINED IN-PLACE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED. RELOCATION OR REMOVAL OF ANY EXISTING UTILITIES NOT COVERED BY THESE PLANS SHALL BE PERFORMED BY OR UNDER THE DIRECTION OF THE RESPECTIVE UTILITY OWNERS AT THE EXPENSE OF THE DEVELOPER.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PRESERVATION OF ALL, IF ANY, EXISTING SURVEY MONUMENTS.
- THE CONTRACTOR SHALL POSSESS A VALID STATE CONTRACTOR'S LICENSE AND SHALL BE REQUIRED TO POSSESS A VALID CITY BUSINESS LICENSE WHILE PERFORMING WORK ON THIS PROJECT.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS PRIOR TO COMMENCEMENT OF ANY WORK COVERED BY THESE PLANS.
- THE CONTRACTOR(S) AGREES THAT HE SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING THE SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO WORKING HOURS.
- THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE OWNER AND ENGINEER OF RECORD HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT FOR LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER AND/OR THE ENGINEER OF RECORD.
- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN HEREON AT THE JOB SITE PRIOR TO ANY CONSTRUCTION. THE ENGINEER OF RECORD SHALL BE NOTIFIED OF ANY DISCREPANCIES. REVISIONS TO THE PLAN SHALL BE APPROVED BY THE ENGINEER IN WRITING PRIOR TO IMPLEMENTATION.
- UNAUTHORIZED CHANGES & USES: THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE FOR, OR LIABLE FOR, UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

UTILITIES

NOTIFICATION:
CONTRACTOR SHALL CONTACT THE FOLLOWING AGENCIES AND UTILITY COMPANIES AT LEAST TWO WORKING DAYS PRIOR TO STARTING CONSTRUCTION:

UNDERGROUND SERVICE ALERT OF SOUTHERN CALIFORNIA	(800) 227-2600
SOUTHERN CALIFORNIA GAS	(951) 335-3928
TIME WARNER CABLE	(951) 975-3342
INLAND EMPIRE UTILITIES AGENCY	(951) 993-1522
CITY OF ONTARIO	(909) 395-2150
PACIFIC PIPELINE SYSTEM INC.	(526) 728-2351
VERIZON-ENGINEERING DEPARTMENT	(951) 469-6369
SOUTHERN CALIFORNIA EDISON	
ONTARIO DISTRICT EDISON OPERATOR	(951) 930-8591

BASIS OF BEARINGS

THE BEARINGS SHOWN HEREON ARE BASED ON THE GRID BEARING "NORTH 0°41'49" WEST" BETWEEN HORIZONTAL CONTROL STATIONS "20798" AND "20765" AS PER RECORDS ON FILE WITH THE SAN BERNARDINO COUNTY SURVEYOR.

DATUM STATEMENT

COORDINATES SHOWN HEREON ARE BASED ON THE CALIFORNIA COORDINATE SYSTEM (CCS 83) ZONE V, NAD 83 (1992.60 EPOCH ADJUSTMENT), AS PER RECORDS ON FILE WITH THE SAN BERNARDINO COUNTY SURVEYOR.

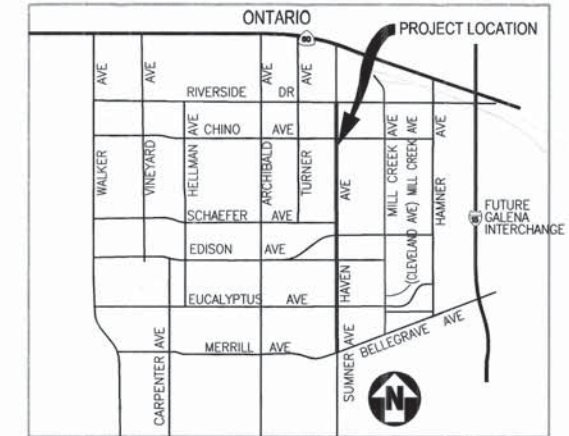
UNLESS OTHERWISE NOTED, ALL DISTANCES SHOWN HEREON ARE GROUND. TO OBTAIN GRID DISTANCES, MULTIPLY GROUND DISTANCES BY 0.99997816761.

CITY OF ONTARIO

STORM DRAIN IMPROVEMENT PLANS

HAVEN AVE

FROM BELLEGRAVE AVE TO RIVERSIDE DR



GENERAL NOTES

- ALL WORK SHALL BE DONE IN STRICT CONFORMANCE WITH THE CURRENT CITY OF ONTARIO STANDARD SPECIFICATIONS AND STANDARD DRAWINGS AND CURRENT SPPWC STANDARD SPECIFICATIONS AND STANDARD DRAWINGS UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER.
- ANY CONTRACTOR PERFORMING WORK ON THIS PROJECT SHALL FAMILIARIZE HIMSELF/HERSELF WITH THE SITE AND SHALL BE SOLELY RESPONSIBLE FOR ANY DAMAGE TO EXISTING FACILITIES RESULTING DIRECTLY OR INDIRECTLY FROM HIS/HER OPERATIONS, WHETHER OR NOT THE FACILITY IS SHOWN ON THESE PLANS.
- ALL OBSTRUCTIONS WITHIN THE AREA TO BE IMPROVED SHALL BE REMOVED AND/OR RELOCATED AT THE DIRECTION OF THE CITY ENGINEER. UTILITIES ARE TO BE RELOCATED BY THEIR RESPECTIVE OWNERS UNLESS NOTED OTHERWISE. THE CONTRACTOR IS REFERRED TO SECTION 5 OF THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
- UTILITY LINE LOCATIONS WERE TAKEN FROM AVAILABLE RECORD DATA AND WERE NOT LOCATED IN THE FIELD, UNLESS OTHERWISE NOTED ON THE PLAN. THE CONTRACTOR IS REFERRED TO SECTION 5 OF THE STANDARD SPECIFICATIONS.
- IN CASE OF ANY ACCIDENTS INVOLVING SAFETY MATTERS COVERED BY SECTION 6409(B) OF THE CALIFORNIA LABOR CODE, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE STATE DIVISION OF INDUSTRIAL SAFETY.
- STATE LAW (SB 3019) REQUIRES THE CONTRACTOR TO CONTACT UNDERGROUND SERVICE ALERT AND OBTAIN AN IDENTIFICATION NUMBER PRIOR TO THE ISSUANCE OF CITY'S ENCROACHMENT PERMIT. THE CONTRACTOR SHALL NOTIFY UNDERGROUND SERVICE ALERT AT 1-800-227-2600 A MINIMUM OF 48 HOURS IN ADVANCE OF ANY CONSTRUCTION ACTIVITIES.
- PRIOR TO THE START OF ANY WORK THE CONTRACTOR SHALL OBTAIN A BUSINESS LICENSE FROM THE CITY OF ONTARIO. THE CONTRACTOR SHALL ALSO OBTAIN AN ENCROACHMENT PERMIT FROM THE ENGINEERING DEPARTMENT NO LESS THAN 48 HOURS PRIOR TO START OF ANY CONSTRUCTION WITHIN THE PUBLIC RIGHT-OF-WAY.
- THE CONTRACTOR SHALL OBTAIN A TRAFFIC CONTROL PERMIT 48 HOURS PRIOR TO TIME OF CONSTRUCTION WITHIN THE PUBLIC RIGHT-OF-WAY. THE CONTRACTOR IS REFERRED TO SECTION 7-10.3 OF THE STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION.
- THE CONTRACTOR SHALL RENEW OR REPLACE ANY EXISTING TRAFFIC STRIPING AND/OR PAVEMENT MARKINGS, WHICH DURING HIS OPERATIONS HAVE BEEN EITHER REMOVED OR THE EFFECTIVENESS OF WHICH HAS BEEN REDUCED. RENEWAL OF TRAFFIC STRIPING AND MARKINGS SHALL BE DONE USING REFLECTIVE THERMO-PLASTIC MARKINGS IN CONFORMANCE WITH SECTION 84 & 85 OF THE CALTRANS STANDARD SPECIFICATIONS AND AS DIRECTED BY THE ENGINEER. ALL REGULATORY, WARNING, AND GUIDE SIGNS SHALL HAVE 3M DIAMOND, VIP GRADE SHEETING WITH SERIES 1160 PROTECTIVE OVERLAY FILM.
- THE LAND SURVEYORS ACT, SECTION 8771 OF THE BUSINESS & PROFESSIONAL CODE, AND SECTIONS 732.5, 1492-5, 1810-5 OF THE STREETS AND HIGHWAY CODE REQUIRE THAT SURVEY MONUMENTS SHALL BE PROTECTED AND PERPETUATED. IT SHALL BE THE RESPONSIBILITY OF THE DEVELOPER OR OTHERS PERFORMING THE CONSTRUCTION WORK TO RETAIN A QUALIFIED REGISTERED CIVIL ENGINEER AND/OR LICENSED LAND SURVEYOR PRIOR TO THE START OF CONSTRUCTION TO LOCATE, REFERENCE AND FILE THE NECESSARY CORNER RECORDS WITH THE COUNTY SURVEYOR'S OFFICE FOR SURVEY CONTROL POINTS/ MONUMENTS THAT EXIST AS SHOWN ON RECORDED TRACT MAPS, PARCEL MAPS, RECORDS OF SURVEYS AND HIGHWAY MAPS, AND MAY BE DISTURBED OR DAMAGED BY THE PROPOSED CONSTRUCTION. AFTER THE COMPLETION OF THE PROPOSED CONSTRUCTION, SAID MONUMENTS AND/OR CONTROL SURVEY POINTS SHALL BE RESET TO THE NEW SURFACE IN ACCORDANCE WITH CURRENT PROFESSIONAL LAND SURVEYING PRACTICES. CORNER RECORDS SHALL BE FILED WITH THE COUNTY SURVEYOR FOR ALL THE NEW MONUMENTS SET. THE DEVELOPER SHALL PROVIDE THE CITY WITH A COMPLETED SET OF "AS BUILT", MYLAR
- DRAWINGS PRIOR TO THE FINAL INSPECTION. A CITY ACCEPTED/APPROVED SET OF PLANS SHALL BE KEPT ON THE JOB SITE AT ALL TIMES.
- A PRE-CONSTRUCTION MEETING SHALL OCCUR PRIOR TO CONSTRUCTION.
- ATTENDEES SHALL INCLUDE A CITY REPRESENTATIVE AND THE CONTRACTOR WHO WILL PERFORM THE WORK. "CUT-SHEETS" SHALL BE PROVIDED TO THE CITY AT THIS MEETING FOR ITS REVIEW.
- CITY ACCEPTANCE OF PLANS DOES NOT RELIEVE THE DEVELOPER FROM RESPONSIBILITY FOR THE CORRECTION OF ERROR AND OMISSION DISCOVERED DURING CONSTRUCTION. UPON REQUEST OF THE CITY INSPECTOR, THE REQUIRED PLAN REVISIONS SHALL BE PROMPTLY SUBMITTED TO THE CITY ENGINEER FOR REVIEW.
- ANY REQUIRED RIGHT-OF-WAY OR EASEMENT SHALL BE DEDICATED TO AND ACCEPTED BY THE CITY PRIOR TO COMMENCEMENT OF CONSTRUCTION OF THE IMPROVEMENTS WITHIN THE REQUIRED RIGHT-OF-WAY OR EASEMENT.

STORM DRAIN NOTES

- WHERE UTILITIES NEED TO BE SUPPORTED, SAID SUPPORTS SHALL BE IN ACCORDANCE WITH SPPWC STANDARD 224-1 UNLESS OTHERWISE INDICATED.
- PRIOR TO STARTING CONSTRUCTION, THE CONTRACTOR SHALL VERIFY LOCATION AND ELEVATION OF EXISTING SEWER MAIN(S) AND NOTIFY THE DESIGN ENGINEER OF ANY VARIATION FROM DESIGN.
- THE PIPE SHOWN HEREON SHALL BE INSTALLED IN ACCORDANCE WITH CASE III BEDDING AS SHOWN ON LACDPW STANDARD 3080-2, UNLESS OTHERWISE SHOWN. "W" VALUE SHALL BE AS SPECIFIED ON LACDPW STANDARD 3080-2 FOR CASE III BEDDING, NOTES 3(a), 3(b), AND 3(c). IF THE "W" VALUE AT THE TOP OF THE PIPE IS EXCEEDED, THE BEDDING SHALL BE MODIFIED AND/OR PIPE OF ADDITIONAL STRENGTH SHALL BE PROVIDED. ALL PROPOSED MODIFICATIONS TO THIS REQUIREMENT SHALL BE APPROVED BY THE CITY ENGINEER.
- MINIMUM COVER OF STORM DRAIN MAIN SHALL BE 7 FEET FROM THE FINISHED SURFACE PER STANDARD DRAWING 1302-1304.
- STORM DRAIN MAIN STATIONING IS PER PIPE CENTERLINE.
- STREET CENTERLINE STATIONING IS PER THE STREET IMPROVEMENT PLANS AND PROVIDED FOR REFERENCE.

ADDITIONAL STORM DRAIN NOTES

- CONTRACTOR SHALL NOT BACKFILL TRENCH UNTIL THE CITY INSPECTOR HAS OBTAINED AS-BUILT STATIONING ON ALL STRUCTURES.
- AT COMPLETION OF ALL UTILITIES AND PRIOR TO ACCEPTANCE OF THE WATERLINES AND PRIOR TO PAVING, THE DEVELOPER SHALL HIRE A CITY APPROVED VIDEO COMPANY TO VIDEO TAPE THE PIPELINES. CITY SHALL REVIEW SAID VIDEOTAPES FOR POTENTIAL CONSTRUCTION DEFECTS PRIOR TO ACCEPTANCE OF THE PROJECT.

LEGEND

	ROADWAY CENTERLINE
	PROPERTY - R/W LINE
	PROPOSED PIPELINE
	PIPELINE CENTERLINE STATION
	DOMESTIC WATER LINE
	SANITARY SEWER LINE
	RECYCLED WATER LINE
	STORM DRAIN LINE
	TELEMETRY PULL BOX/ TERMINAL CABINET
	WATER SERVICE METER
	SEWER OR STORM SEWER MANHOLE
	ISOLATION VALVE
	AIR & VACUUM RELEASE VALVE
	END OF LINE BLOWOFF ASSEMBLY
	LINE DRAIN BLOWOFF ASSEMBLY
	CORROSION TEST STATION
	WATER QUALITY SAMPLING STATION
	FIRE HYDRANT

STORM DRAIN CONSTRUCTION NOTES

1	CONSTRUCT 18" RCP (D-LOAD PER PROFILE).	1589 LF.
2	CONSTRUCT 24" RCP (D-LOAD PER PROFILE).	584 LF.
3	CONSTRUCT 36" RCP (D-LOAD PER PROFILE).	221 LF.
4	CONSTRUCT 42" RCP (D-LOAD PER PROFILE).	1055 LF.
5	CONSTRUCT 54" RCP (D-LOAD PER PROFILE).	649 LF.
6	CONSTRUCT 60" RCP (D-LOAD PER PROFILE).	1107 LF.
7	CONSTRUCT 72" RCP (D-LOAD PER PROFILE).	1631 LF.
8	CONSTRUCT 84" RCP (D-LOAD PER PROFILE).	2517 LF.
9	CONSTRUCT 96" RCP (D-LOAD PER PROFILE).	1041 LF.
10	CONSTRUCT BRICK & MORTAR BULKHEAD SEE SHEET 17.	10 EA
11	CONSTRUCT WATER QUALITY STRUCTURE (NSBB 11-16-114 WITH MINIMUM 30" MANHOLES).	1 EA
12	CONSTRUCT TRANSITION STRUCTURE PIPE TO PIPE PER SPPWC STD. PLAN 340-2.	13 EA
13	CONSTRUCT MANHOLE NO. 2 PER CITY OF ONTARIO STD. DWG. 3008.	23 EA
14	CONSTRUCT MANHOLE PIPE TO PIPE (LARGE SIDE INLET) PER SPPWC STD. PLAN 322-2.	14 EA
15	CATCH BASIN NO. 2 PER CITY OF ONTARIO STD. DWG. 3003 (W PER PLANS)	32 EA
16	CONSTRUCT MANHOLE-CONCRETE BOX STORM DRAIN PER SPPWC STD. PLAN 323-2.	3 EA
18	CONSTRUCT 48" RCP (D-LOAD PER PROFILE).	1259 LF.
19	CONSTRUCT 78" RCP (D-LOAD PER PROFILE).	669 LF.
20	CONSTRUCT 90" RCP (D-LOAD PER PROFILE).	2442 LF.
21	CONSTRUCT CONCRETE ENCASEMENT PER CITY OF ONTARIO STD. 2102 CASE 4	1 EA
22	CONSTRUCT PIPE ANCHORS PER SPPWC STD. PLAN 221-2.	2 EA
23	CONSTRUCT CONCRETE ENCASEMENT PER CITY OF ONTARIO STD. 2102 CASE 3	4 EA

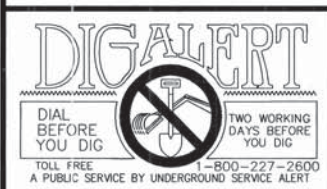
ESTIMATED QUANTITIES

LIST OF ABBREVIATIONS

∠PT	ANGLE POINT	NTS	NOT TO SCALE
AB	AGGREGATE BASE	OC	ON CENTER
AC	ASPHALTIC CONCRETE	O.C.	ON CURVE
APN	ASSESSORS PARCEL NUMBER	PCC	POINT OF COMPOUND CURVATURE
BC	BEGIN CURVE	PI	POINT OF INTERSECTION
CAB	CRUSHED AGGREGATE BASE	PRC	POINT OF REVERSE CURVATURE
CB	CATCH BASIN	PROP	PROPOSED
∠	CENTERLINE	PRS	PRESSURE REDUCING STATION
C/G	CURB AND GUTTER	PRV	PRESSURE REDUCING VALVE
CONC	CONCRETE	PUE	PUBLIC UTILITY EASEMENT
DW	DOMESTIC WATER	PVI	POINT OF VERTICAL INTERSECTION
DWG	DRAWING	R	RADIUS
EC	END OF CURVE	RCP	REINFORCED CONCRETE PIPE
ESMT	EASEMENT	RET	RETAINING
EOR	END CURB RETURN	RT	RIGHT
ELEV	ELEVATION	R/W	RECYCLED WATER
EP	EDGE OF PAVEMENT	R/W	RIGHT OF WAY
FG	FINISH GRADE	SD	STORM DRAIN
FL	FLOWLINE ELEVATION	SF	SQUARE FEET
FS	FINISH SURFACE	SS	SANITARY SEWER
GB	GRADE BREAK	ST	STREET
HORIZ	HORIZONTAL	STA	STATION
INT	INTERSECTION	STD	STANDARD
L	LENGTH	STR	STRUCTURE
LF	LINEAR FEET	TC	TOP OF CURB
LT	LEFT	TCE	TEMPORARY CONSTRUCTION EASEMENT
MAX	MAXIMUM	TELE	TELEPHONE
MH	MANHOLE	TT#	TENTATIVE TRACT NO
MIN	MINIMUM	TYP	TYPICAL
NO.	NUMBER	VZN	VERTICAL
		WQ	WATER QUALITY

SHEET INDEX

SHEET NO.	DESCRIPTION
1.	TITLE SHEET
2.	DETAILS
3.	SHEET INDEX
4.	FROM BELLEGRAVE AVE TO 270' ± N/O BELLEGRAVE AVE
5.	FROM 270' ± N/O BELLEGRAVE AVE TO 1,070' ± N/O BELLEGRAVE AVE
6.	FROM 1,070' ± N/O BELLEGRAVE AVE TO 2,070' ± N/O BELLEGRAVE AVE
7.	FROM 490' ± S/O EUCALYPTUS AVE TO 510' ± N/O EUCALYPTUS AVE
8.	FROM 510' ± N/O EUCALYPTUS AVE TO 1,510' ± N/O EUCALYPTUS AVE
9.	FROM 1,510' ± N/O EUCALYPTUS AVE TO 2,510' ± N/O EUCALYPTUS AVE
10.	FROM 2,510' ± N/O EUCALYPTUS AVE TO 430' ± S/O EDISON AVE
11.	FROM 430' ± S/O EDISON AVE TO 570' ± N/O EDISON AVE
12.	FROM 570' ± N/O EDISON AVE TO 1,570' ± N/O EDISON AVE
13.	FROM 1,570' ± N/O EDISON AVE TO 2,570' ± N/O EDISON AVE
14.	FROM 2,570' ± N/O EDISON AVE TO 410' ± S/O CHINO AVE
15.	FROM 410' ± S/O CHINO AVE TO 540' ± N/O CHINO AVE
16.	FROM 540' ± N/O CHINO AVE TO 1,590' ± N/O CHINO AVE
17.	FROM 1,590' ± N/O CHINO AVE TO 580' ± S/O RIVERSIDE DR
18.	LATERAL PROFILES
19.	LATERAL PROFILES
20.	LATERAL PROFILES
21.	LATERAL PROFILES
22.	LATERAL PROFILES



REVISIONS		DESIGNED BY:
MARK	DATE	DATE:
		DRAWN BY:
		DATE:
		CHECKED BY: Hwl
		DATE: 7/30/14

CITY OF ONTARIO	
RECOMMENDED BY:	DATE:
RAYMOND LEE, P.E., ASSISTANT CITY ENGINEER	8/7/14
ACCEPTED BY:	DATE:
LOUIS ABI-YOUNES, P.E., CITY ENGINEER	8-12-14

BENCH MARK No. S.P.S.N 00789	ELEVATION 785.572
SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W, 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, S11" "LS 3258" SET FLUSH IN AC.	
NGVD29 / 1990 ADJ.	

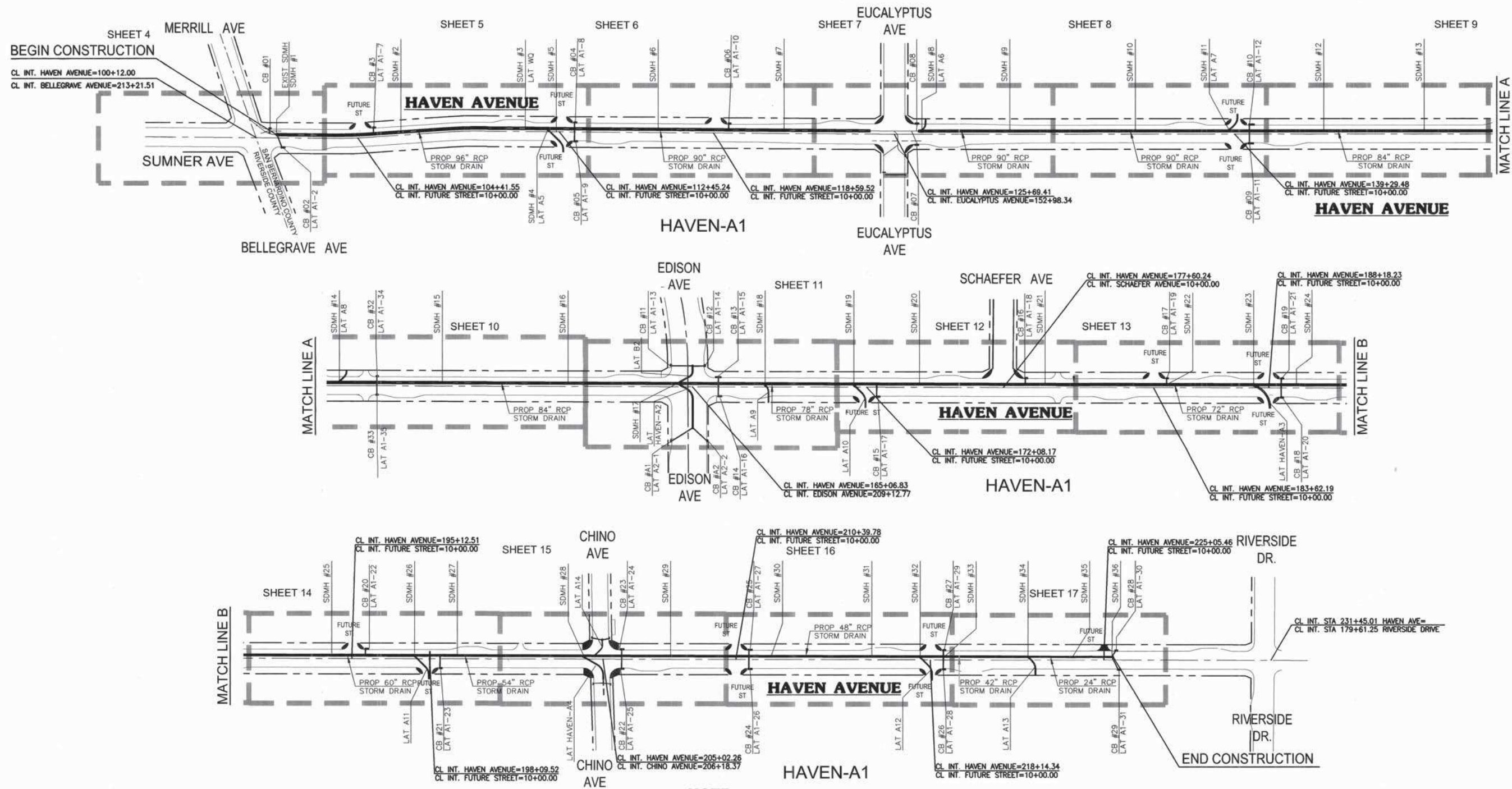
T M A D	901 Via Piemonte, Suite 400
TAYLOR & GAINES	Ontario, California 91764
	Phone: 909.477.6915 Fax: 909.477.6916
	www.ttgcorp.com
CHARLES W. LOCKMAN	42485
	RCE No. 07/07/2014

STORM DRAIN IMPROVEMENT
PLAN AND PROFILE
HAVEN AVENUE
TITLE SHEET

SHEET 1 of 22
CONTRACT
ACCOUNT
DWG. No. M-442
D13601

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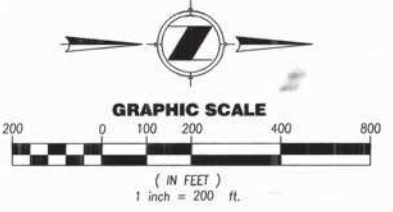
SHEET INDEX MAP



NOTE:

THE NAMES OF THE STORM DRAIN SYSTEM SHOWN ON THESE PLANS ARE BASED ON THE CITY OF ONTARIO MASTER PLAN OF DRAINAGE (DATED 2010). THE CITY'S MASTER PLAN OF DRAINAGE (MPD) FOR NEW MODEL COLONY (DATED MARCH, 2012) REVISED THE STORM DRAIN (SD) LINE NAMES. THE TABLE BELOW SHOWS THE PREVIOUS NAME CONVENTION (SHOWN ON THESE PLANS), AND THE CORRESPONDING NAMES PER THE 2012 MPD.

STORM DRAIN LINE NAME TABLE:		
LINE NAME:	2010 MPD	2012 MPD
HAVEN AVENUE MAIN LINE	HAVEN-A1	HAVN-X-1
LATERAL AT EDISON AVE.	LAT HAVEN-A2	HAVN-X-2
LATERAL AT S.D. STA. 97+79	LAT HAVEN-A3	HAVN-X-3
LATERAL AT CHINO AVE.	LAT HAVEN-A4	HAVN-X-4



REVISIONS		DESIGNED BY:
MARK	DATE	BY

CITY OF ONTARIO

RECOMMENDED BY: *Raymond Lee* 8/7/14
 RAYMOND LEE, P.E., ASSISTANT CITY ENGINEER
 DATE: 8/7/14

ACCEPTED BY: *Louis Abi-Younes* 8-12-14
 LOUIS ABI-YOUNES, P.E., CITY ENGINEER
 DATE: 8-12-14

BENCH MARK No. S.P.S.N 00789 ELEVATION 785.572

SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W. 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, S11" "LS 3258" SET FLUSH IN A.C.

NGVD29 / 1990 ADJ.

T M A D 901 Via Piemonte, Suite 400
 Ontario, California 91764
 Phone: 909.477.6915 Fax: 909.477.6916
 www.tmgcorp.com

TAYLOR & GAINES

Charles W. Lockman 42485 07/07/2014
 CHARLES W. LOCKMAN RCE No. DATE

**STORM DRAIN IMPROVEMENT
 PLAN AND PROFILE
 HAVEN AVENUE**

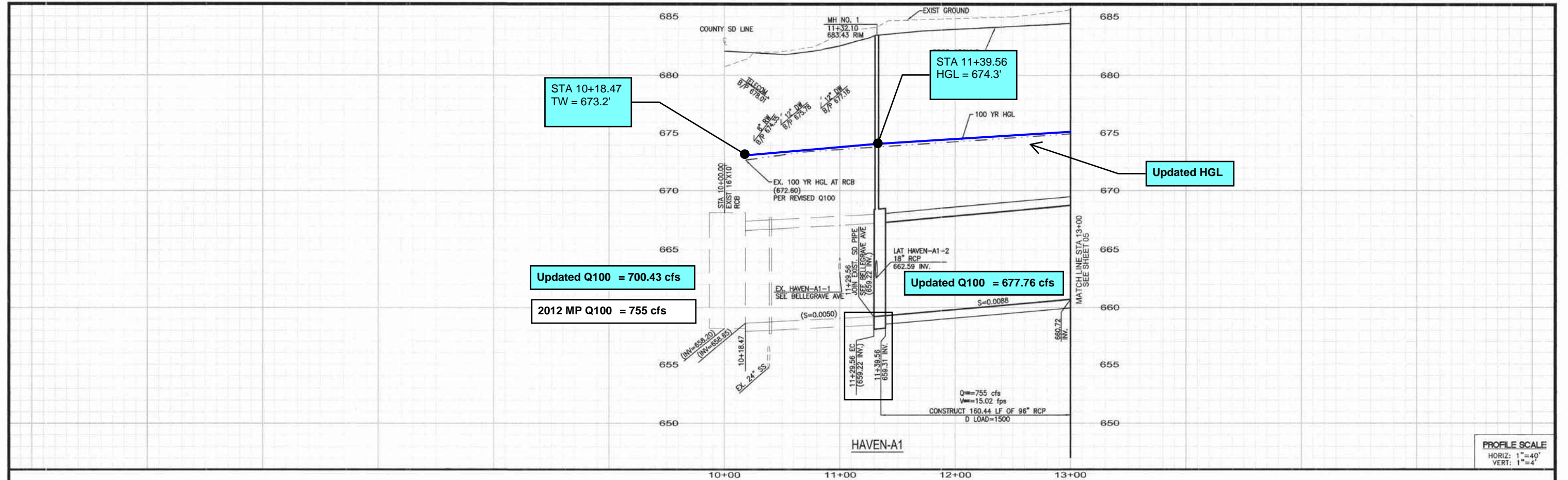
SHEET INDEX MAP

M-442

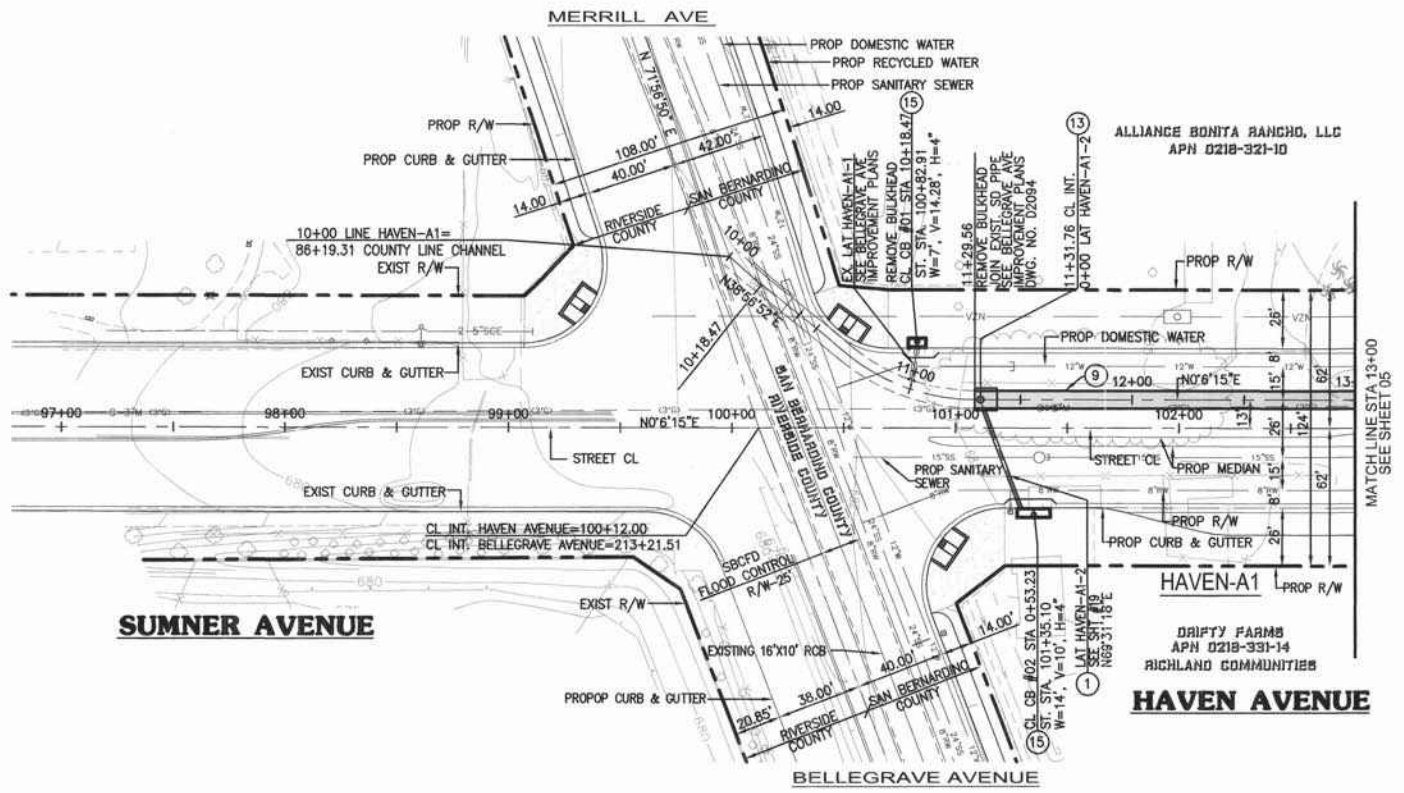
SHEET **3** OF **22**

CONTRACT _____
 ACCOUNT _____
 DWG. NO. **D13603**

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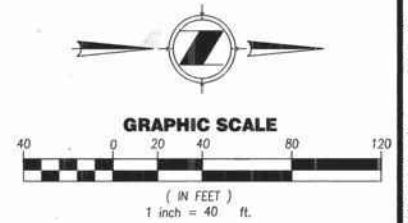


PROFILE SCALE
 HORIZ: 1"=40'
 VERT: 1"=4'



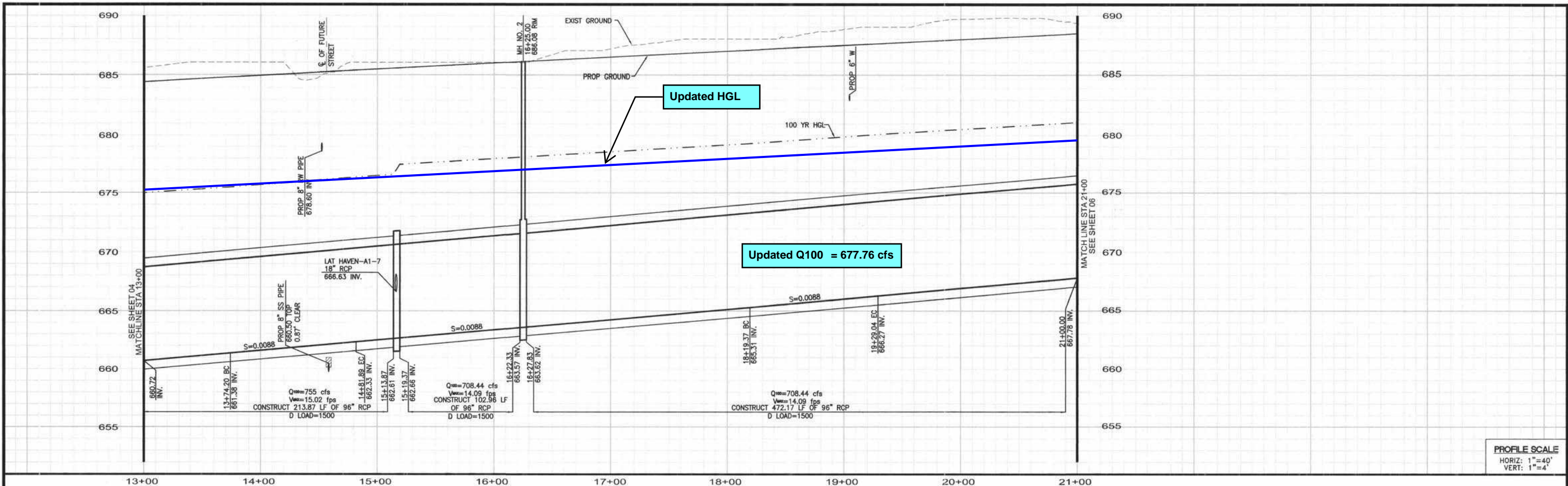
STORM DRAIN CONSTRUCTION NOTES

- ① CONSTRUCT 18" RCP (D-LOAD PER PROFILE).
- ② CONSTRUCT 96" RCP (D-LOAD PER PROFILE).
- ⑬ CONSTRUCT MANHOLE NO. 2 PER CITY OF ONTARIO STD. DWG. 3008.
- ⑮ CATCH BASIN NO. 2 PER CITY OF ONTARIO STD. DWG. 3003 (W PER PLANS)

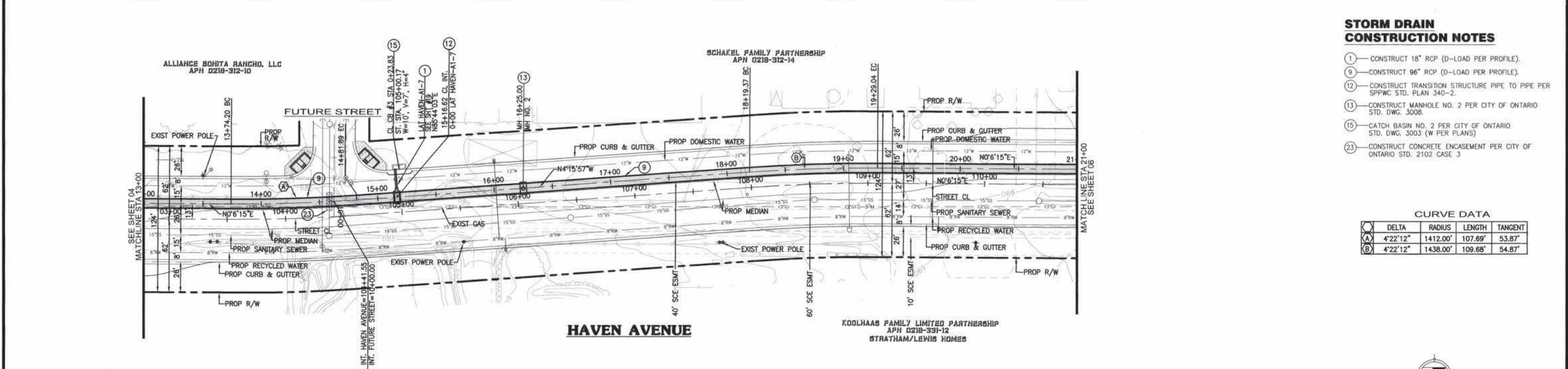


REVISIONS MARK DATE BY APPROVED/RCE No. DATE:		DESIGNED BY: _____ DATE: _____ DRAWN BY: _____ DATE: _____ CHECKED BY: <i>HW</i> <i>kw</i> DATE: 7/30/14		CITY OF ONTARIO RECOMMENDED BY: <i>Raymond Lee</i> 8/7/14 DATE: _____ RAYMOND LEE, P.E./ ASSISTANT CITY ENGINEER ACCEPTED BY: <i>Louis Abi-Younes</i> 9-12-14 DATE: _____ LOUIS ABI-YOUNES, P.E./ CITY ENGINEER		BENCH MARK No. S.P.S.N 00789 ELEVATION: 785.572 SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W, 2" BRASS DISK STAMPED 'T2S, R7W, 1/4, S2, S11" 'LS 3258" SET FLUSH IN A.C. NGVD29 / 1990 ADJ.		T M A D 901 Via Piemonte, Suite 400 Ontario, California 91764 Phone: 909.477.6915 Fax: 909.477.6916 www.tmgcorp.com CHARLES W. LOCKMAN 42485 07/07/2014 RCE No. DATE		STORM DRAIN IMPROVEMENT PLAN & PROFILE HAVEN AVENUE (LINE A-1) FROM BELLEGRAVE AVE TO 270' ± N/O BELLEGRAVE AVE		SHEET 4 OF 22 CONTRACT: _____ ACCOUNT: _____ DWG. No. D13604	
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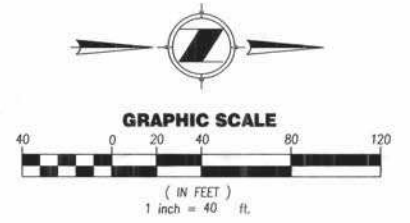
PROFILE SCALE
 HORIZ: 1"=40'
 VERT: 1"=4'



- STORM DRAIN CONSTRUCTION NOTES**
- 1 — CONSTRUCT 18" RCP (D-LOAD PER PROFILE).
 - 9 — CONSTRUCT 96" RCP (D-LOAD PER PROFILE).
 - 12 — CONSTRUCT TRANSITION STRUCTURE PIPE TO PIPE PER SPPWC STD. PLAN 340-2.
 - 13 — CONSTRUCT MANHOLE NO. 2 PER CITY OF ONTARIO STD. DWG. 3008.
 - 15 — CATCH BASIN NO. 2 PER CITY OF ONTARIO STD. DWG. 3003 (W PER PLANS).
 - 23 — CONSTRUCT CONCRETE ENCASUREMENT PER CITY OF ONTARIO STD. 2102 CASE 3.

CURVE DATA

Δ	DELTA	RADIUS	LENGTH	TANGENT
A	4°22'12"	1412.00'	107.69'	53.87'
B	4°22'12"	1438.00'	109.68'	54.87'



<p>REVISIONS</p> <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>MARK</th> <th>DATE</th> <th>BY</th> <th>APPROVED/RCE No.</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>	MARK	DATE	BY	APPROVED/RCE No.					<p>DESIGNED BY: _____ DATE: _____</p> <p>DRAWN BY: _____ DATE: _____</p> <p>CHECKED BY: <i>H.W.</i> DATE: 7/30/14</p>	<p>CITY OF ONTARIO</p> <p>RECOMMENDED BY: <i>Raymond Lee</i> DATE: 8/7/14</p> <p>ACCEPTED BY: <i>Louis Abi-Younes</i> DATE: 8-12-14</p> <p>LOUIS ABI-YOUNES, P.E. / CITY ENGINEER</p>	<p>BENCH MARK No. S.P.S.N 00789 ELEVATION 785.572</p> <p>SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W. 2" BRASS DISK STAMPED 'T2S, R7W, 1/4, S2, 511" 'LS 3258" SET FLUSH IN A.C.</p> <p>NGVD29 / 1990 ADJ.</p>	<p>T M A D</p> <p>TAYLOR & GAINES</p> <p>901 Via Piemonte, Suite 400 Ontario, California 91764 Phone: 909.477.6915 Fax: 909.477.6916 www.ttgcorp.com</p> <p><i>Charles W. Lockman</i> CHARLES W. LOCKMAN RCE No. 42485 DATE: 07/07/2014</p>	<p>STORM DRAIN IMPROVEMENT PLAN & PROFILE</p> <p>HAVEN AVENUE (HAVEN-A1)</p> <p>FROM 270' ± N/O BELLEGRAVE AVE TO 1,070' ± N/O BELLEGRAVE AVE</p>	<p>SHEET 5 OF 22</p> <p>CONTRACT _____</p> <p>ACCOUNT _____</p> <p>DWG. NO. D13605</p>
MARK	DATE	BY	APPROVED/RCE No.											

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WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	1 IS A SYSTEM OUTLET	*	*	*																
	U/S DATA	STATION	INVERT	SECT						W S ELEV										
		992.84	667.23	3						681.50										
ELEMENT NO	2 IS A REACH	*	*	*																
	U/S DATA	STATION	INVERT	SECT							RADIUS	ANGLE	ANG PT	MAN H						
		992.84	667.23	3					N		0.00	0.00	0.00	0						
									0.014											
ELEMENT NO	3 IS A REACH	*	*	*																
	U/S DATA	STATION	INVERT	SECT							RADIUS	ANGLE	ANG PT	MAN H						
		1495.00	668.76	3					N		0.00	0.00	0.00	0						
									0.014											
ELEMENT NO	4 IS A JUNCTION	*	*	*	*	*	*	*	*	*										
	U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4							
		1500.00	668.78	3	4	0	0.014	27.0	0.0	669.70	0.00	30.00	0.00							
ELEMENT NO	5 IS A REACH	*	*	*																
	U/S DATA	STATION	INVERT	SECT							RADIUS	ANGLE	ANG PT	MAN H						
		2351.00	671.44	3					N		0.00	0.00	0.00	0						
									0.014											
ELEMENT NO	6 IS A TRANSITION	*	*	*																
	U/S DATA	STATION	INVERT	SECT																
		2401.00	671.61	2					N											
									0.014											
ELEMENT NO	7 IS A REACH	*	*	*																
	U/S DATA	STATION	INVERT	SECT							RADIUS	ANGLE	ANG PT	MAN H						
		2421.00	671.95	2					N		0.00	0.00	0.00	0						
									0.014											
ELEMENT NO	8 IS A TRANSITION	*	*	*																
	U/S DATA	STATION	INVERT	SECT																
		2471.67	676.62	1					N											
									0.013											
ELEMENT NO	9 IS A REACH	*	*	*																
	U/S DATA	STATION	INVERT	SECT							RADIUS	ANGLE	ANG PT	MAN H						
		2479.67	676.69	1					N		0.00	0.00	0.00	0						
									0.013											
ELEMENT NO	10 IS A SYSTEM HEADWORKS	*	*	*					*											
	U/S DATA	STATION	INVERT	SECT							W S ELEV									
		2479.67	676.69	1							0.00									

NO EDIT ERRORS ENCOUNTERED-COMPUTATION IS NOW BEGINNING

** WARNING NO. 2 ** - WATER SURFACE ELEVATION GIVEN IS LESS THAN OR EQUALS INVERT ELEVATION IN HDWKDS, W.S.ELEV = INV + DC

SUBAREA 29
MILL CREEK LINE
2022-04-25

STATION	INVERT ELEV	DEPTH OF FLOW	W.S. ELEV	Q	VEL	VEL HEAD	ENERGY GRD.EL.	SUPER ELEV	CRITICAL DEPTH	HGT/DIA	BASE/ID NO.	ZL	NO PIER	AVBPR
L/ELEM	SO					SF AVE	HF		NORM DEPTH			ZR		
992.84	667.23	14.270	681.500	1129.9	14.17	3.117	684.617	0.00	8.527	10.00	8.00	0.00	0	0.00
502.16	0.00305					.006170	3.10		10.000			0.00		
1495.00	668.76	15.838	684.598	1129.9	14.17	3.117	687.715	0.00	8.527	10.00	8.00	0.00	0	0.00
JUNCT STR	0.00400					.006024	0.03					0.00		
1500.00	668.78	16.108	684.888	1102.9	13.83	2.970	687.858	0.00	8.391	10.00	8.00	0.00	0	0.00
851.00	0.00313					.005878	5.00		10.000			0.00		
2351.00	671.44	18.450	689.890	1102.9	13.83	2.970	692.860	0.00	8.391	10.00	8.00	0.00	0	0.00
TRANS STR	0.00340					.003744	0.19					0.00		
2401.00	671.61	20.500	692.110	1102.9	8.50	1.122	693.232	0.00	6.071	10.00	13.00	0.00	0	0.00
20.00	0.01700					.001609	0.03		3.531			0.00		
2421.00	671.95	20.192	692.142	1102.9	8.50	1.122	693.264	0.00	6.071	10.00	13.00	0.00	0	0.00
TRANS STR	0.09217					.004595	0.23					0.00		
2471.67	676.62	12.919	689.539	1102.9	17.34	4.667	694.206	0.00	8.003	9.00	0.00	0.00	0	0.00
8.00	0.00875					.007802	0.06		6.963			0.00		
2479.67	676.69	12.912	689.602	1102.9	17.34	4.667	694.269	0.00	8.003	9.00	0.00	0.00	0	0.00

.
667.23 669.93 672.64 675.34 678.05 680.75 683.45 686.16 688.86 691.57 694.27

N O T E S

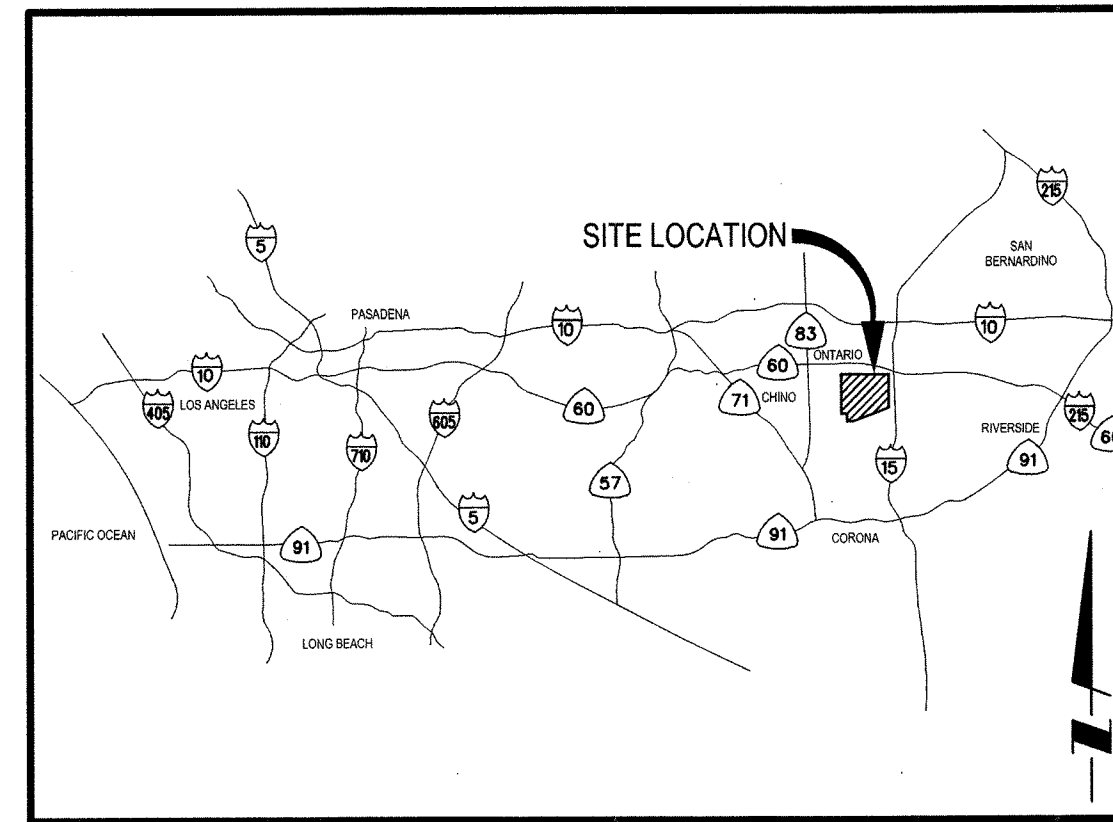
1. GLOSSARY

- I = INVERT ELEVATION
- C = CRITICAL DEPTH
- W = WATER SURFACE ELEVATION
- H = HEIGHT OF CHANNEL
- E = ENERGY GRADE LINE
- X = CURVES CROSSING OVER
- B = BRIDGE ENTRANCE OR EXIT
- Y = WALL ENTRANCE OR EXIT

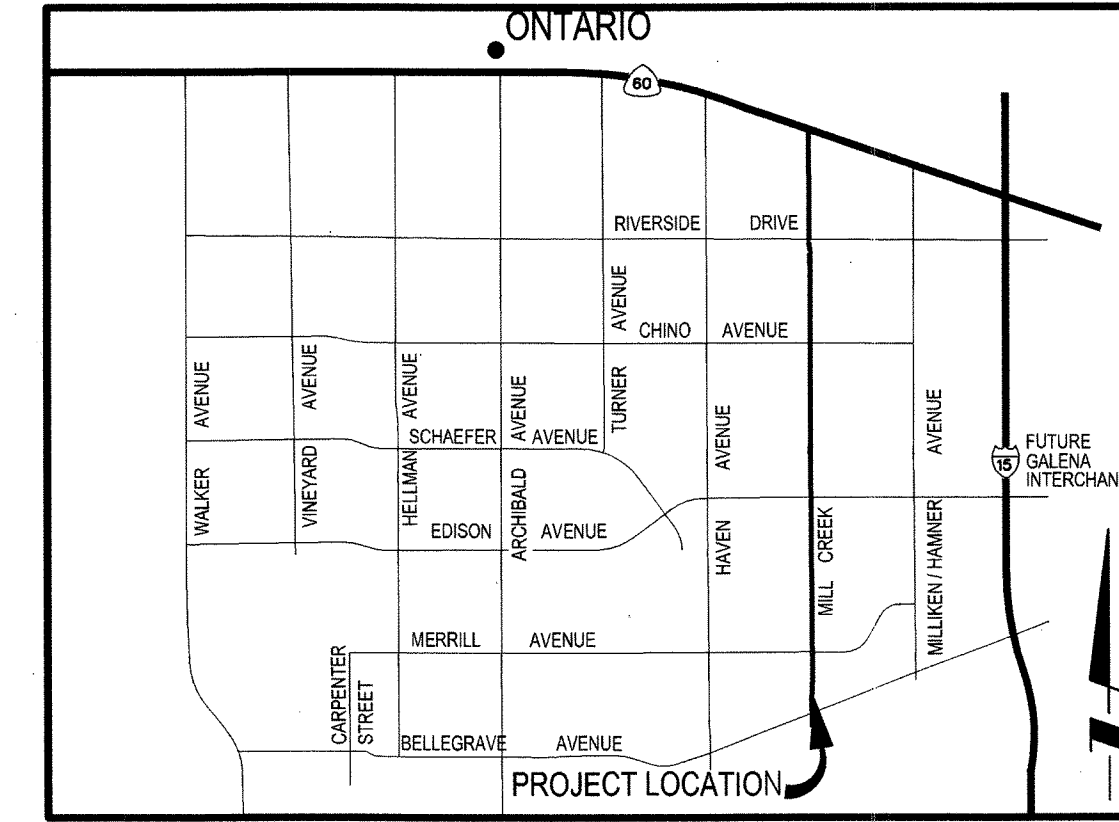
2. STATIONS FOR POINTS AT A JUMP MAY NOT BE PLOTTED EXACTLY



STORM DRAIN IMPROVEMENT PLANS FOR MILL CREEK AVENUE FROM BELLEGRAVE AVENUE TO RIVERSIDE DRIVE IN THE CITY OF ONTARIO



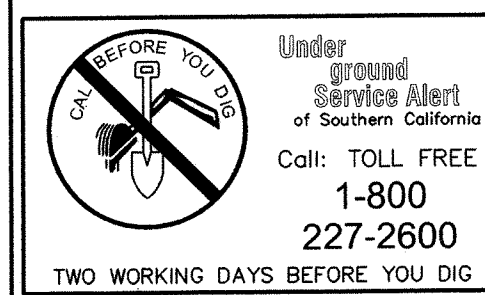
LOCATION MAP
NOT TO SCALE



VICINITY MAP
NOT TO SCALE

SHEET INDEX

SHT. NO.	SHT. NAME	DESCRIPTION
1	TS-01	TITLE SHEET
2	GN-01	GENERAL NOTES & CONSTRUCTION NOTES
3	SD-00	INDEX MAP
4	SD-01	MILL CREEK PLAN & PROFILE STA 10+00.00 TO STA 17+00.00
5	SD-02	MILL CREEK PLAN & PROFILE STA 17+00.00 TO STA 27+00.00
6	SD-03	MILL CREEK PLAN & PROFILE STA 27+00.00 TO STA 34+00.00
7	SD-04	MILL CREEK PLAN & PROFILE STA 34+00.00 TO STA 44+00.00
8	SD-05	MILL CREEK PLAN & PROFILE STA 44+00.00 TO STA 54+00.00
9	SD-06	MILL CREEK PLAN & PROFILE STA 54+00.00 TO STA 63+00.00
10	SD-07	MILL CREEK PLAN & PROFILE STA 63+00.00 TO STA 73+00.00
11	SD-08	MILL CREEK PLAN & PROFILE STA 73+00.00 TO STA 83+00.00
12	SD-09	MILL CREEK PLAN & PROFILE STA 83+00.00 TO STA 93+00.00
13	SD-10	MILL CREEK PLAN & PROFILE STA 93+00.00 TO STA 103+00.00
14	SD-11	MILL CREEK PLAN & PROFILE STA 103+00.00 TO STA 113+00.00
15	SD-12	MILL CREEK PLAN & PROFILE STA 113+00.00 TO STA 123+00.00
16	SD-13	MILL CREEK PLAN & PROFILE STA 123+00.00 TO STA 129+96.90
17	SD-14	LATERAL PROFILES
18	SD-15	LATERAL PROFILES
19	SD-16	LATERAL PROFILES
20	SD-17	LATERAL PROFILES
21	DT-01	DETAIL SHEET - BRICK & MORTAR PLUG AND REMOVE EXISTING STUB-OUT
22	DT-02	DETAIL SHEET - TEMPORARY CSP INLET RISER
23	DT-03	DETAIL SHEET - RCB TO RCB JUNCTION
24	DT-04	DETAIL SHEET - RCB TO RCB JUNCTION
25	DT-05	DETAIL SHEET - WATER QUALITY UNIT PLAN & PROFILE
26	DT-06	DETAIL SHEET - WATER QUALITY UNIT
27	DT-07	DETAIL SHEET - INTERIM CSP RISER



RECOMMENDATION FOR ACCEPTANCE

Tina York 4/9/08
TINA M. YORK, RCE #46367, BUREAU VERITAS
DATE: 4/9/08

CITY ACCEPTANCE OF PLANS DOES NOT RELIEVE THE DEVELOPER FROM RESPONSIBILITY FOR THE CORRECTION OF ERROR AND OMISSION DISCOVERED DURING CONSTRUCTION. UPON REQUEST OF THE CITY INSPECTOR, THE REQUIRED PLAN REVISIONS SHALL BE PROMPTLY SUBMITTED TO THE CITY ENGINEER FOR REVIEW.

MARK	DATE	BY	APPROVED/ RCE NO.

CITY OF ONTARIO

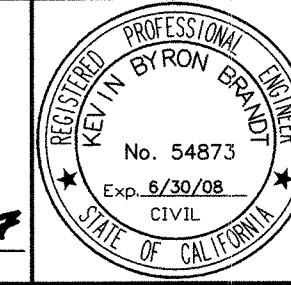
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DRAWN BY: NL DATE: 8/20/07
CHECKED BY: [Signature] DATE: 4/9/08

RECOMMENDED BY: [Signature] ASSISTANT CITY ENGINEER DATE: 3/17/08
ACCEPTED BY: [Signature] CITY ENGINEER DATE: 6/20/08

BENCH MARK No. S.P.S.N 00789 ELEV. 785.572
LOCATION: SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W, 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, S11" "LS 3258" SET FLUSH IN A.C.
NGVD29 / 1990 ADJ.

Stantec CONSULTING INC.
3105 EAST GUASTI ROAD
SUITE 100
ONTARIO, CA 91761
909.390.8880

Kwinn Dandl
54873 8/30/08 1277-07
C.E. NO. EXP. DATE

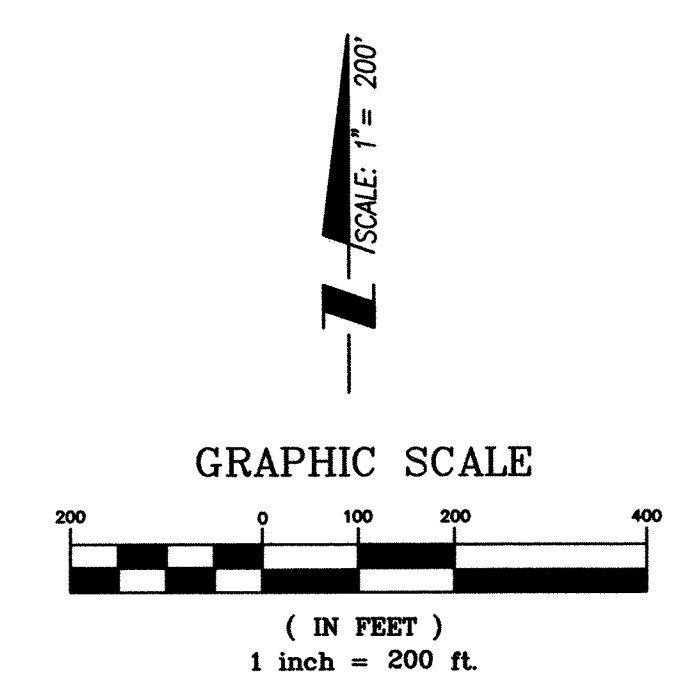
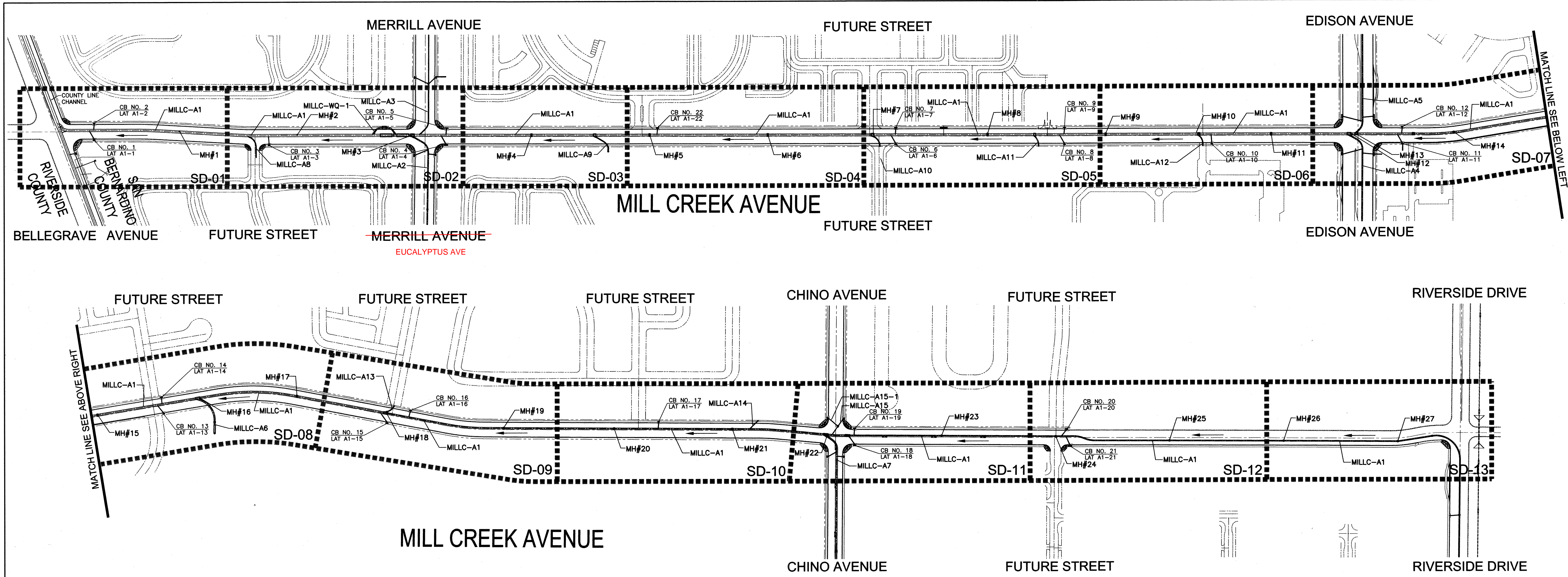


**NEW MODEL COLONY (EAST)
MILL CREEK AVENUE
STORM DRAIN IMPROVEMENT PLANS**
TITLE SHEET

M-442

SHEET 1 OF 27
CONTRACT
ACCOUNT
DWG. NO. D2107
D12004

DRAWING: v:\projects\14355300.dwg\p0172.dwg PLOTTED: 12/10/2007 10:38 AM BY: Allison, Ryan



SHEET LEGEND

SD-01	DRAWING NUMBER
MILLC-A1	STORM DRAIN MAIN LINE FACILITY ID
MILLC-A1-1	STORM DRAIN LATERAL FACILITY ID
CB NO. 1	CATCH BASIN ID
MH #1	MANHOLE ID
→	FLOW DIRECTION

BEFORE YOU DIG
 Under ground Service Alert of Southern California
 Call: TOLL FREE
 1-800-227-2600
 TWO WORKING DAYS BEFORE YOU DIG.

REVISIONS			
MARK	DATE	BY	APPROVED / RCE NO.

CITY OF ONTARIO

DESIGNED BY: BS
 DATE: 8/20/07

DRAWN BY: EH
 DATE: 8/20/07

CHECKED BY: [Signature]
 DATE: 10/19/07

RECOMMENDED BY: [Signature] ASSISTANT CITY ENGINEER
 DATE: 3/2/08

ACCEPTED BY: [Signature] CITY ENGINEER
 DATE: 6/20/08

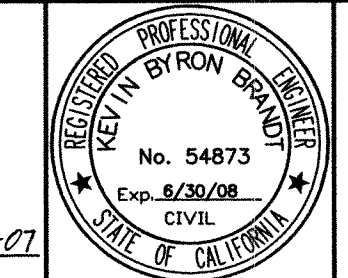
BENCH MARK No. S.P.S.N. 00789 ELEV. 785.572

LOCATION: SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W, 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, S11" "LS 3258" SET FLUSH IN A.C.

NGVD29 / 1990 ADJ.

STANTEC CONSULTING INC.
 3105 EAST GUASTI ROAD
 SUITE 100
 ONTARIO, CA 91761
 909.390.8880

Signature: [Signature]
 DATE: 6/30/08



**NEW MODEL COLONY (EAST)
 MILL CREEK AVENUE
 STORM DRAIN IMPROVEMENT PLANS
 INDEX MAP**

M-442

SHEET **3** OF **27**

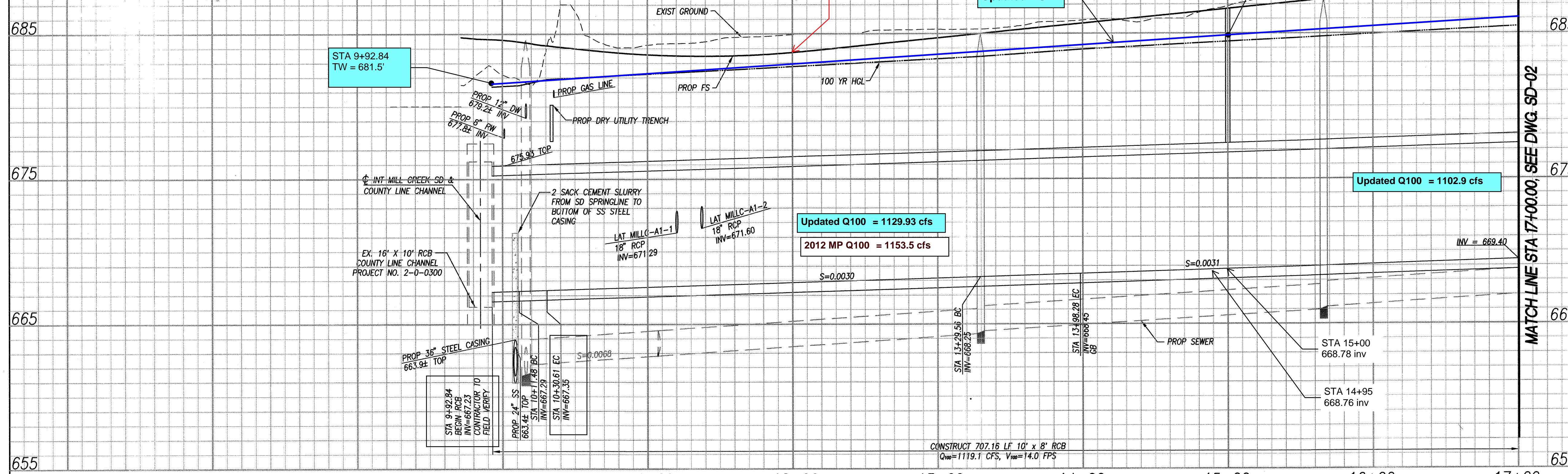
CONTRACT _____

ACCOUNT _____

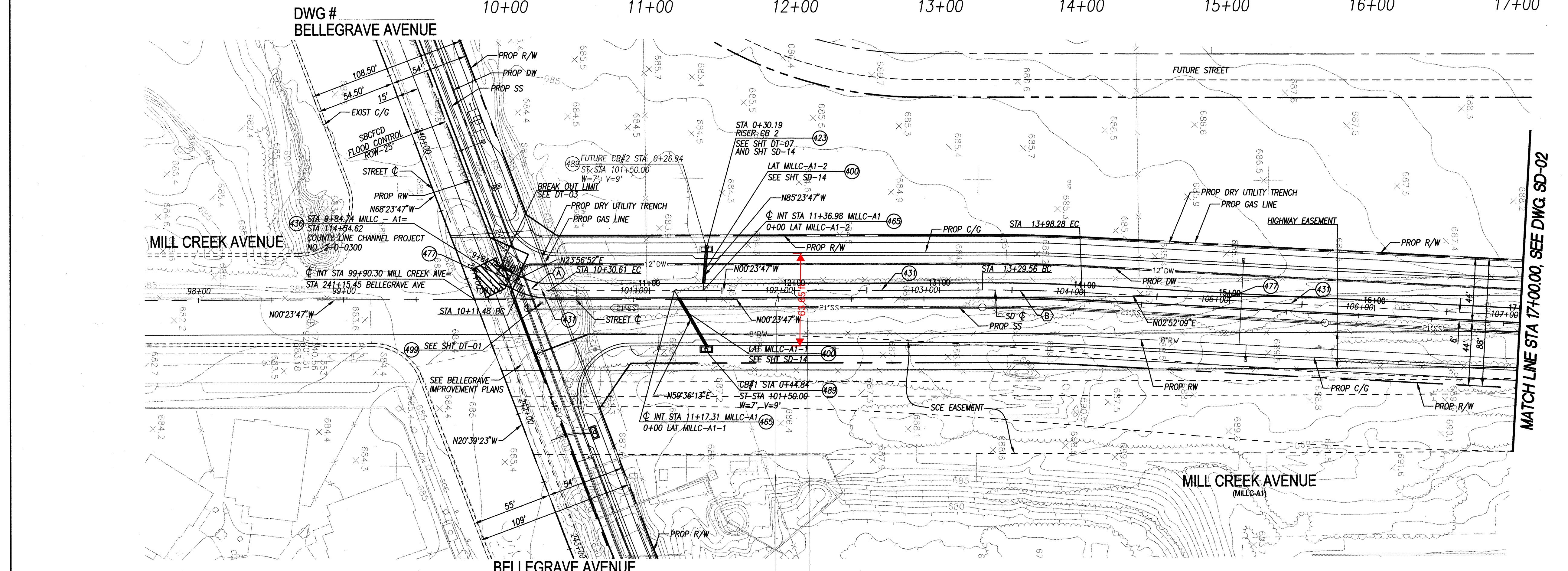
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DRAWING: \\projects\14355\001\dwg\mrad0242.dwg PLOTTED: 9/12/2007 1:42 PM BY: Sunguo, Juan

HORIZ. 1"=40'
VERT. 1"=4'



- ### CONSTRUCTION NOTES
- 400 - CONSTRUCT 18" RCP (D-LOAD PER PROFILE).
 - 423 - CONSTRUCT INTERIM CSP RISER.
 - 431 - CONSTRUCT 10"X8" RCB PER CALTRANS STANDARD DWG. DB0.
 - 436 - CONNECT RCB TO EXISTING RCB PER DETAIL SHOWN ON SHEETS DT-03 & DT-04.
 - 469 - CONSTRUCT JUNCTION STRUCTURE PIPE TO RCB INLET ID 30" OR LESS PER SPPWC STD. PLAN 334-1.
 - 477 - CONSTRUCT MANHOLE-CONCRETE BOX STORM DRAIN PER SPPWC 323-1.
 - 489 - CATCH BASIN NO. 2 PER CITY OF ONTARIO STD. DWG. 3003 (W PER PLANS).
 - 499 - REMOVE EXISTING STUBOUT.

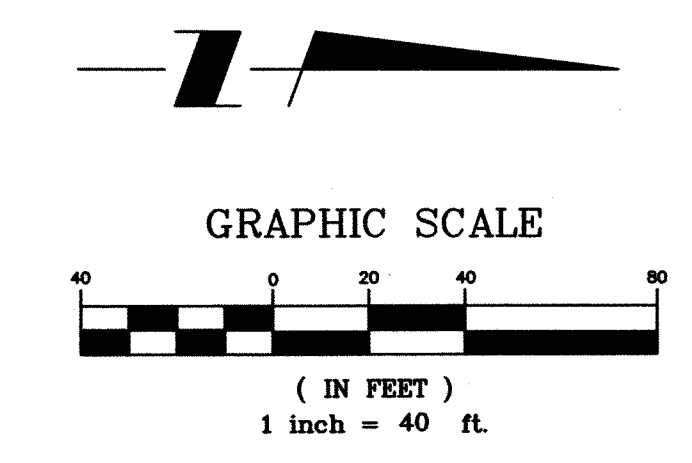


NOTE:
SEE STREET IMPROVEMENT PLANS FOR DISPOSITION OF EXISTING SURFACE FEATURES

NOTE:
ALL EXISTING UNDERGROUND FACILITIES ARE TO BE PROTECTED IN PLACE UNLESS NOTED OTHERWISE ON THE PLANS.

CURVE DATA

Δ	RADIUS	LENGTH	TANGENT
A	24'20"38"	45.00'	19.12' 9.71'
B	03'15"56"	1206.00'	68.74' 34.38'



Underground Services Alert of Southern California
Call: TOLL FREE 1-800-227-2600
TWO WORKING DAYS BEFORE YOU DIG

REVISIONS		DESIGNED BY: NL	DATE: 8/20/07
MARK	DATE	BY	APPROVED/ RCE NO.

CITY OF ONTARIO

RECOMMENDED BY: *[Signature]* DATE: 3/1/08

ACCEPTED BY: *[Signature]* DATE: 6/20/08

BENCH MARK No. S.P.S.N 00789 ELEV.785.572

LOCATION: SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W. 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, S11" "LS 3258" SET FLUSH IN A.C.

NGVD29 / 1990 ADJ.

STANTEC CONSULTING INC.
3105 EAST GUASTI ROAD
SUITE 100
ONTARIO, CA 91761
909.390.8880

[Signature] DATE: 6/30/08

**NEW MODEL COLONY (EAST)
MILL CREEK AVENUE
STORM DRAIN IMPROVEMENT PLANS
STA 10+00.00 TO STA 17+00.00**

M-442

SHEET 4 OF 27

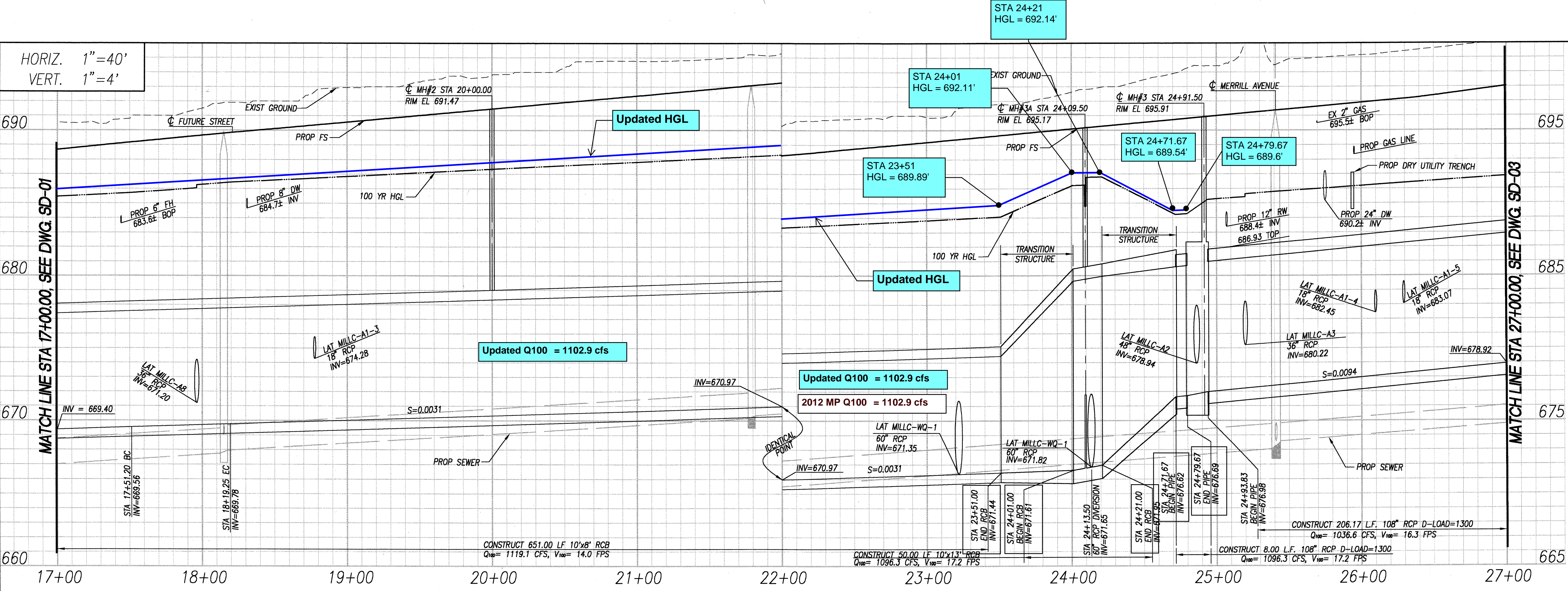
CONTRACT _____

ACCOUNT _____

DWG. NO. D2110

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HORIZ. 1"=40'
VERT. 1"=4'



CONSTRUCTION NOTES

- 400 - CONSTRUCT 18" RCP (D-LOAD PER PROFILE).
- 403 - CONSTRUCT 36" RCP (D-LOAD PER PROFILE).
- 415 - CONSTRUCT 108" RCP (D-LOAD PER PROFILE).
- 423 - CONSTRUCT INTERIM CSP RISER.
- 428 - CONSTRUCT BRICK & MORTAR BULKHEAD (SEE SHEET DT-01)
- 431 - CONSTRUCT 10'X8' RCB PER CALTRANS STANDARD DWG. D80.
- 465 - CONSTRUCT JUNCTION STRUCTURE PIPE TO RCB INLET ID 30" OR LESS PER SPPWC STD. PLAN 334-1.
- 466 - CONSTRUCT JUNCTION STRUCTURE NO.1 PER CITY OF ONTARIO STD. DWG. 3010.
- 467 - CONSTRUCT JUNCTION STRUCTURE NO.2 PER CITY OF ONTARIO STD. DWG. 3012.
- 468 - CONSTRUCT JUNCTION STRUCTURE PIPE TO RCB PER SPPWC STD. PLAN 333-1.
- 475 - CONSTRUCT MANHOLE PIPE TO PIPE (LARGE SIDE INLET) PER SPPWC STD. PLAN 322-1.
- 477 - CONSTRUCT MANHOLE-CONCRETE BOX STORM DRAIN PER SPPWC 323-1.
- 489 - CATCH BASIN NO. 2 PER CITY OF ONTARIO STD. DWG. 3003 (W PER PLANS)
- 492 - CONSTRUCT CATCH BASIN WATER QUALITY STRUCTURE

JUNCTION STRUCTURE TABLE

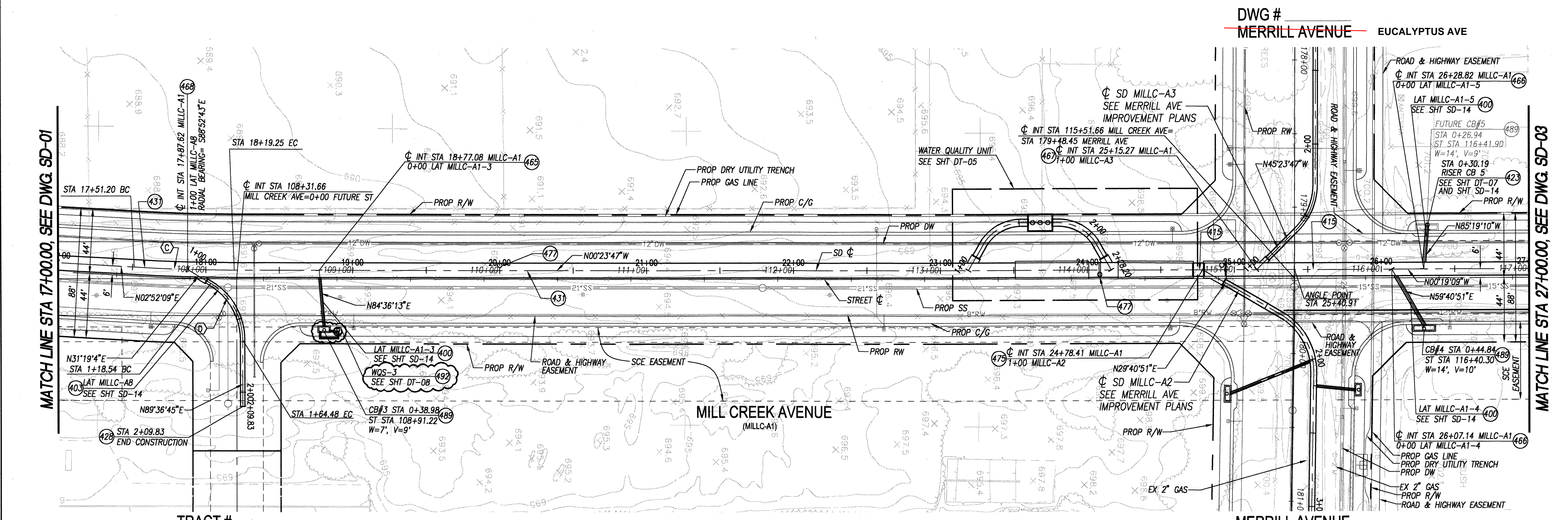
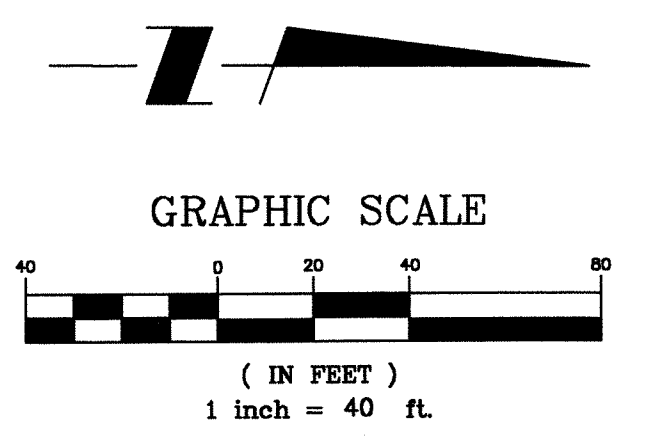
STORM DRAIN C	A	B	C	D1	D2	R	S
17+87.62	30'11"47"	36"	5.00'	10.0'	10.0'	671.56	671.20
24+78.41	30'04"38"	48"	7.73'	9.0'	9.0'	678.79	678.94
25+15.27	45'00"00"	36"	3.80'	9.0'	9.0'	680.36	680.22

NOTE:
SEE STREET IMPROVEMENT PLANS FOR DISPOSITION OF EXISTING SURFACE FEATURES

NOTE:
ALL EXISTING UNDERGROUND FACILITIES ARE TO BE PROTECTED IN PLACE UNLESS NOTED OTHERWISE ON THE PLANS.

CURVE DATA

DELTA	RADIUS	LENGTH	TANGENT
C	03'15"56"	1194.00'	68.05'
D	58'17"41"	45.00'	45.94'



REVISIONS

MARK	DATE	BY	APPROVED / RCE NO.

DESIGNED BY: NL
DATE: 8/20/07
DRAWN BY: NL
DATE: 8/20/07
CHECKED BY: [Signature]
DATE: 1/20/08

CITY OF ONTARIO

RECOMMENDED BY: [Signature] ASSISTANT CITY ENGINEER
DATE: 3/1/08

ACCEPTED BY: [Signature] CITY ENGINEER
DATE: 4/20/08

BENCH MARK No. S.P.S.N. 00789 ELEV. 785.572
LOCATION: SAN BERNARDINO COUNTY BENCHMARK LOCATED AT THE INTERSECTION OF TURNER & RIVERSIDE AVENUES, 1/4 COR. SEC. 2/11, T2S, R7W, 2" BRASS DISK STAMPED "T2S, R7W, 1/4, S2, 511" "LS 3258" SET FLUSH IN A.C.
NGVD29 / 1990 ADJ.

STANTEC CONSULTING INC.
3105 EAST GUASTI ROAD
SUITE 100
ONTARIO, CA 91761
909.390.8880

Signature: [Signature]
DATE: 6/30/08

NEW MODEL COLONY (EAST)
MILL CREEK AVENUE
STORM DRAIN IMPROVEMENT PLANS
STA 17+00.00 TO STA 27+00.00

SD-02

SHEET 5 OF 27

M-442

CONTRACT: [Blank]
ACCOUNT: [Blank]
DWG. NO.: D2111

DRAWING: v:\projects\14355300.dwg\mcd064.dwg PLOTTED: 7/14/2008 3:17 PM BY: Allison Ryan

Appendix C – Excerpt from 2012 Ontario Master Plan of Drainage with Subsequent Approved Revisions

MASTER PLAN OF DRAINAGE

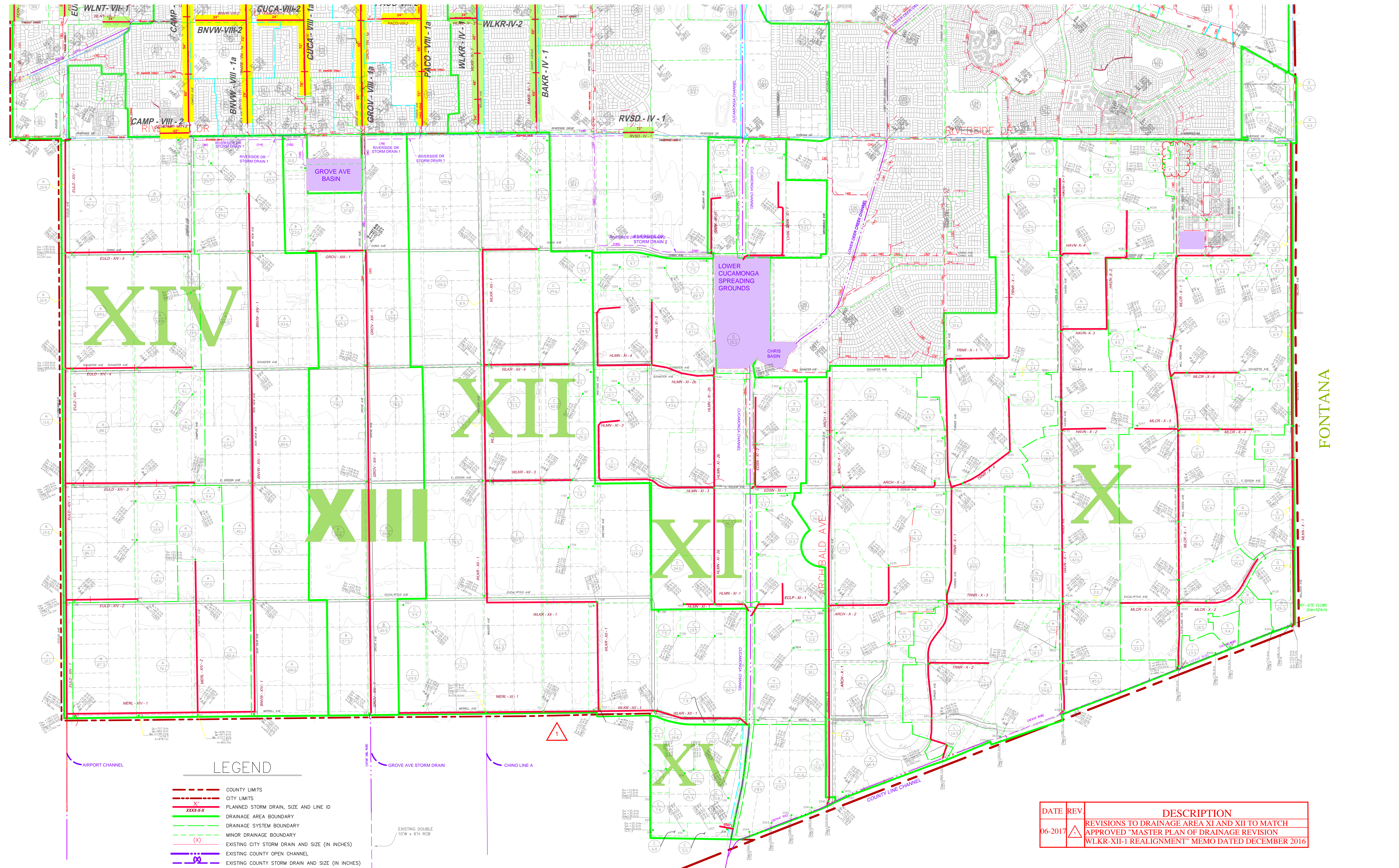
CITY OF ONTARIO

Volume I



FINAL REPORT
DECEMBER 2011
Revised: June, 2017



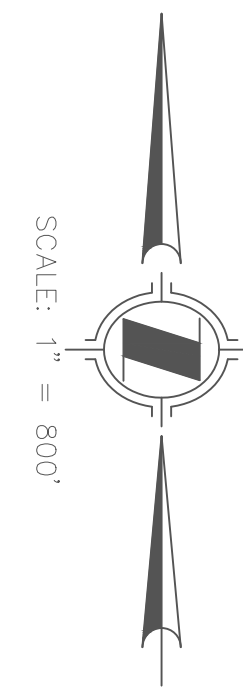


FONTANA

LEGEND

- COUNTY LIMITS
- CITY LIMITS
- PLANNED STORM DRAIN, SIZE AND LINE ID
- DRAINAGE AREA BOUNDARY
- DRAINAGE SYSTEM BOUNDARY
- MINOR DRAINAGE BOUNDARY
- EXISTING CITY STORM DRAIN AND SIZE (IN INCHES)
- EXISTING COUNTY OPEN CHANNEL
- EXISTING COUNTY STORM DRAIN AND SIZE (IN INCHES)
- X DRAINAGE AREA #
- WLKR-XII-3 DRAINAGE SYSTEM NAME
- XII-3 SYSTEM IDENTIFICATION
- XII-3 DRAINAGE AREA NUMBER
- XII-3 LINE NUMBER
- X AREA DESIGNATION
- XX AREA ACREAGE (IN ACRES)
- Q10 = 861.0cfs PEAK FLOW RATE
- Q5 = 931.1cfs TIME OF CONCENTRATION
- T = 36.1 AREA
- A = 342.7ac

DATE	REV.	DESCRIPTION
06-2017	1	REVISIONS TO DRAINAGE AREA XI AND XII TO MATCH APPROVED "MASTER PLAN OF DRAINAGE REVISION WLKR-XII-1 REALIGNMENT" MEMO DATED DECEMBER 2016



REVISIONS 5-20-2017: REVISED TO MERGE DRAINAGE AREA XII AND XIII.	CITY OF ONTARIO MASTER PLAN OF DRAINAGE HYDROLOGY MAP NEW MODEL COLONY
HUNSAKER & ASSOCIATES IRVINE, INC. PLANNING ■ ENGINEERING ■ SURVEYING Three Hughes Irvine, CA 92618 PH: (949) 583-1910 FX: (949) 583-0758	MARCH 2012 SHEET 3 OF 3

Appendix D – Excerpt from “Preliminary H&H for San Bernardino County Flood Control District County Line Channel” (Hunsaker & Associates, 2011)

**PRELIMINARY
HYDROLOGY AND
HYDRAULIC STUDY
FOR
SAN BERNARDINO COUNTY FLOOD
CONTROL DISTRICTS
COUNTY LINE CHANNEL**

PREPARED FOR:

CITY OF ONTARIO
CITY HALL – 303 EAST “B” STREET
ONTARIO, CA 91764

PREPARED BY:

HUNSAKER & ASSOCIATES IRVINE, INC.
THREE HUGHES
IRVINE, CA 92618

PREPARED UNDER THE SUPERVISION OF:



MOHAMMED NAJIEB KHAN ROWTHER, RCE # 37127
EXPIRES: 06/30/2012

Date: September 6, 2011

PRELIMINARY HYDROLOGY AND HYDRAULIC STUDY
FOR
SAN BERNARDINO COUNTY FLOOD CONTROL DISTRICT'S
COUNTY LINE CHANNEL

1. INTRODUCTION

This preliminary hydrology and hydraulics report for the existing County Line Channel has been prepared as a part of the City of Ontario's (City's) Storm Drains Master Plan Update Project. The City is updating its Master Plan of Drainage to evaluate its existing and planned storm drain facilities based on their Revised Land Use Plan, adopted the City Council on January 27, 2010.

As a part of this study, Hunsaker and Associates (H&A) has revised the master plan hydrology for all the existing and proposed future storm drain facilities serving the Old Model Colony and New Model Colony.

2. HYDROLOGIC EVALUATION

The County Line Channel is tributary to Cucamonga Creek and runs in a easterly direction along the San Bernardino/Riverside County Line. Its tributary watershed is comprised of approximately 3,610 acres, of which approximately 2,984 acres are in the easterly portion of New Model Colony and 716 acres from areas to the east of the City of Ontario. The New Model Colony portion of the watershed will be serviced by five (5) storm drain systems that will extend along Archibald Avenue, Turner Avenue, Haven Avenue, Mill Creek Avenue, and Milliken Avenue.

This report includes the revised 100-year hydrology calculation for each of the City's master planned storm drain systems tributary to the County Line Channel, along with a hydrology calculation for the mainstream (County Line Channel) based on the overall watershed.

The revised master plan hydrology study is based on the City's Revised Land Use Plan and San Bernardino County Flood Control District's (District's) Hydrology Manual dated 1986. The revised hydrology study has been prepared using AES's Hydrology Computer Program Version 18.0, released on July 1, 2011.

Peak flow rates in the County Line Channel for the 100-year frequency storm, based on the revised hydrology and that based on the as-built plans, are summarized on the Hydrology Map

3. HYDRAULIC EVALUATION

In order to evaluate the hydraulic capacity of the existing channel to handle its tributary flows based on the revised hydrology of the watershed, hydraulic grade line (HGL) calculation using Los Angeles County Department of Public Works' WSPG program.

Hydraulic model was run for the following two conditions are included in this report:

- Run 1: Using hydraulic data included in the as-built plans for the channel.
- Run 2: Using peak flows calculated in the revised hydrology calculation for the County Line Channel's overall watershed.

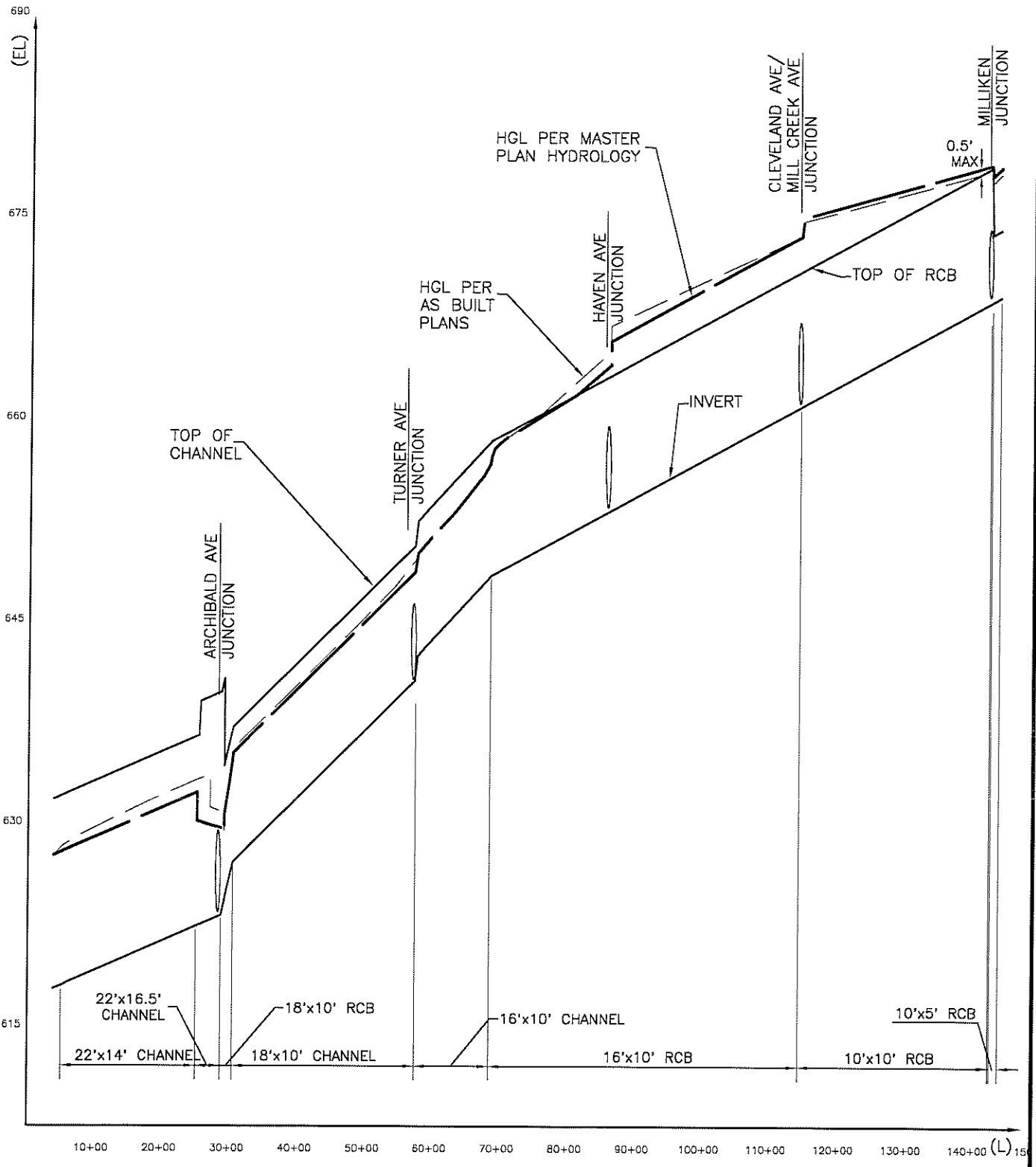
The HGL profile of channel from Cucamonga Channel to Milliken Avenue is shown in the Exhibit 1. It should be noted that 524 cfs is entering the channel from east of Milliken Avenue (from east of City Limits) under both condition, in conformance with the as-built hydraulic data.

4. CONCLUSIONS:

The HGL has a minimum of 1-foot of freeboard in the open channel section of the County Line Channel using 100-year peak flows under both conditions; i.e. as-built flows and the Revised Hydrology flows.

Although the HGL for the Revised Hydrology is slightly higher in the RC Box segment of the channel between Mill Creek Avenue and Milliken Avenue, the water surface in the channel using the revised hydrology flows are lower than the as-built flows for the remaining length of the channel.

Based on the finding of this study the existing County Line Channel has the capacity to convey 100-year frequency runoff, based on the revised hydrology of its tributary watershed.



HYDRAULIC
COUNTY LINE CHANNEL

MASTER PLAN HYDROLOGY COUNTY LINE CHANNEL

Q₁₀₀

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 SAN BERNARDINO CO. HYDROLOGY CRITERION)
(c) Copyright 1983-2011 Advanced Engineering Software (aes)
Ver. 18.0 Release Date: 07/01/2011 License ID 1239

Analysis prepared by:

HUNSAKER & ASSOCIATES
Irvine, Inc
Planning * Engineering * Surveying
Three Hughes * Irvine, California 92618 * (949)583-1010

***** DESCRIPTION OF STUDY *****
* COUNTY LINE CHANNEL *
* LAKE AREAS TO HAVEN & MILL CREEK *
* 100-YR STUDY *

FILE NAME: CHANNEL.DAT
TIME/DATE OF STUDY: 14:00 09/02/2011

=====

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 24.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.90
USER-DEFINED LOGARITHMIC INTERPOLATION USED FOR RAINFALL

SLOPE OF INTENSITY DURATION CURVE(LOG(I;IN/HR) vs. LOG(Tc;MIN)) = 0.6000
USER SPECIFIED 1-HOUR INTENSITY(INCH/HOUR) = 1.2000

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- CROWN TO		STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES:			MANNING FACTOR (n)
	WIDTH (FT)	CROSSFALL (FT)			WIDTH (FT)	LIP (FT)	HIKE (FT)	
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:
1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

UNIT-HYDROGRAPH MODEL SELECTIONS/PARAMETERS:
WATERSHED LAG = 0.80 * Tc
USED "VALLEY UNDEVELOPED" S-GRAPH FOR DEVELOPMENTS OF
2 UNITS/ACRE AND LESS; AND "VALLEY DEVELOPED" S-GRAPH
FOR DEVELOPMENTS OF 3-4 UNITS/ACRE AND MORE.
SIERRA MADRE DEPTH-AREA FACTORS USED.

DURATION	AREA-AVERAGED RAINFALL(INCH)
5-MINUTES	0.44
30-MINUTES	0.91
1-HOUR	1.20
3-HOUR	2.10
6-HOUR	3.00
24-HOUR	6.00

ANTECEDENT MOISTURE CONDITION (AMC) II ASSUMED FOR UNIT HYDROGRAPH METHOD

FLOW PROCESS FROM NODE 6000.00 TO NODE 6130.00 IS CODE = 15.1


```

-----
>>>>DEFINE MEMORY BANK # 1 <<<<<
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PEAK FLOWRATE TABLE FILE NAME: MILLIKEN.DNA
MEMORY BANK # 1 DEFINED AS FOLLOWS:
STREAM      Q      Tc      Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS)  (MIN.)  (INCH/HR)   (ACRES)  NODE
  1         172.14  27.82  0.65( 0.20) 0.31    110.5   6080.00
  2         171.75  29.40  0.65( 0.20) 0.31    114.4   6110.00
  3         161.71  36.89  0.66( 0.19) 0.30    126.0   6000.00
TOTAL AREA(ACRES) = 126.0
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

*****
FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 14.0
-----
>>>>MEMORY BANK # 1 COPIED ONTO MAIN-STREAM MEMORY<<<<<
=====
MAIN-STREAM MEMORY DEFINED AS FOLLOWS:
STREAM      Q      Tc      Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS)  (MIN.)  (INCH/HR)   (ACRES)  NODE
  1         172.14  27.82  0.65( 0.20) 0.31    110.5   6080.00
  2         171.75  29.40  0.65( 0.20) 0.31    114.4   6110.00
  3         161.71  36.89  0.66( 0.19) 0.30    126.0   6000.00
TOTAL AREA(ACRES) = 126.0
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6130.00 = 12690.00 FEET.

*****
FLOW PROCESS FROM NODE 6130.00 TO NODE 6130.00 IS CODE = 16
-----
>>>>USER SPECIFIED CONSTANT SOURCE FLOW AT NODE<<<<<
=====
USER-SPECIFIED CONSTANT SOURCE FLOW = 524.00(CFS)
USER-SPECIFIED AREA ASSOCIATED TO SOURCE FLOW = 716.18(ACRES)
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.18
* SUMMED DATA: FLOW(CFS) = 696.14 TOTAL AREA(ACRES) = 842.18

*****
FLOW PROCESS FROM NODE 6130.00 TO NODE 6200.00 IS CODE = 48
-----
>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 673.60 DOWNSTREAM(FEET) = 672.60
FLOW LENGTH(FEET) = 374.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
FLOWDEPTH IN BOX IS 6.73 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.34
BOX-FLOW(CFS) = 696.14
BOX-FLOW TRAVEL TIME(MIN.) = 0.60 Tc(MIN.) = 28.43
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6200.00 = 13064.00 FEET.

*****
FLOW PROCESS FROM NODE 6200.00 TO NODE 6200.00 IS CODE = 81
-----
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
MAINLINE Tc(MIN.) = 28.43
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.879
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/      SCS SOIL  AREA      Fp      Ap      SCS
LAND USE              GROUP  (ACRES)  (INCH/HR)  (DECIMAL)  CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE"  C      17.10    0.57    0.500    69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
SUBAREA AREA(ACRES) = 17.10 SUBAREA RUNOFF(CFS) = 24.56
EFFECTIVE AREA(ACRES) = 127.61 AREA-AVERAGED Fm(INCH/HR) = 0.21
AREA-AVERAGED Fp(INCH/HR) = 0.63 AREA-AVERAGED Ap = 0.34
TOTAL AREA(ACRES) = 143.1 PEAK FLOW RATE(CFS) = 191.30

```

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
 * SUMMED DATA: FLOW(CFS) = 715.30 TOTAL AREA(ACRES) = 859.3

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	SOURCE FLOW
1	191.41	28.40	1.880	0.63(0.21)	0.34	127.6	6080.00	524.0
2	190.26	29.95	1.821	0.63(0.21)	0.34	131.5	6110.00	0.0
3	178.71	37.43	1.593	0.64(0.21)	0.32	143.1	6000.00	0.0

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 191.41 Tc(MIN.) = 28.40
 AREA-AVERAGED Fm(INCH/HR) = 0.21 AREA-AVERAGED Fp(INCH/HR) = 0.63
 AREA-AVERAGED Ap = 0.34 EFFECTIVE AREA(ACRES) = 127.61

* CUMULATIVE SOURCE FLOW DATA:

FLOW(CFS) = 524.00 AREA(ACRES) = 716.2

* SUMMED DATA:

FLOW(CFS) = 715.41 TOTAL AREA(ACRES) = 843.8

FLOW PROCESS FROM NODE 6200.00 TO NODE 6300.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 672.60 DOWNSTREAM(FEET) = 669.40
 FLOW LENGTH(FEET) = 1140.00 MANNING'S N = 0.015
 GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
 FLOWDEPTH IN BOX IS 6.75 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.60
 BOX-FLOW(CFS) = 715.41
 BOX-FLOW TRAVEL TIME(MIN.) = 1.79 Tc(MIN.) = 30.19
 * TOTAL SOURCE FLOW(CFS) = 524.00
 LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6300.00 = 14204.00 FEET.

FLOW PROCESS FROM NODE 6300.00 TO NODE 6300.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

MAINLINE Tc(MIN.) = 30.19
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.812
 SUBAREA LOSS RATE DATA(AMC II):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN
RESIDENTIAL "5-7 DWELLINGS/ACRE"	C	9.40	0.57	0.500	69

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.500
 SUBAREA AREA(ACRES) = 9.40 SUBAREA RUNOFF(CFS) = 12.93
 EFFECTIVE AREA(ACRES) = 137.01 AREA-AVERAGED Fm(INCH/HR) = 0.22
 AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.35
 TOTAL AREA(ACRES) = 152.5 PEAK FLOW RATE(CFS) = 196.56

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2

* SUMMED DATA: FLOW(CFS) = 720.56 TOTAL AREA(ACRES) = 868.7

** PEAK FLOW RATE TABLE **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE	SOURCE FLOW
1	196.89	30.12	1.814	0.62(0.22)	0.35	137.0	6080.00	524.0
2	195.96	31.60	1.763	0.63(0.22)	0.35	140.9	6110.00	0.0
3	184.39	39.02	1.553	0.63(0.21)	0.33	152.5	6000.00	0.0

NEW PEAK FLOW DATA ARE:

PEAK FLOW RATE(CFS) = 196.89 Tc(MIN.) = 30.12
 AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.62
 AREA-AVERAGED Ap = 0.35 EFFECTIVE AREA(ACRES) = 137.01

* CUMULATIVE SOURCE FLOW DATA:

FLOW(CFS) = 524.00 AREA(ACRES) = 716.2

* SUMMED DATA:

FLOW(CFS) = 720.89 TOTAL AREA(ACRES) = 853.2

FLOW PROCESS FROM NODE 6300.00 TO NODE 6400.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 669.40 DOWNSTREAM(FEET) = 668.60
FLOW LENGTH(FEET) = 293.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
FLOWDEPTH IN BOX IS 6.86 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.51
BOX-FLOW(CFS) = 720.89
BOX-FLOW TRAVEL TIME(MIN.) = 0.46 Tc(MIN.) = 30.59
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 6400.00 = 14497.00 FEET.

FLOW PROCESS FROM NODE 6400.00 TO NODE 6400.00 IS CODE = 81

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

MAINLINE Tc(MIN.) = 30.59
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 1.798
SUBAREA LOSS RATE DATA(AMC II):
DEVELOPMENT TYPE/ SCS SOIL AREA Fp Ap SCS
LAND USE GROUP (ACRES) (INCH/HR) (DECIMAL) CN
RESIDENTIAL
"5-7 DWELLINGS/ACRE" C 12.30 0.57 0.500 69
PUBLIC PARK C 1.00 0.57 0.850 69
SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp(INCH/HR) = 0.57
SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.526
SUBAREA AREA(ACRES) = 13.30 SUBAREA RUNOFF(CFS) = 17.95
EFFECTIVE AREA(ACRES) = 150.31 AREA-AVERAGED Fm(INCH/HR) = 0.22
AREA-AVERAGED Fp(INCH/HR) = 0.62 AREA-AVERAGED Ap = 0.36
TOTAL AREA(ACRES) = 165.8 PEAK FLOW RATE(CFS) = 212.79

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
* SUMMED DATA: FLOW(CFS) = 736.79 TOTAL AREA(ACRES) = 882.0

** PEAK FLOW RATE TABLE **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER SOURCE
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE FLOW
1 212.88 30.57 1.798 0.62(0.22) 0.36 150.3 6080.00 524.0
2 211.53 32.03 1.749 0.62(0.22) 0.36 154.2 6110.00 0.0
3 197.96 39.43 1.544 0.63(0.22) 0.35 165.8 6000.00 0.0
NEW PEAK FLOW DATA ARE:
PEAK FLOW RATE(CFS) = 212.88 Tc(MIN.) = 30.57
AREA-AVERAGED Fm(INCH/HR) = 0.22 AREA-AVERAGED Fp(INCH/HR) = 0.62
AREA-AVERAGED Ap = 0.36 EFFECTIVE AREA(ACRES) = 150.31

* CUMULATIVE SOURCE FLOW DATA:
FLOW(CFS) = 524.00 AREA(ACRES) = 716.2
* SUMMED DATA:
FLOW(CFS) = 736.88 TOTAL AREA(ACRES) = 866.5

FLOW PROCESS FROM NODE 6400.00 TO NODE 5470.00 IS CODE = 48

>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 668.60 DOWNSTREAM(FEET) = 666.10
FLOW LENGTH(FEET) = 990.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 10.00 GIVEN BOX HEIGHT(FEET) = 10.00
FLOWDEPTH IN BOX IS 7.20 FEET BOX-FLOW VELOCITY(FEET/SEC.) = 10.24
BOX-FLOW(CFS) = 736.88
BOX-FLOW TRAVEL TIME(MIN.) = 1.61 Tc(MIN.) = 32.18
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 5470.00 = 15487.00 FEET.

```

*****
FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 15.1
-----
>>>>DEFINE MEMORY BANK # 2 <<<<<
=====
PEAK FLOWRATE TABLE FILE NAME: MCREEK_M.DNA
MEMORY BANK # 2 DEFINED AS FOLLOWS:
PEAK FLOW RATE(CFS) = 1153.54 Tc(MIN.) = 39.35
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 886.1
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.
*****
FLOW PROCESS FROM NODE 5470.00 TO NODE 5470.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER SOURCE
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE FLOW
1 212.88 32.11 1.746 0.62( 0.22) 0.36 150.3 6080.00 524.0
2 211.53 33.52 1.702 0.62( 0.22) 0.36 154.2 6110.00 524.0
3 197.96 40.86 1.511 0.63( 0.22) 0.35 165.8 6000.00 524.0
* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
LONGEST FLOWPATH FROM NODE 6000.00 TO NODE 5470.00 = 15487.00 FEET.
** MEMORY BANK # 2 CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 1153.54 Tc(MIN.) = 39.35
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 886.1
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)=100.0%;VALLEY(UNDEV.)/DESERT= 0.0%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.66; LAG(HR) = 0.52; Fm(INCH/HR) = 0.22; Ybar = 0.25
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.95; 30M = 0.95; 1HR = 0.95;
3HR = 0.99; 6HR = 1.00; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1051.9
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 5470.00 = 16638.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0298; Lca/L=0.4,n=.0267; Lca/L=0.5,n=.0246;Lca/L=0.6,n=.0229
TIME OF PEAK FLOW(HR) = 16.58 RUNOFF VOLUME(AF) = 404.81
PEAK FLOW RATE(CFS) = 1359.19
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 1883.19 TOTAL AREA(ACRES) = 1768.1
*****
FLOW PROCESS FROM NODE 5470.00 TO NODE 4325.00 IS CODE = 48
-----
>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 666.10 DOWNSTREAM(FEET) = 658.00
FLOW LENGTH(FEET) = 2835.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 16.00 GIVEN BOX HEIGHT(FEET) = 10.00
*GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 17.36
ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 10.85
BOX-FLOW(CFS) = 1883.19
BOX-FLOW TRAVEL TIME(MIN.) = 4.36 Tc(MIN.) = 43.70
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19473.00 FEET.
*****
FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 12
-----

```



```
>>>>CLEAR MEMORY BANK # 1 <<<<<
=====
*****
FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 12
-----
```

```
>>>>CLEAR MEMORY BANK # 2 <<<<<
=====
*****
FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 15.1
-----
```

```
>>>>DEFINE MEMORY BANK # 1 <<<<<
=====
PEAK FLOWRATE TABLE FILE NAME: HAVEN_M.DNA
MEMORY BANK # 1 DEFINED AS FOLLOWS:
STREAM      Q      Tc      Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR)    (ACRES)  NODE
1           744.50  18.97  0.95( 0.34) 0.36     402.2   4175.00
2           754.97  22.69  0.95( 0.34) 0.36     463.4   4210.00
3           733.94  32.17  0.95( 0.35) 0.37     584.6   4020.00
4           727.00  33.00  0.95( 0.35) 0.37     591.0    1.00
5           726.70  33.03  0.95( 0.35) 0.37     591.1   4000.00
6           718.96  33.57  0.95( 0.35) 0.37     592.1   4045.00
TOTAL AREA(ACRES) = 592.1
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4325.00 = 15596.00 FEET.
```

```
*****
FLOW PROCESS FROM NODE 4325.00 TO NODE 4325.00 IS CODE = 11
-----
```

```
>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<
=====
```

```
** MAIN STREAM CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 1359.19 Tc(MIN.) = 43.70
AREA-AVERAGED Fm(INCH/HR) = 0.22 Ybar = 0.25
TOTAL AREA(ACRES) = 1051.9
```

```
* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19473.00 FEET.
```

```
** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      Q      Tc      Intensity  Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR) (INCH/HR)  (ACRES)  NODE
1           744.50  18.97  2.395  0.95( 0.34) 0.36     402.2   4175.00
2           754.97  22.69  2.150  0.95( 0.34) 0.36     463.4   4210.00
3           733.94  32.17  1.744  0.95( 0.35) 0.37     584.6   4020.00
4           727.00  33.00  1.718  0.95( 0.35) 0.37     591.0    1.00
5           726.70  33.03  1.717  0.95( 0.35) 0.37     591.1   4000.00
6           718.96  33.57  1.700  0.95( 0.35) 0.37     592.1   4045.00
LONGEST FLOWPATH FROM NODE 1.00 TO NODE 4325.00 = 15596.00 FEET.
```

```
COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)= 99.9%;VALLEY(UNDEV.)/DESERT= 0.1%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.73; LAG(HR) = 0.58; Fm(INCH/HR) = 0.27; Ybar = 0.29
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.93; 30M = 0.93; 1HR = 0.93;
3HR = 0.99; 6HR = 0.99; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1644.0
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 4325.00 = 19473.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0288; Lca/L=0.4,n=.0259; Lca/L=0.5,n=.0238; Lca/L=0.6,n=.0222
TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 595.16
PEAK FLOW RATE(CFS) = 1861.14
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 2385.14 TOTAL AREA(ACRES) = 2360.2
```

```
*****
```

```

FLOW PROCESS FROM NODE 4325.00 TO NODE 3630.00 IS CODE = 48
-----
>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 658.00 DOWNSTREAM(FEET) = 654.30
FLOW LENGTH(FEET) = 1530.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 20.00 GIVEN BOX HEIGHT(FEET) = 10.00
*GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 22.54
ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 10.58
BOX-FLOW(CFS) = 2385.14
BOX-FLOW TRAVEL TIME(MIN.) = 2.41 Tc(MIN.) = 46.11
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3630.00 = 21003.00 FEET.
*****
FLOW PROCESS FROM NODE 3630.00 TO NODE 3630.00 IS CODE = 15.1
-----
>>>>DEFINE MEMORY BANK # 3 <<<<<
=====
PEAK FLOWRATE TABLE FILE NAME: TURN-E.DNA
MEMORY BANK # 3 DEFINED AS FOLLOWS:
STREAM Q Tc Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (ACRES) NODE
1 112.72 25.40 0.97( 0.51) 0.53 83.7 3600.00
TOTAL AREA(ACRES) = 83.7
LONGEST FLOWPATH FROM NODE 3600.00 TO NODE 3630.00 = 4590.00 FEET.
*****
FLOW PROCESS FROM NODE 3630.00 TO NODE 3630.00 IS CODE = 11
-----
>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<
=====
** MAIN STREAM CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 1861.14 Tc(MIN.) = 46.11
AREA-AVERAGED Fm(INCH/HR) = 0.27 Ybar = 0.29
TOTAL AREA(ACRES) = 1644.0

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3630.00 = 21003.00 FEET.

** MEMORY BANK # 3 CONFLUENCE DATA **
STREAM Q Tc Intensity Fp(Fm) Ap Ae HEADWATER
NUMBER (CFS) (MIN.) (INCH/HR) (INCH/HR) (ACRES) NODE
1 112.72 25.40 2.010 0.97( 0.51) 0.53 83.7 3600.00
LONGEST FLOWPATH FROM NODE 3600.00 TO NODE 3630.00 = 4590.00 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)= 99.9%;VALLEY(UNDEV.)/DESERT= 0.1%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.77; LAG(HR) = 0.61; Fm(INCH/HR) = 0.28; Ybar = 0.31
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.92; 30M = 0.92; 1HR = 0.92;
3HR = 0.99; 6HR = 0.99; 24HR= 1.00
UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 1727.7
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3630.00 = 21003.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0285; Lca/L=0.4,n=.0255; Lca/L=0.5,n=.0234;Lca/L=0.6,n=.0219
TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 615.81
PEAK FLOW RATE(CFS) = 1899.26
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 2423.26 TOTAL AREA(ACRES) = 2443.9
*****
FLOW PROCESS FROM NODE 3630.00 TO NODE 3700.00 IS CODE = 48
-----
>>>>COMPUTE BOX-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING USER-SPECIFIED BOX SIZE (EXISTING ELEMENT)<<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 654.30 DOWNSTREAM(FEET) = 653.70
FLOW LENGTH(FEET) = 227.00 MANNING'S N = 0.015
GIVEN BOX BASEWIDTH(FEET) = 20.00 GIVEN BOX HEIGHT(FEET) = 10.00
*GIVEN BOX HEIGHT(FEET) = 10.00 ESTIMATED BOX BASEWIDTH(FEET) = 22.02
ASSUME FULL-FLOWING BOX BOX-FLOW VELOCITY(FEET/SEC.) = 11.01
BOX-FLOW(CFS) = 2423.26
BOX-FLOW TRAVEL TIME(MIN.) = 0.34 Tc(MIN.) = 46.46
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3700.00 = 21230.00 FEET.

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*****
FLOW PROCESS FROM NODE 3700.00 TO NODE 3550.00 IS CODE = 51
-----

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```

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<
-----

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```

ELEVATION DATA: UPSTREAM(FEET) = 653.70 DOWNSTREAM(FEET) = 647.17
CHANNEL LENGTH THRU SUBAREA(FEET) = 1115.00 CHANNEL SLOPE = 0.0059
CHANNEL BASE(FEET) = 16.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 2423.26
FLOW VELOCITY(FEET/SEC.) = 19.06 FLOW DEPTH(FEET) = 7.95
TRAVEL TIME(MIN.) = 0.97 Tc(MIN.) = 47.43
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3550.00 = 22345.00 FEET.

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*****
FLOW PROCESS FROM NODE 3550.00 TO NODE 3550.00 IS CODE = 12
-----

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>>>>CLEAR MEMORY BANK # 1 <<<<
-----

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*****
FLOW PROCESS FROM NODE 3550.00 TO NODE 3550.00 IS CODE = 12
-----

```

```

>>>>CLEAR MEMORY BANK # 3 <<<<
-----

```

```

*****
FLOW PROCESS FROM NODE 3550.00 TO NODE 3550.00 IS CODE = 15.1
-----

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>>>>DEFINE MEMORY BANK # 1 <<<<
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PEAK FLOWRATE TABLE FILE NAME: TURN.DNA
MEMORY BANK # 1 DEFINED AS FOLLOWS:

```

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	582.87	23.83	0.96(0.47)	0.49	390.7	3300.00
2	588.46	30.02	0.96(0.47)	0.49	475.4	3085.00
3	588.12	30.07	0.96(0.47)	0.49	475.8	3500.00
4	520.61	42.17	0.96(0.48)	0.50	565.3	3056.00
5	485.77	47.44	0.96(0.48)	0.50	588.4	3000.00
6	406.33	57.50	0.96(0.48)	0.50	593.5	3021.00
TOTAL AREA(ACRES) =		593.5				

```

LONGEST FLOWPATH FROM NODE 3000.00 TO NODE 3550.00 = 16891.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 3550.00 TO NODE 3550.00 IS CODE = 11
-----

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>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
-----

```

```

** MAIN STREAM CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 1899.26 Tc(MIN.) = 47.43
AREA-AVERAGED Fm(INCH/HR) = 0.28 Ybar = 0.31
TOTAL AREA(ACRES) = 1727.7

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```

* SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(ACRES) = 716.18
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3550.00 = 22345.00 FEET.

```

** MEMORY BANK # 1 CONFLUENCE DATA **

STREAM NUMBER	Q (CFS)	Tc (MIN.)	Intensity (INCH/HR)	Fp(Fm) (INCH/HR)	Ap	Ae (ACRES)	HEADWATER NODE
1	582.87	23.83	2.089	0.96 (0.47)	0.49	390.7	3300.00
2	588.46	30.02	1.818	0.96 (0.47)	0.49	475.4	3085.00
3	588.12	30.07	1.816	0.96 (0.47)	0.49	475.8	3500.00
4	520.61	42.17	1.483	0.96 (0.48)	0.50	565.3	3056.00
5	485.77	47.44	1.382	0.96 (0.48)	0.50	588.4	3000.00
6	406.33	57.50	1.231	0.96 (0.48)	0.50	593.5	3021.00

LONGEST FLOWPATH FROM NODE 3000.00 TO NODE 3550.00 = 16891.00 FEET.

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

UNIT-HYDROGRAPH DATA:

RAINFALL (INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00

S-GRAPH: VALLEY(DEV.)= 99.9%;VALLEY(UNDEV.)/DESERT= 0.1%

MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%

Tc(HR) = 0.79; LAG(HR) = 0.63; Fm(INCH/HR) = 0.33; Ybar = 0.36

USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.

DEPTH-AREA FACTORS: 5M = 0.90; 30M = 0.90; 1HR = 0.90;

3HR = 0.98; 6HR = 0.99; 24HR= 1.00

UNIT-INTERVAL(MIN) = 5.00 TOTAL AREA(ACRES) = 2321.1

LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 3550.00 = 22345.00 FEET.

EQUIVALENT BASIN FACTOR APPROXIMATIONS:

Lca/L=0.3,n=.0278; Lca/L=0.4,n=.0249; Lca/L=0.5,n=.0229;Lca/L=0.6,n=.0214

TIME OF PEAK FLOW(HR) = 16.67 RUNOFF VOLUME(AF) = 770.36

PEAK FLOW RATE(CFS) = 2342.65

* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00 AREA(AC.) = 716.2

* SUMMED DATA: FLOW(CFS) = 2866.65 TOTAL AREA(ACRES) = 3037.3

FLOW PROCESS FROM NODE 3550.00 TO NODE 2550.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 647.17 DOWNSTREAM(FEET) = 634.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 2391.00 CHANNEL SLOPE = 0.0055
CHANNEL BASE(FEET) = 18.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 2866.65
FLOW VELOCITY(FEET/SEC.) = 19.41 FLOW DEPTH(FEET) = 8.20
TRAVEL TIME(MIN.) = 2.05 Tc(MIN.) = 49.48
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 2550.00 = 24736.00 FEET.

FLOW PROCESS FROM NODE 2550.00 TO NODE 2540.00 IS CODE = 51

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 634.00 DOWNSTREAM(FEET) = 630.51
CHANNEL LENGTH THRU SUBAREA(FEET) = 470.00 CHANNEL SLOPE = 0.0074
CHANNEL BASE(FEET) = 18.00 "Z" FACTOR = 0.000
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 10.00
CHANNEL FLOW THRU SUBAREA(CFS) = 2866.65
FLOW VELOCITY(FEET/SEC.) = 21.67 FLOW DEPTH(FEET) = 7.35
TRAVEL TIME(MIN.) = 0.36 Tc(MIN.) = 49.84
* TOTAL SOURCE FLOW(CFS) = 524.00
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 2540.00 = 25206.00 FEET.

FLOW PROCESS FROM NODE 2540.00 TO NODE 2540.00 IS CODE = 12

>>>>CLEAR MEMORY BANK # 1 <<<<<

FLOW PROCESS FROM NODE 2540.00 TO NODE 2540.00 IS CODE = 15.1

>>>>DEFINE MEMORY BANK # 1 <<<<<


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=====
PEAK FLOWRATE TABLE FILE NAME: ARCH.DNA
MEMORY BANK # 1 DEFINED AS FOLLOWS:
STREAM      Q      Tc      Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR)    (ACRES)  NODE
1           767.58  16.84  0.61( 0.32) 0.52     334.4   2638.00
2           778.74  17.86  0.61( 0.32) 0.52     353.2   2660.00
3           779.78  18.02  0.61( 0.32) 0.52     355.7   2245.00
4           779.13  28.95  0.63( 0.33) 0.52     522.8   2600.00
5           778.78  30.22  0.63( 0.33) 0.52     540.6   2050.00
6           732.57  34.59  0.64( 0.33) 0.52     569.3   2215.00
7           718.55  35.65  0.64( 0.33) 0.52     572.3   2110.00
TOTAL AREA(ACRES) =      572.3
LONGEST FLOWPATH FROM NODE 2050.00 TO NODE 2540.00 = 11670.00 FEET.

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*****
FLOW PROCESS FROM NODE 2540.00 TO NODE 2540.00 IS CODE = 11
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>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<
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** MAIN STREAM CONFLUENCE DATA **
PEAK FLOW RATE(CFS) = 2342.65      Tc(MIN.) = 49.84
AREA-AVERAGED Fm(INCH/HR) = 0.33    Ybar = 0.36
TOTAL AREA(ACRES) = 2321.1

* SOURCE FLOW DATA: FLOW(CFS) = 524.00      AREA(ACRES) = 716.18
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 2540.00 = 25206.00 FEET.

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```

** MEMORY BANK # 1 CONFLUENCE DATA **
STREAM      Q      Tc      Intensity  Fp(Fm)      Ap      Ae      HEADWATER
NUMBER      (CFS) (MIN.) (INCH/HR)    (INCH/HR)  (ACRES)  NODE
1           767.58  16.84  2.572  0.61( 0.32) 0.52     334.4   2638.00
2           778.74  17.86  2.483  0.61( 0.32) 0.52     353.2   2660.00
3           779.78  18.02  2.470  0.61( 0.32) 0.52     355.7   2245.00
4           779.13  28.95  1.858  0.63( 0.33) 0.52     522.8   2600.00
5           778.78  30.22  1.811  0.63( 0.33) 0.52     540.6   2050.00
6           732.57  34.59  1.670  0.64( 0.33) 0.52     569.3   2215.00
7           718.55  35.65  1.640  0.64( 0.33) 0.52     572.3   2110.00
LONGEST FLOWPATH FROM NODE 2050.00 TO NODE 2540.00 = 11670.00 FEET.

```

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COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
UNIT-HYDROGRAPH DATA:
RAINFALL(INCH): 5M= 0.44;30M= 0.91;1H= 1.20;3H= 2.10;6H= 3.00;24H= 6.00
S-GRAPH: VALLEY(DEV.)= 99.6%;VALLEY(UNDEV.)/DESERT= 0.4%
MOUNTAIN= 0.0%;FOOTHILL= 0.0%;DESERT(UNDEV.)= 0.0%
Tc(HR) = 0.83; LAG(HR) = 0.66; Fm(INCH/HR) = 0.33; Ybar = 0.35
USED SIERRA MADRE DEPTH-AREA CURVES WITH AMC II CONDITION.
DEPTH-AREA FACTORS: 5M = 0.87; 30M = 0.87; 1HR = 0.87;
3HR = 0.98; 6HR = 0.99; 24HR= 0.99
UNIT-INTERVAL(MIN) = 5.00      TOTAL AREA(ACRES) = 2893.4
LONGEST FLOWPATH FROM NODE 5000.00 TO NODE 2540.00 = 25206.00 FEET.
EQUIVALENT BASIN FACTOR APPROXIMATIONS:
Lca/L=0.3,n=.0266; Lca/L=0.4,n=.0238; Lca/L=0.5,n=.0219;Lca/L=0.6,n=.0204
TIME OF PEAK FLOW(HR) = 16.75  RUNOFF VOLUME(AF) = 960.36
PEAK FLOW RATE(CFS) = 2716.75
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00  AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 3240.75  TOTAL AREA(ACRES) = 3609.6

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END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 2893.4  TC(MIN.) = 49.84
AREA-AVERAGED Fm(INCH/HR)= 0.33  Ybar = 0.35
PEAK FLOW RATE(CFS) = 2716.75
* CUMULATIVE SOURCE FLOW DATA: FLOW(CFS) = 524.00  AREA(AC.) = 716.2
* SUMMED DATA: FLOW(CFS) = 3240.75  TOTAL AREA(ACRES) = 3609.6

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END OF INTEGRATED RATIONAL/UNIT-HYDROGRAPH METHOD ANALYSIS

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WSPG
HYDRAULICS FOR
COUNTY LINE CHANNEL
PER MASTER PLAN
HYDROLOGY

WATER SURFACE PROFILE - CHANNEL DEFINITION LISTING

CARD CODE	SECT NO	CHN TYPE	NO OF PIER/PIP	AVE PIER WIDTH	HEIGHT 1 DIAMETER	BASE WIDTH	ZL	ZR	INV DROP	Y(1)	Y(2)	Y(3)	Y(4)	Y(5)	Y(6)	Y(7)	Y(8)	Y(9)	Y(10)
CD	1	2	0	.000	14.000	22.000			.00										
CD	2	3	0	.000	14.000	22.000	.000	.000	.00										
CD	3	2	0	.000	16.500	22.000			.00										
CD	5	4	1		8.000														
CD	6	3	0	.000	10.000	18.000	.000	.000	.00										
CD	8	2	0	.000	10.000	18.000			.00										
CD	9	4	1		7.500														
CD	10	2	0	.000	10.000	16.000			.00										
CD	11	3	0	.000	10.000	10.000	.000	.000	.00										
CD	12	3	0	.000	10.000	16.000	.000	.000	.00										
CD	13	4	1		2.000														
CD	14	4	1		9.000														
CD	15	4	1		5.000														
CD	16	3	0	.000	5.000	10.000	.000	.000	.00										

W S P G W

WATER SURFACE PROFILE - TITLE CARD LISTING

HEADING LINE NO 1 IS -

W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE

HEADING LINE NO 2 IS -

100-YR STUDY, COUNTY LINE CHANNEL

HEADING LINE NO 3 IS -

LAKE AREAS TO HAVEN & MILL CREEK

W S P G W

WATER SURFACE PROFILE - ELEMENT CARD LISTING

ELEMENT NO	DESCRIPTION	STATION	INVERT	SECT	W S ELEV	RADIUS	ANGLE	ANG PT	MAN H
1	IS A SYSTEM OUTLET	410.000	625.140	1	635.000				
2	IS A REACH	543.890	625.430	1					
3	IS A WALL EXIT	543.890	625.430	2					
4	IS A REACH	568.520	625.480	2					
5	IS A REACH	600.000	625.560	2					
6	IS A WALL ENTRANCE	600.000	625.560	1		300.012	6.012	.000	0
7	IS A REACH	786.920	625.970	1					
8	IS A REACH					300.001	35.699	.000	0

ELEMENT NO	23	IS A	WALL EXIT	6865.290	655.730	10				.014			.000	.000	.000	0
			U/S DATA	STATION	INVERT	SECT										
				6865.290	655.730	12										

W S P G W

PAGE NO 4

ELEMENT NO	24	IS A	REACH	WATER SURFACE PROFILE - ELEMENT CARD LISTING												
			U/S DATA	STATION	INVERT	SECT										
				6960.420	655.990	12										

ELEMENT NO	25	IS A	REACH	*	*	*										
			U/S DATA	STATION	INVERT	SECT										
				6997.170	656.090	12										

ELEMENT NO	26	IS A	REACH	*	*	*										
			U/S DATA	STATION	INVERT	SECT										
				7092.320	656.350	12										

ELEMENT NO	27	IS A	REACH	*	*	*										
			U/S DATA	STATION	INVERT	SECT										
				8619.320	660.530	12										

ELEMENT NO	28	IS A	JUNCTION	*	*	*	*	*								
			U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
				8619.330	660.540	12	5	0	.014	501.900	.000	661.500	.000	-30.000	.000	

ELEMENT NO	29	IS A	REACH	*	*	*										
			U/S DATA	STATION	INVERT	SECT										
				11452.000	668.300	12			.014							

ELEMENT NO	30	IS A	JUNCTION	*	*	*	*	*								
			U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
				11485.000	668.390	11	14	0	.014	1146.300	.000	668.350	.000	-30.000	.000	

ELEMENT NO	31	IS A	REACH	*	*	*										
			U/S DATA	STATION	INVERT	SECT										
				14277.000	676.040	11			.014							

ELEMENT NO	32	IS A	JUNCTION	*	*	*	*	*								
			U/S DATA	STATION	INVERT	SECT	LAT-1	LAT-2	N	Q3	Q4	INVERT-3	INVERT-4	PHI 3	PHI 4	
				14295.000	676.090	16	15	0	.014	212.900	.000	676.500	.000	-30.000	.000	

W S P G W

PAGE NO 5

ELEMENT NO	33	IS A	REACH	WATER SURFACE PROFILE - ELEMENT CARD LISTING												
			U/S DATA	STATION	INVERT	SECT										
				14420.000	676.430	16			.014							

ELEMENT NO	34	IS A	SYSTEM HEADWORKS	*												
			U/S DATA	STATION	INVERT	SECT										
				14420.000	676.430	16										

W S ELEV
676.430

WATER SURFACE PROFILE LISTING
 W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE
 100-YR STUDY, COUNTY LINE CHANNEL
 LAKE AREAS TO HAVEN & MILL CREEK

Date: 8- 4-2011 Time: 8:46:59

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
410.000	625.140	9.860	635.000	3240.80	14.94	3.47	638.47	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
133.890	.0022					.0022	.29	9.86	.84	9.92	.014	.00	.00	RECTANG
543.890	625.430	9.874	635.304	3240.80	14.92	3.46	638.76	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
WALL EXIT														
543.890	625.430	9.874	635.304	3240.80	14.92	3.46	638.76	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
24.630	.0020					.0022	.05	9.87	.84	10.16	.014	.00	.00	BOX
568.520	625.480	9.887	635.367	3240.80	14.90	3.45	638.81	.25	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
31.480	.0025					.0022	.07	10.14	.83	9.35	.014	.00	.00	BOX
WALL ENTRANCE														
600.000	625.560	9.852	635.412	3240.80	14.95	3.47	638.88	.25	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
186.920	.0022					.0022	.41	10.11	.84	9.87	.014	.00	.00	RECTANG
786.920	625.970	9.858	635.828	3240.80	14.94	3.47	639.30	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
1724.235	.0022					.0022	3.79	9.86	.84	9.86	.014	.00	.00	RECTANG
2511.155	629.759	9.863	639.622	3240.80	14.94	3.46	643.09	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
16.979	.0022					.0022	.04	9.86	.84	9.86	.014	.00	.00	RECTANG
2528.135	629.796	9.863	639.659	3240.80	14.94	3.46	643.12	.00	8.77	22.00	14.000	22.000	.00	0 .0
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
HYDRAULIC JUMP														

WATER SURFACE PROFILE LISTING
 W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE
 100-YR STUDY, COUNTY LINE CHANNEL
 LAKE AREAS TO HAVEN & MILL CREEK

Date: 8- 4-2011 Time: 8:46:59

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2528.135	629.796	7.755	637.551	3240.80	19.00	5.60	643.15	.00	8.77	22.00	14.000	22.000	.00	0 .0
28.955	.0022					.0043	.13	7.76	1.20	9.86	.014	.00	.00	RECTANG
2557.090	629.860	7.635	637.495	3240.80	19.29	5.78	643.28	.04	8.77	22.00	14.000	22.000	.00	0 .0
16.910	.0024					.0045	.08	7.68	1.23	9.60	.014	.00	.00	RECTANG
2574.000	629.900	7.568	637.468	3240.80	19.46	5.88	643.35	.04	8.77	22.00	16.500	22.000	.00	0 .0
56.093	.0022					.0048	.27	7.61	1.25	9.86	.014	.00	.00	RECTANG
2630.093	630.023	7.340	637.363	3240.80	20.07	6.25	643.62	.05	8.77	22.00	16.500	22.000	.00	0 .0
91.275	.0022					.0053	.48	7.39	1.31	9.86	.014	.00	.00	RECTANG
2721.368	630.224	6.998	637.223	3240.80	21.05	6.88	644.10	.05	8.77	22.00	16.500	22.000	.00	0 .0
93.562	.0022					.0061	.57	7.05	1.40	9.86	.014	.00	.00	RECTANG
2814.930	630.430	6.673	637.103	3240.80	22.08	7.57	644.67	.00	8.77	22.00	16.500	22.000	.00	0 .0
68.960	.0022					.0068	.47	6.67	1.51	9.90	.014	.00	.00	RECTANG
2883.890	630.580	6.442	637.022	3240.80	22.87	8.12	645.14	.00	8.77	22.00	16.500	22.000	.00	0 .0
JUNCT STR	.0224					.0092	.41	6.44	1.59		.014	.00	.00	RECTANG
2928.890	631.590	5.053	636.643	2866.70	25.79	10.33	646.97	.00	8.08	22.00	16.500	22.000	.00	0 .0
WALL EXIT														
2928.890	631.590	6.384	637.974	2866.70	24.95	9.66	647.64	.00	9.24	18.00	10.000	18.000	.00	0 .0
39.505	.0224					.0090	.35	6.38	1.74	4.72	.014	.00	.00	BOX

WATER SURFACE PROFILE LISTING
 W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE
 100-YR STUDY, COUNTY LINE CHANNEL
 LAKE AREAS TO HAVEN & MILL CREEK

Date: 8- 4-2011 Time: 8:46:59

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
2968.395	632.474	6.673	639.147	2866.70	23.87	8.85	647.99	.00	9.24	18.00	10.000	18.000	.00	0 .0
33.063	.0224					.0079	.26	6.67	1.63	4.72	.014	.00	.00	BOX
3001.458	633.214	6.998	640.213	2866.70	22.76	8.04	648.25	.00	9.24	18.00	10.000	18.000	.00	0 .0
25.218	.0224					.0069	.18	7.00	1.52	4.72	.014	.00	.00	BOX
3026.676	633.779	7.340	641.119	2866.70	21.70	7.31	648.43	.00	9.24	18.00	10.000	18.000	.00	0 .0
18.803	.0224					.0061	.11	7.34	1.41	4.72	.014	.00	.00	BOX
3045.479	634.200	7.698	641.898	2866.70	20.69	6.65	648.54	.00	9.24	18.00	10.000	18.000	.00	0 .0
13.411	.0224					.0054	.07	7.70	1.31	4.72	.014	.00	.00	BOX
3058.890	634.500	8.075	642.575	2866.70	19.72	6.04	648.62	.00	9.24	18.00	10.000	18.000	.00	0 .0
313.500	.0050					.0050	1.57	8.07	1.22	8.07	.014	.00	.00	RECTANG
3372.390	636.070	8.077	644.147	2866.70	19.72	6.04	650.18	.07	9.24	18.00	10.000	18.000	.00	0 .0
257.840	.0050					.0050	1.29	8.15	1.22	8.08	.014	.00	.00	RECTANG
3630.230	637.360	8.078	645.438	2866.70	19.72	6.04	651.47	.00	9.24	18.00	10.000	18.000	.00	0 .0
1522.024	.0050					.0050	7.61	8.08	1.22	8.08	.014	.00	.00	RECTANG
5152.254	644.971	8.078	653.049	2866.70	19.72	6.04	659.08	.00	9.24	18.00	10.000	18.000	.00	0 .0
589.746	.0050					.0050	2.96	8.08	1.22	8.08	.014	.00	.00	RECTANG
5742.000	647.920	8.060	655.980	2866.70	19.76	6.06	662.04	.00	9.24	18.00	10.000	18.000	.00	0 .0
JUNCT STR	.0443					.0053	.22	8.06	1.23		.014	.00	.00	RECTANG

WATER SURFACE PROFILE LISTING
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100-YR STUDY, COUNTY LINE CHANNEL
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Date: 8- 4-2011 Time: 8:46:59

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
5784.000	649.780	7.618	657.398	2385.10	19.57	5.95	663.34	.00	8.84	16.00	10.000	16.000	.00	0 .0
67.000	.0055					.0055	.37	7.62	1.25	7.62	.014	.00	.00	RECTANG
5851.000	650.150	7.616	657.766	2385.10	19.57	5.95	663.71	.00	8.84	16.00	10.000	16.000	.00	0 .0
TRANS STR	.0055					.0048	.10	7.62	1.25		.013	.00	.00	RECTANG
5871.000	650.260	7.637	657.897	2385.10	19.52	5.92	663.81	.00	8.84	16.00	10.000	16.000	.00	0 .0
448.823	.0055					.0055	2.46	7.64	1.24	7.63	.014	.00	.00	RECTANG
6319.823	652.729	7.659	660.388	2385.10	19.46	5.88	666.27	.00	8.84	16.00	10.000	16.000	.00	0 .0
429.875	.0055					.0051	2.20	7.66	1.24	7.63	.014	.00	.00	RECTANG
6749.698	655.094	8.033	663.127	2385.10	18.56	5.35	668.47	.00	8.84	16.00	10.000	16.000	.00	0 .0
95.396	.0055					.0045	.43	8.03	1.15	7.63	.014	.00	.00	RECTANG
6845.094	655.619	8.425	664.044	2385.10	17.69	4.86	668.91	.00	8.84	16.00	10.000	16.000	.00	0 .0
20.196	.0055					.0040	.08	8.42	1.07	7.63	.014	.00	.00	RECTANG
6865.290	655.730	8.837	664.567	2385.10	16.87	4.42	668.99	.28	8.84	16.00	10.000	16.000	.00	0 .0
WALL EXIT														
6865.290	655.730	8.837	664.567	2385.10	16.87	4.42	668.99	.14	8.84	16.00	10.000	16.000	.00	0 .0
38.218	.0027					.0033	.13	8.98	1.00	10.00	.014	.00	.00	BOX
6903.508	655.834	9.268	665.103	2385.10	16.08	4.02	669.12	.13	8.84	16.00	10.000	16.000	.00	0 .0
56.912	.0027					.0031	.18	9.40	.93	10.00	.014	.00	.00	BOX

----- WARNING - Flow depth near top of box conduit -----

WATER SURFACE PROFILE LISTING
 W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE
 100-YR STUDY, COUNTY LINE CHANNEL
 LAKE AREAS TO HAVEN & MILL CREEK

Date: 8- 4-2011 Time: 8:46:59

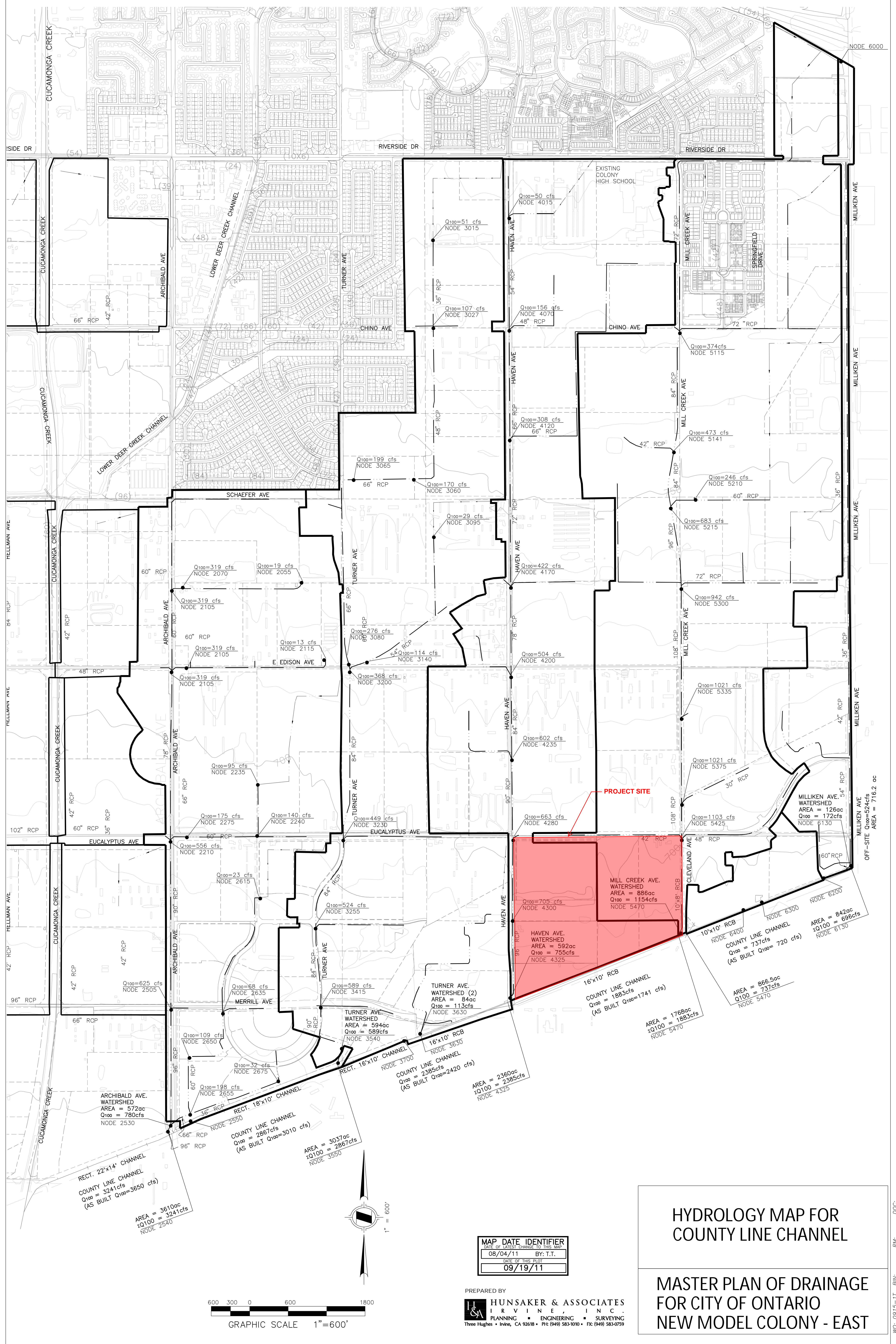
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
6960.420	655.990	9.443	665.433	2385.10	15.79	3.87	669.30	.00	8.84	16.00	10.000	16.000	.00	0 .0
36.750	.0027					.0031	.11	9.44	.91	10.00	.014	.00	.00	BOX
----- WARNING - Flow depth near top of box conduit -----														
6997.170	656.090	9.517	665.607	2385.10	15.66	3.81	669.42	.12	8.84	16.00	10.000	16.000	.00	0 .0
95.150	.0027					.0030	.28	9.64	.89	10.00	.014	.00	.00	BOX
----- WARNING - Flow depth near top of box conduit -----														
7092.320	656.350	9.644	665.994	2385.10	15.46	3.71	669.70	.00	8.84	16.00	10.000	16.000	.00	0 .0
1025.902	.0027					.0027	2.77	9.64	.88	10.00	.014	.00	.00	BOX
----- WARNING - Flow depth near top of box conduit -----														
8118.222	659.158	10.000	669.158	2385.10	14.91	3.45	672.61	.00	8.84	16.00	10.000	16.000	.00	0 .0
501.099	.0027					.0044	2.21	10.00	.83	10.00	.014	.00	.00	BOX
8619.320	660.530	10.837	671.367	2385.10	14.91	3.45	674.82	.00	8.84	16.00	10.000	16.000	.00	0 .0
JUNCT STR	1.0187					.0027	.00	10.84	.83		.014	.00	.00	BOX

TW of 669.16' used
 for Q3 analysis of
 County line

WATER SURFACE PROFILE LISTING
 W.O. #915-1T, ONTARIO MASTER PLAN OF DRAINAGE
 100-YR STUDY, COUNTY LINE CHANNEL
 LAKE AREAS TO HAVEN & MILL CREEK

Date: 8- 4-2011 Time: 8:46:59

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt or I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****	*****
8619.330	660.540	12.583	673.123	1883.20	11.77	2.15	675.27	.00	7.55	16.00	10.000	16.000	.00	0 .0
2832.670	.0027					.0027	7.78	12.58	.66	8.30	.014	.00	.00	BOX
11452.000	668.300	12.606	680.906	1883.20	11.77	2.15	683.06	.00	7.55	16.00	10.000	16.000	.00	0 .0
JUNCT STR	.0027					.0014	.05	12.61	.66		.014	.00	.00	BOX
11485.000	668.390	13.893	682.283	736.90	7.37	.84	683.13	.00	5.52	10.00	10.000	10.000	.00	0 .0
2792.000	.0027					.0014	3.97	13.89	.41	6.60	.014	.00	.00	BOX
14277.000	676.040	10.209	686.249	736.90	7.37	.84	687.09	.00	5.52	10.00	10.000	10.000	.00	0 .0
JUNCT STR	.0028					.0014	.03	10.21	.41		.014	.00	.00	BOX
14295.000	676.090	9.363	685.453	524.00	10.48	1.71	687.16	.00	4.40	10.00	5.000	10.000	.00	0 .0
125.000	.0027					.0049	.62	9.36	.83	5.00	.014	.00	.00	BOX
14420.000	676.430	9.640	686.070	524.00	10.48	1.71	687.78	.00	4.40	10.00	5.000	10.000	.00	0 .0



NODE 6000

SIDE DR

RIVERSIDE DR

RIVERSIDE DR

CUCAMONGA CREEK

ARCHIBALD AVE

LOWER DEER CREEK CHANNEL

TURNER AVE

CHINO AVE

EXISTING COLONY HIGH SCHOOL

MILL CREEK AVE

SPRINGFIELD DRIVE

MILLIKEN AVE

LOWER DEER CREEK CHANNEL

SCHAEFER AVE

TURNER AVE

CHINO AVE

CHINO AVE

MILL CREEK AVE

MILLIKEN AVE

HELLMAN AVE

CUCAMONGA CREEK

ARCHIBALD AVE

ARCHIBALD AVE

TURNER AVE

TURNER AVE

HAVEN AVE

HAVEN AVE

MILL CREEK AVE

MILLIKEN AVE

HELLMAN AVE

CUCAMONGA CREEK

ARCHIBALD AVE

ARCHIBALD AVE

TURNER AVE

TURNER AVE

HAVEN AVE

HAVEN AVE

MILL CREEK AVE

MILLIKEN AVE

HELLMAN AVE

CUCAMONGA CREEK

ARCHIBALD AVE

ARCHIBALD AVE

TURNER AVE

TURNER AVE

HAVEN AVE

HAVEN AVE

MILL CREEK AVE

MILLIKEN AVE

EUCALYPTUS AVE

TURNER AVE

EUCALYPTUS AVE

HAVEN AVE

HAVEN AVE

MILL CREEK AVE

MILLIKEN AVE

PROJECT SITE

MILL CREEK AVE. WATERSHED
AREA = 886ac
Q100 = 1154cfs
NODE 5470

HAVEN AVE. WATERSHED
AREA = 592ac
Q100 = 755cfs
NODE 4325

TURNER AVE. WATERSHED (2)
AREA = 84ac
Q100 = 113cfs
NODE 3630

TURNER AVE. WATERSHED
AREA = 594ac
Q100 = 589cfs
NODE 3540

RECT. 18'x10' CHANNEL
COUNTY LINE CHANNEL
Q100 = 2385cfs
(AS BUILT Q100=2420 cfs)

RECT. 18'x10' CHANNEL
COUNTY LINE CHANNEL
Q100 = 2867cfs
(AS BUILT Q100=3010 cfs)

ARCHIBALD AVE. WATERSHED
AREA = 572ac
Q100 = 780cfs
NODE 2530

RECT. 22'x14' CHANNEL
COUNTY LINE CHANNEL
Q100 = 3241cfs
(AS BUILT Q100=3650 cfs)

AREA = 3610ac
±Q100 = 3241cfs
NODE 2540

AREA = 3037ac
±Q100 = 2867cfs
NODE 3550

AREA = 2360ac
±Q100 = 2385cfs
NODE 4325

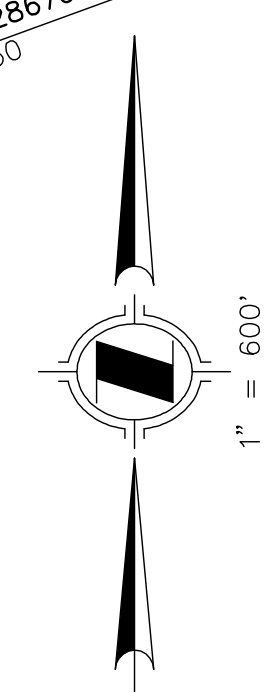
AREA = 1768ac
±Q100 = 1883cfs
NODE 5470

AREA = 866.5ac
Q100 = 737cfs
NODE 5470

AREA = 842ac
±Q100 = 696cfs
NODE 6130

MILLIKEN AVE. WATERSHED
AREA = 126ac
Q100 = 172cfs
NODE 6130

OFF-SITE Q100=524cfs
AREA = 716.2 ac



600 300 0 600 1800
GRAPHIC SCALE 1"=600'

MAP DATE IDENTIFIER	
DATE OF LATEST CHANGE TO THIS MAP	BY: T.T.
DATE OF THIS PLOT	09/19/11

PREPARED BY
HUNSAKER & ASSOCIATES
IRVINE, INCA.
PLANNING ENGINEERING SURVEYING
Three Hughes • Irvine, CA 92618 • P: (949) 583-1010 • F: (949) 583-0759

HYDROLOGY MAP FOR COUNTY LINE CHANNEL

MASTER PLAN OF DRAINAGE FOR CITY OF ONTARIO NEW MODEL COLONY - EAST

WO: 0915-17 BN: DOC: MASTER PLAN OF DRAINAGE FOR CITY OF ONTARIO - NEW MODEL COLONY - EAST

Appendix E – USDA NRCS NCSS Soil Report



United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for San Bernardino County Southwestern Part, California, and Western Riverside Area, California



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

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identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Map Scale: 1:10,200 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 11N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)




















Soils







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at scales ranging from 1:15,800 to 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: San Bernardino County Southwestern Part, California
 Survey Area Data: Version 13, Sep 13, 2021

Soil Survey Area: Western Riverside Area, California
 Survey Area Data: Version 14, Sep 13, 2021

Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or at different levels of detail. This may result in map unit symbols, soil properties, and interpretations that do not completely agree across soil survey area boundaries.

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Nov 15, 2020—Dec 5, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

MAP LEGEND

MAP INFORMATION

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Db	Delhi fine sand	269.0	59.3%
Hr	Hilmar loamy fine sand	80.6	17.8%
Subtotals for Soil Survey Area		349.5	77.1%
Totals for Area of Interest		453.3	100.0%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
DaD2	Delhi fine sand, 2 to 15 percent slopes, wind-eroded	12.7	2.8%
Db	Delhi fine sand	24.4	5.4%
HhA2	Hilmar loamy sand, 0 to 2 percent slopes, eroded	42.5	9.4%
HIA	Hilmar loamy very fine sand, 0 to 2 percent slopes	11.0	2.4%
Hr	Hilmar loamy fine sand	13.2	2.9%
Subtotals for Soil Survey Area		103.7	22.9%
Totals for Area of Interest		453.3	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the

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scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

San Bernardino County Southwestern Part, California

Db—Delhi fine sand

Map Unit Setting

National map unit symbol: hcjq
Elevation: 30 to 1,400 feet
Mean annual precipitation: 10 to 16 inches
Mean annual air temperature: 59 to 64 degrees F
Frost-free period: 225 to 310 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Delhi and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Delhi

Setting

Landform: Alluvial fans
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy alluvium derived from granite

Typical profile

H1 - 0 to 18 inches: fine sand
H2 - 18 to 60 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: A
Ecological site: R019XG912CA - Sandy Fan
Hydric soil rating: No

Minor Components

Unnamed

Percent of map unit: 5 percent
Hydric soil rating: No

Unnamed

Percent of map unit: 5 percent
Landform: Depressions
Hydric soil rating: Yes

Tujunga, loamy sand

Percent of map unit: 5 percent
Hydric soil rating: No

Hr—Hilmar loamy fine sand

Map Unit Setting

National map unit symbol: hck6
Elevation: 540 to 890 feet
Mean annual precipitation: 11 to 15 inches
Mean annual air temperature: 63 to 65 degrees F
Frost-free period: 320 to 365 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Hilmar and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hilmar

Setting

Landform: Alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from granite

Typical profile

A - 0 to 13 inches: loamy fine sand
C1 - 13 to 16 inches: loamy sand
C2 - 16 to 23 inches: loamy sand
2C - 23 to 60 inches: stratified loam to sandy loam

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
(0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Rare
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent

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Maximum salinity: Very slightly saline to moderately saline (2.0 to 8.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: B
Ecological site: R019XG911CA - Loamy Fan
Hydric soil rating: No

Minor Components

Delhi

Percent of map unit: 10 percent
Landform: Alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Tujunga, loamy sand

Percent of map unit: 5 percent
Landform: Alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Western Riverside Area, California

DaD2—Delhi fine sand, 2 to 15 percent slopes, wind-eroded

Map Unit Setting

National map unit symbol: hcsx
Elevation: 200 to 1,400 feet
Mean annual precipitation: 13 inches
Mean annual air temperature: 63 degrees F
Frost-free period: 225 to 310 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Delhi and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Delhi

Setting

Landform: Dunes, alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Convex, linear
Parent material: Wind modified alluvium derived from granite

Typical profile

H1 - 0 to 10 inches: fine sand
H2 - 10 to 48 inches: sand
H3 - 48 to 60 inches: fine sandy loam

Properties and qualities

Slope: 2 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 5.0 inches)

Interpretive groups

Land capability classification (irrigated): 3e
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: A
Ecological site: R019XD035CA - SANDY (1975)
Hydric soil rating: No

Minor Components

Hilmar

Percent of map unit: 5 percent
Hydric soil rating: No

Unnamed

Percent of map unit: 5 percent
Hydric soil rating: No

Tujunga

Percent of map unit: 5 percent
Hydric soil rating: No

Db—Delhi fine sand

Map Unit Setting

National map unit symbol: sqq6
Elevation: 30 to 1,400 feet
Mean annual precipitation: 10 to 16 inches
Mean annual air temperature: 59 to 64 degrees F
Frost-free period: 225 to 310 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Delhi and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Delhi

Setting

Landform: Alluvial fans
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Sandy alluvium derived from granite

Typical profile

H1 - 0 to 18 inches: fine sand
H2 - 18 to 60 inches: sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.4 inches)

Interpretive groups

Land capability classification (irrigated): 3e

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Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: A

Ecological site: R019XG912CA - Sandy Fan

Hydric soil rating: No

Minor Components

Unnamed

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Tujunga, loamy sand

Percent of map unit: 5 percent

Hydric soil rating: No

Unnamed

Percent of map unit: 5 percent

Hydric soil rating: No

HhA2—Hilmar loamy sand, 0 to 2 percent slopes, eroded

Map Unit Setting

National map unit symbol: hcw8

Elevation: 300 to 900 feet

Mean annual precipitation: 11 inches

Mean annual air temperature: 63 degrees F

Frost-free period: 230 to 300 days

Farmland classification: Prime farmland if irrigated

Map Unit Composition

Hilmar and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hilmar

Setting

Landform: Alluvial fans

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Alluvium derived from granite

Typical profile

H1 - 0 to 16 inches: loamy sand

H2 - 16 to 22 inches: stratified very fine sandy loam to silt loam

H3 - 22 to 72 inches: stratified very fine sandy loam to silt loam

Properties and qualities

Slope: 0 to 2 percent

Depth to restrictive feature: More than 80 inches

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Drainage class: Moderately well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Rare
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Very slightly saline to moderately saline (2.0 to 8.0 mmhos/cm)
Sodium adsorption ratio, maximum: 50.0
Available water supply, 0 to 60 inches: Moderate (about 8.3 inches)

Interpretive groups

Land capability classification (irrigated): 2e
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: C
Ecological site: R019XD035CA - SANDY (1975)
Hydric soil rating: No

Minor Components

Tujunga

Percent of map unit: 5 percent
Hydric soil rating: No

Delhi

Percent of map unit: 5 percent
Hydric soil rating: No

Chino

Percent of map unit: 5 percent
Hydric soil rating: No

HIA—Hilmar loamy very fine sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: hcw9
Elevation: 10 to 20 feet
Mean annual precipitation: 8 to 15 inches
Mean annual air temperature: 61 to 63 degrees F
Frost-free period: 250 to 260 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Hilmar and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hilmar

Setting

Landform: Alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from granite

Typical profile

H1 - 0 to 16 inches: loamy fine sand
H2 - 16 to 72 inches: loamy fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Rare
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum: 50.0
Available water supply, 0 to 60 inches: Low (about 4.9 inches)

Interpretive groups

Land capability classification (irrigated): 2e
Land capability classification (nonirrigated): 3e
Hydrologic Soil Group: A
Ecological site: R019XD035CA - SANDY (1975)
Hydric soil rating: No

Minor Components

Delhi

Percent of map unit: 5 percent
Hydric soil rating: No

Chino

Percent of map unit: 5 percent
Hydric soil rating: No

Tujunga

Percent of map unit: 5 percent
Hydric soil rating: No

Hr—Hilmar loamy fine sand

Map Unit Setting

National map unit symbol: sqq5
Elevation: 10 to 2,500 feet
Mean annual precipitation: 10 to 25 inches
Mean annual air temperature: 59 to 64 degrees F
Frost-free period: 225 to 350 days
Farmland classification: Prime farmland if irrigated

Map Unit Composition

Hilmar and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hilmar

Setting

Landform: Alluvial fans
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from granite

Typical profile

A - 0 to 13 inches: loamy fine sand
C1 - 13 to 16 inches: loamy sand
C2 - 16 to 23 inches: loamy sand
2C3 - 23 to 60 inches: stratified loamy sand to loam

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat poorly drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Rare
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Very slightly saline to moderately saline (2.0 to 8.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

Interpretive groups

Land capability classification (irrigated): 2e
Land capability classification (nonirrigated): 3s
***Hydrologic Soil Group:* C**
Ecological site: R019XG911CA - Loamy Fan
Hydric soil rating: No

Minor Components

Delhi

Percent of map unit: 10 percent

Landform: Alluvial fans

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

Tujunga, loamy sand

Percent of map unit: 5 percent

Landform: Alluvial fans

Landform position (three-dimensional): Tread

Down-slope shape: Linear

Across-slope shape: Linear

Hydric soil rating: No

References

- American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.
- American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.
- Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.
- Federal Register. July 13, 1994. Changes in hydric soils of the United States.
- Federal Register. September 18, 2002. Hydric soils of the United States.
- Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.
- National Research Council. 1995. Wetlands: Characteristics and boundaries.
- Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262
- Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577
- Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580
- Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.
- United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.
- United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374
- United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084>

Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf